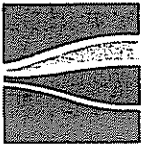


REPORT OF
PRELIMINARY GEOTECHNICAL INVESTIGATION
PROPOSED PLAYA VISTA MARINA
PLAYA VISTA DEVELOPMENT - PARCEL A
LINCOLN BOULEVARD AND BALLONA CREEK
LOS ANGELES COUNTY, CALIFORNIA
FOR
HOWARD HUGHES PROPERTIES





LeROY CRANDALL AND ASSOCIATES

Geotechnical Consultants • One of the Law Companies

900 Grand Central Avenue, Glendale, California 91201-3009, Phone (818) 243-4140, Fax (818) 246-4308

Offices: Glendale • Anaheim • Marina del Rey • San Diego

April 4, 1988

Howard Hughes Properties
13250 Jefferson Boulevard
Los Angeles, California 90094

(LCA AE-88473)

Attention: Mr. Charles A. Alders

Gentlemen:

Our draft "Report of Preliminary Geotechnical Investigation, Proposed Playa Vista Marina, Playa Vista Development - Parcel A, Lincoln Boulevard and Ballona Creek, Los Angeles County, California, for Howard Hughes Properties" is herewith submitted.

The report is being submitted in draft form so that comments and suggestions may be incorporated in the final submittal. We will be pleased to discuss the draft with you at your convenience.

Respectfully submitted,

LeROY CRANDALL AND ASSOCIATES

by

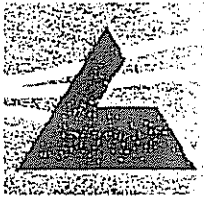
Boris Korin
Project Engineer

by

James L. Van Beveren
Principal Engineer/Vice President

X95/pa
(5 copies submitted)





LAW/CRANDALL, INC. ▲ geotechnical environmental & construction materials consultants
200 Citadel Drive, Los Angeles, California 90040-1554. Phone (213) 889-5300. Fax (213) 721-6700
Los Angeles ▲ Anaheim ▲ Los Alamitos ▲ Marina del Rey ▲ Riverside ▲ San Diego

August 6, 1991

Maguire Thomas Partners
13250 Jefferson Boulevard
Los Angeles, California 90094

(L91096.AFB)

Attention: Mr. Joel Stensby
Vice President

Gentlemen:

Report of Preliminary Geotechnical Investigation
Proposed Marina
Playa Vista Project - Parcel A
Lincoln Boulevard and Ballona Creek
Los Angeles County, California

This letter transmits our prior report of preliminary geotechnical investigation for the proposed marina. That report, which is dated April 4, 1988 (AE-88473), was performed for Howard Hughes Properties and was submitted in draft form. The report was never finalized.

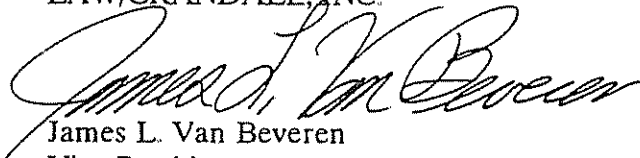
The configuration of the marina has changed somewhat from that described in our April 4, 1988 report. Currently, the marina is planned to extend closer to Ballona Creek and two islands are planned inside the marina. The entrance channel will also be located further to the south. The marina is still planned to be developed by excavating to about Elevation -15. The other aspects of the marina, including perimeter treatment and surrounding development are unchanged from that described in the report.

August 6, 1991
(L91096.AFB)

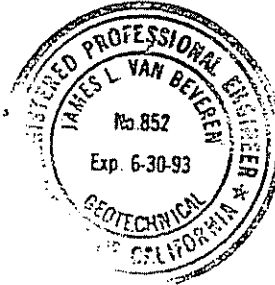
Our report of April 4, 1988 may still be used for preliminary design purposes. As discussed in that report, prior to final design, the recommendations should be reviewed to determine the appropriateness of the design data based on the final configuration.

Sincerely,

LAW/CRANDALL, INC.



James L. Van Beveren
Vice President
Director of Engineering Services



G1-25/bgs
(2 copies submitted)

- cc: (2) Psomas & Associates
Attn: Mr. Jacob Lipa
(1) Moffatt & Nichol, Engineers
Attn: Mr. James Kimo Walker III
(3) City of Los Angeles, Planning Division
(unbound and unpunched)
Attn: Mr. Dick Takase

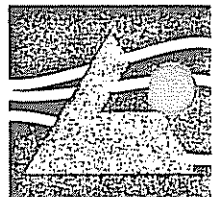


TABLE OF CONTENTS

Section

1.0	SCOPE	1
2.0	PROPOSED CONSTRUCTION	3
3.0	SITE CONDITIONS	4
4.0	GEOLOGY	5
4.1	General	5
4.2	Geologic Materials	6
4.2.1	Fill	6
4.2.2	Holocene Deposits	6
4.2.3	Pleistocene and Older Deposits	7
4.3	Ground Water	7
4.4	Oil and Gas Wells	8
4.5	Natural Gas Storage	8
4.6	Geologic Hazards	9
4.6.1	General	9
4.6.2	Faults	9
4.6.3	Seismicity	11
4.6.4	Liquefaction and Seismically Induced Settlement	12
4.6.5	Stability	14
4.6.6	Subsidence	14
4.6.7	Peat	15
4.6.8	Collapsible Soils	15
4.6.9	Tsunamis and Seiches	15
4.6.10	Flooding	16
5.0	CONCLUSIONS AND RECOMMENDATIONS	17
5.1	General	17
5.2	Foundations	19
5.2.1	General	19
5.2.2	Driven Piling	19
5.2.3	Spread Foundations	22
5.3	Walls Below Grade	23
5.3.1	Gravity Retaining Walls	23
5.3.2	Sheet Pile Walls	24
5.3.3	Seismically-Induced Earth Pressure ...	26
5.3.4	Backfill	27
5.4	Grading	28
5.4.1	Excavation	28
5.4.2	Compacted Fill	30
5.5	Foundations for Structures Adjacent to Marina Perimeter	32

TABLE OF CONTENTS (Continued)

Section

6.0 BIBLIOGRAPHY	33
------------------------	----

APPENDIX A - EXPLORATIONS AND LABORATORY TESTS

APPENDIX B - CONE PENETRATION TESTS

LIST OF PLATES

1. Site Plan	
2. Local Geology	
3. Generalized Soil Profile	
4. Driven Pile Capacities	
5. Pile Driven Criteria	
A-1.1 through A-1.20	Log of Boring
A-2	Unified Soil Classification System
A-3.1 and A-3.2	Direct Shear Test Data
A-4.1 through A-4.12 ..	Consolidation Test Data
A-5.1 through A-5.3 ..	Particle Size Distribution
A-6	Compaction Test Data
A-7	Permeability Test Data
A-8.1 through A-8.6 ...	Report by M. J. Schiff & Associates
B-1.1 through B-1.5 ..	Cone Penetration Plots
B-2.1 through B-2.5 ..	Interpreted Results

1.0 SCOPE

This report presents the results of a preliminary geotechnical investigation of the site of a proposed marina to be located within Parcel A of the Playa Vista Development. The locations of the site and our exploration borings are shown on Plate 1, Site Plan. We previously performed a preliminary environmental audit for the project, and presented the results in our report dated December 21, 1988 (our Job No. F-88473). We performed a concurrent study to determine the location of an existing natural gas storage reservoir, and the effect of the reservoir on the marina construction. The results of that study were presented in a report dated March 31, 1989 (LCA L89101.AEB).

This investigation was authorized to determine the static physical characteristics of the soils beneath the site and to provide recommendations for: perimeter wall design including recommended lateral pressures and foundation support; various sloping configurations of perimeter treatment; information on excavation and dewatering; and preliminary information for location of building foundations adjacent to the perimeter. The investigation was to include a geologic-seismic hazards study to determine any geologic hazards that might affect the proposed construction.

This investigation is based on the presently anticipated layout of the marina. Prior to final design, the recommendations contained herein should be reviewed to determine the appropriateness of the design data based on the final configuration. Additional explorations may be required.

The recommendations contained herein are based on the results of our field explorations and laboratory tests, the engineering analyses based thereon, and on the geologic studies. The results of the exploration

borings and laboratory tests are presented in the attached Appendix A. Cone penetration test results are presented in Appendix B.

Our professional services have been performed using that degree of care and skill ordinarily exercised, under similar circumstances, by reputable geotechnical consultants practicing in this or similar localities. No other warranty, expressed or implied, is made as to the professional advice included in this report. This report has been prepared for Howard Hughes Properties and their design consultants to be used solely in the preliminary design of the proposed marina. The report has not been prepared for use by other parties, and may not contain sufficient information for purposes of other parties or other uses.

2.0 PROPOSED CONSTRUCTION

It is proposed to construct a marina within the subject 139-acre Parcel A; the marina will connect with the existing Marina del Rey. The configuration of proposed marina, as presently planned, is shown on Plate 1. The configuration shown is based on the Playa Vista Land Use Plan; we understand that other configurations are being studied. The marina will be developed by excavating and dredging to about Elevation -15, some 23 to 33 feet below the existing grade. The perimeter treatment for the marina may consist of gravity retaining walls, sheet pile walls, slopes, or combinations of walls and slopes.

The areas surrounding the marina are to be developed with hotels, office buildings, retail stores, and condominiums. Some of these structures may be built near or adjacent to the marina perimeter. Foundation design data for these structures are not within the scope of this report.

3.0 SITE CONDITIONS

The site is located northwest of Lincoln and Culver Boulevards in the Marina del Rey district of Los Angeles County. The property is bordered on the north and northwest by Fiji Way and on the southeast by the Ballona Creek Channel. There are seven wells on or adjacent to the site. The wells were originally drilled as oil wells. One of the wells has been abandoned. The other six wells are operated by the Southern California Gas Company in conjunction with their subterranean natural gas storage reservoir for gas withdrawal and monitoring. There are pipelines between the wells.

The site has been previously filled with excavated materials and dredgings from the existing marina and Ballona Creek Channel, and is about 8 to 18 feet above sea level.

Except for the wells, the property is currently vacant; various dirt roads and paths cross the site. There were structures formerly on the site including radio towers; the foundations may still be in place. The eastern portion of the site was formerly used as a dump for celery trimmings. The site is fenced to restrict access.

The Ballona Creek Channel was excavated in the 1930s and the excavated/dredged soils were placed on the southern portion of the site. Material dredged for development of the existing Marina del Rey in the early 1960s was deposited on the site as hydraulically placed fill.

4.0 GEOLOGY

4.1 GENERAL

The project site is located on the Los Angeles Coastal Plain within the Peninsular Ranges geomorphic province of California which extends from Baja California on the south to the Transverse Ranges on the north. The Peninsular Ranges geomorphic province is characterized by northwest-southeast trending mountain ranges and basins composed predominantly of Mesozoic era igneous and metamorphic rocks and Cenozoic era sedimentary rocks.

The site is situated in the Ballona Creek Flood Plain at the north end of the El Segundo Sand Hills. The Ballona Escarpment, which is an erosional feature caused by the ancient Los Angeles River that flowed through Ballona Gap, is located about one-half mile to the south of the site. The Ballona Escarpment rises approximately 120 to 140 feet above the flood plain and is composed of Pleistocene sediments, dune sand deposits, and the underlying Lakewood and San Pedro Formations.

The site is located approximately 2½ miles west-southwest of the Baldwin Hills, which are part of a northwest trending succession of hills which represent the surface expression of the Newport-Inglewood Fault Zone.

The Pacific Ocean is located approximately one-half mile west-southwest of the site. The current site elevation is about 8 to 18 feet above mean sea level (U.S. Geological Survey datum - Mean Sea Level = Elevation 0).

The southwestern portion of the site overlies a section of the Del Rey Hills area of the Playa del Rey Oil field.

The geology and topography in the vicinity of the site are shown on Plate 2, Local Geology. A section showing the typical soil profile beneath the site is presented on Plate 3, Generalized Soil Profile.

4.2 GEOLOGIC MATERIALS

4.2.1 Fill

Fill soils, 9 to 17½ feet in thickness, were encountered in the borings. The fill consists of silt, clay, and silty sand. The silt and clay are soft to medium stiff; the silty sand is loose to medium dense. There are traces of organic matter in the clayey soils. Debris was encountered and the presence of a hydrocarbon was detected in the fill in Boring 17. The majority of the fill was apparently placed hydraulically during dredging of the Marina del Rey harbor although some dump fill is also apparent. The fill appears to have been placed directly over vegetation present on the surface of the natural soils.

4.2.2 Holocene Deposits

Beneath the fill, the site is underlain by Holocene alluvium extending to an estimated depth of 100 feet below ground surface (California Department of Water Resources, 1961). The alluvial deposits appear to be fairly uniform throughout the parcel, consisting predominantly of cohesive soils to depths of about 50 to 70 feet underlain by dense sand and gravel. The upper cohesive soils

consist of soft to medium stiff silts with some layers of loose to dense silty sand and sand with occasional minor layers of peat. The alluvial deposits are of estuarine origin and contain decomposing organic materials which generate the organic odor (hydrogen sulfide) noted on the boring logs.

The dense sand and gravel deposits were encountered at depths between 48 and 67 feet. These coarser sediments were described by Poland (1959) as the "50-foot gravel," a ground water aquifer.

4.2.3 Pleistocene and Older Deposits

Early Pleistocene age San Pedro Formation sediments underlie the Holocene deposits. These sediments consist primarily of sand with some gravel. Locally, there are thick interbeds of silt. The San Pedro Formation deposits extend to an estimated depth of 200 feet below the site (Poland, 1959). In our other recent investigations in the Ballona Gap area that penetrated into the San Pedro Formation, the sediments consisted of silts and clays.

At depth, approximately 5,800 feet of Tertiary age sedimentary rocks underlie the San Pedro Formation. These sedimentary rocks rest upon metamorphic basement rocks of the Mesozoic (?) age Catalina Schist.

4.3 GROUND WATER

The site is within the Santa Monica Hydrologic Subarea of the Coastal Plain of Los Angeles. Regional ground water levels are near sea level. Ground water was encountered in our borings at

depths of about 7 to 15½ feet below ground surface corresponding to elevations of about 3 feet below sea level to 7 feet above sea level.

Nearly all ground water from the Ballona Gap subunit, which underlies the project area, has been degraded. This degradation has been attributed to sea water intrusion resulting from overpumping in the past.

4.4 OIL AND GAS WELLS

One oil well, designated Del Rey #16, is located in the planned area of excavation. Records on file for the well indicate that it was abandoned on April 17, 1938, in accordance with D.O.G. requirements in effect at that time. The abandonment report (dated May 3, 1938) indicates that the well was abandoned by plugging with cement at depths of 5,981 to 6,074 feet, 1,450 to 1,700 feet, and 502 to 732 feet. This well may have to be reabandoned following current D.O.G. requirements to allow the completion of the proposed marina.

Oil wells Del Rey #13, #14, #15, #17, #18, and #19 are currently used as monitoring and withdrawal wells for gas storage in the Playa del Rey Oil Field, see Plate 1 for the locations of the wells.

4.5 NATURAL GAS STORAGE

The Southern California Gas Company owns the existing on-site and adjacent wells, Del Rey #13 through #19, and uses the underlying Playa Del Rey Oil field for storage of natural gas. Gas storage is

generally at a depth of approximately 6,200 feet below the surface (D.O.G., 1974), although an easement allows storage at depths of 500 to 7,00 feet below ground surface. The lateral extent of the Playa Del Rey Oil Field is shown on Plate 2.

4.6 GEOLOGIC HAZARDS

4.6.1 General

The geologic hazards at the site are essentially limited to those caused by earthquakes. The major cause of damage from earthquakes results from violent shaking from earthquake waves; damage to structures from surface rupture is typically confined to facilities located directly over a fault, and therefore is much less frequent. Violent earthquake shaking would occur not only immediately adjacent to the earthquake epicenter, but within many miles in all directions.

4.6.2 Faults

The numerous faults in Southern California include active, potentially active, and inactive faults. Based on criteria modified from the Association of Engineering Geologists (1973), a fault is considered active if it has moved during historic time (approximately the last 200 years), or is included in a State of California Special Studies Zone for fault rupture hazard. A potentially active faults has moved in the last two million years, but not during historic time. Faults which have not moved in the last two million years are considered inactive.

The closest active fault to the site is the Inglewood Fault of the Newport-Inglewood Fault Zone, located 4.4 miles to the east-northeast. The large number of low magnitude earthquake epicenters on or near the Newport-Inglewood Fault indicates that it is active, at least at depth. The 1933 Long Beach earthquake (Magnitude 6.3) occurred on the Newport-Inglewood Fault Zone.

The active Malibu Coast Fault is located about 7 miles northwest of the site. The major San Andreas Fault Zone is located about 43 miles north-northeast of the site.

The potentially active Charnock Fault is mapped as being located 1.7 miles east-northeast of the site. This fault trends north-northwesterly from the Gardena area, beneath the El Segundo Hills, across Ballona Creek, and through the alluvial narrows southwest of Beverly Hills. We performed a detailed investigation to locate the Charnock Fault in the area of Parcel D between Jefferson Boulevard and the Ballona Escarpment. The results were presented in our report dated May 25, 1988 (LCA AE-86125-L). During our investigation, evidence of the fault was not encountered in late Pleistocene age materials nor was there evidence of a fault-related ground water barrier within the 47- to 71-foot depth explored by our borings.

Other nearby potentially active faults include the Overland Fault located 2½ miles east-northeast of the site, the Santa Monica-Hollywood Fault, 5 miles to the north, and the Palos Verdes Fault, 5 miles to the south-southwest.

The California Division of Oil and Gas report on the Playa del Rey Oil Field (1974) indicates one inactive fault underlying and six small inactive faults near the project area. These faults are depicted as offsetting Miocene age bedrock, but not offsetting Pliocene age rock. This indicates that the faults have been inactive since the beginning of Pliocene (5 million years) time. The potential for surface rupture from these inactive faults is extremely low.

4.6.3 Seismicity

In the last 60 years, three earthquakes have caused major damage in the metropolitan Los Angeles area. These events are the Long Beach, San Fernando, and Whittier Narrows earthquakes. The Long Beach earthquake occurred March 10, 1933. The epicenter of this event was located about 35 miles southeast of the site. This earthquake, although only Richter Magnitude 6.3, ranks as one of the major disasters in Southern California. The majority of damage occurred in structures which are now considered to have been of substandard construction and/or were located on filled or saturated ground.

The epicenter of the Magnitude 6.5 February 9, 1971 San Fernando earthquake was located about 29 miles north of the site. Surface rupture occurred on various strands of the San Fernando Fault Zone including the Sylmar and Tujunga Faults. The large amount of damage caused by this earthquake led to the adoption of more stringent building codes.

The Magnitude 5.9 Whittier Narrows earthquake occurred October 1, 1987 on a previously unrecognized east-west trending reverse fault. The earthquake epicenter was located approximately 22 miles east-northeast of the proposed marina. The majority of structural damage resulting from the earthquake occurred in buildings constructed prior to the more stringent building codes which were developed after the 1971 San Fernando Earthquake.

More recently, two Magnitude 5.0 earthquakes, occurring on December 3, 1988 and January 18, 1989, have shaken the greater Los Angeles area. These earthquakes were not associated with a great amount of damage. The epicenter of the December 3rd earthquake was in the Pasadena area (verbal communication, California Institute of Technology) about 21 miles to the northeast of the site. The January 18, 1989 earthquake occurred offshore in the Santa Monica Bay (verbal communication, California Institute of Technology); the epicenter was located about 9 miles west-southwest of the site.

4.6.4 Liquefaction and Seismically Induced Settlement

Liquefaction potential has been found to be the greatest where the ground water level is shallow and loose fine sands occur within a depth of about 50 feet or less. Liquefaction potential decreases with increasing grain size and clay and gravel content, but increases as the ground acceleration and duration of shaking increase. The County of Los Angeles Seismic Safety Element indicates that the project site is located in an area subject to potential liquefaction based on shallow ground water.

The natural soils consist of soft to medium stiff deposits of silt to depths of 50 to 70 feet with layers of loose to dense silty sand and sand underlain by dense sand and gravel. The relative density of the silty sands and sands within the upper 50 to 70 feet varies from about 60% to 80%. The layers of silty sand and sand within the upper 50 to 70 feet are typically less than three feet in thickness (although an eight-foot thick layer was encountered in one boring) and are confined within layers of cohesive silts. The layers of the looser silty sand and sand have a total thickness of up to 18 feet at some of our boring locations. The relative density of the sand and gravel below the 50- to 70-foot depth is greater than 80%.

Water was measured in our borings at depths of 7 to 15½ feet, corresponding to an elevation of 3 feet below sea level to 7 feet above sea level.

In the event of a severe earthquake on the San Andreas Fault Zone or a moderate earthquake on one of the nearby capable faults, liquefaction of the looser sand and silty sand deposits in the upper 50 feet could occur. The liquefaction would not be extensive across the site, but would be confined to the looser sandy layers of limited thickness. Because the potentially liquefiable soils are confined within cohesive layers, sand boils are not expected to develop. Settlements ranging from about one to four inches should be anticipated due to liquefaction.

4.6.5 Stability

For the most part, the site is essentially level with no known slope stability problems. Slopes present on-site are limited to minor 2:1 (horizontal to vertical) graded (both cut and fill) slopes along the existing channel in the northeastern portion of the site, adjacent to Culver Boulevard, along service roads for the existing oil and gas wells, and bordering the Ballona Creek Channel. These graded slopes are generally ten feet or less in height with no indications of major slope instabilities.

The potential for slope stability problems on-site is judged to be low. Additionally, the site is not on or in the path of any existing or potential landslides.

4.6.6 Subsidence

The site is underlain by the Playa del Rey Oil Field which has been identified as a subsidence area in the Urban Geology Master Plan (California Division of Mines and Geology, 1973). Development within the field began in the 1920s with peak production in the area underlying the site in 1935 (California Division of Oil and Gas, 1974). Minor subsidence on the order of 0.02 to 0.07 feet per year was noted between the years 1925 and 1938 (Castle and Yerkes, 1976) within the vicinity of the oil field. These rates of subsidence apparently decreased with no subsidence noted in the area between 1949 and 1955 (Castle and Yerkes, 1976). The field has been essentially depleted and is now used for underground natural gas storage by the Southern California Gas Company. Accordingly, the potential for continued subsidence at the site is low.

4.6.7 Peat

Peat deposits have been known to cause subsidence due to oxidation and shrinkage in the event of drying. As stated previously, layers of peat were encountered in our borings. The peat layers are below the ground water level and not subject to oxidation or drying. Accordingly, the potential for subsidence due to peat oxidation or drying is low.

4.6.8 Collapsible Soils

Collapsible soils are those soils that are subject to increased consolidation with increase in moisture content. The majority of the soils at the site are either below water level or have been subjected to higher water levels in the past. Collapsible soils were not encountered within our investigation.

4.6.9 Tsunamis and Seiches

Ocean access for the proposed marina will be through the existing entrance channel for Marina del Rey. The estimated run-up for the 100-year tsunamis (seismic sea wave) at the mouth of the entrance channel is 7.9 feet (Houston and Garcia, 1974). Tsunamis could cause the water level in the proposed marina to rise accordingly. Locally generated tsunamis have the potential for greater run-up, although there have been no historic tsunamis generated from local offshore earthquakes.

Seiches are oscillations in a body of water caused by earthquake shaking. The resultant "sloshing" of a captive body of water may

occur during moderate to great earthquakes of Richter Magnitude 5.0 and greater. The result of seiching of the planned marina could cause the water level to temporarily rise or fall a few inches to a few feet.

4.6.10 Flooding

The subject property is located within "Zone C" as designated by Federal Emergency Management Agency for flood hazard (1985). "Zone C" designates areas of minimal flooding outside the 100-year and 500-year flood zones. Ballona Creek which lies along the southeastern boundary of the subject parcel has been channelized to contain the 100-year flood zone. To the north and west of the site, existing improvements along the main channel and the harbor basins contain the 100-year flood zone within Marina del Rey. Estimated base flood elevations within the Marina del Rey facilities are six feet. Similar base flood elevations may be expected within the proposed marina.

5.0 CONCLUSIONS AND RECOMMENDATIONS

5.1 GENERAL

Based on the geologic findings, no active or potentially active faults are known to exist within the site. Accordingly, no surface rupture hazard is deemed to affect the site. The inactive fault beneath the site will not impact the development. The site could be subject to strong ground motion in the event of an earthquake. Due to the nature of the project, hazards due to tsunamis and seiches are inherent. The 100-year run-up in the project area due to tsunamis is 7.9 feet. The potential 100-year flood elevation is approximately six feet, similar to that designated for the existing Marina del Rey facilities. The effects of ground shaking, tsunamis, seiches, and flooding can be mitigated if structures are designed and constructed with current building codes and proper engineering practice. Construction of the marina is not expected to affect the natural gas storage reservoir nor is the reservoir expected to have any effect on the marina.

The proposed marina may be developed as planned. The perimeter treatment may consist of vertical walls, revetted slopes, or combinations of slopes and walls. The upper soils are relatively weak and the required slopes will need to be relatively flat.

The upper soils are not suitable for direct support of retaining walls or other structures on spread footings. Driven piling will be required for support on major walls or other structures. Low walls could be supported on spread footings if the soils beneath the footings are excavated and replaced as compacted fill.

Liquefaction is expected to occur in localized zones of loose silty sand. Based on the borings and cone penetration tests, most of the layers of loose silty sand and sand are less than three feet in thickness. In the event of liquefaction, localized settlement of the ground surface of about one to four inches could occur; the settlement is not expected to be uniform. However, the liquefaction is not expected to affect the stability of the proposed perimeter walls, because of its localized nature. Localized slope sloughing could, however, occur. We do not believe that it will be necessary to design the facilities to resist any liquefaction effects; these conclusions should be verified prior to final design after the design scheme is finalized.

Water was measured in our borings at depths of 7 to 15½ feet. The upper fill soils may be excavated to near the water level using conventional equipment. Below these depths, the soils are not expected to be capable of supporting conventional equipment. Draglines or large backhoes operating from levels above water level could be used to excavate below water level. Dredging could also be considered. Dewatering could be performed to facilitate excavation. The soils are relatively impermeable and trapped water pockets may be encountered during excavation even if the site is dewatered.

The conclusions and recommendations presented below are preliminary and necessarily general in nature. The data contained in this report should be reviewed prior to preparing final plans for the site development. The data are not intended for final design of buildings adjacent to the marina.

5.2 FOUNDATIONS

5.2.1 General

The proposed gravity retaining walls should be supported on driven friction piling. The piles should be driven through the soft upper soils and develop their support into the dense sand and gravel which occur about 60 feet below existing grade (30 feet below dredged level). Minor retaining walls may be supported on spread footings.

5.2.2 Driven Piling

5.2.2.1 Driven Pile Capacities

The downward and upward capacities of 14-inch-square prestressed concrete piles are presented on Plate 4, Driven Pile Capacities. Dead plus live load capacities are shown. A one-third increase may be used when considering wind, seismic, or berthing loads. The indicated capacities are based on the supporting characteristics of the soils and would be applicable for either foundation piling or slip guide piling. The pile section itself should be checked to verify its capability of supporting the imposed loads.

The capacities are presented as a function of penetration into the dense sand and gravel, for various depths to the sand and gravel. The piles should be driven at least five feet into the dense sand and gravel, which was encountered below Elevation -38 to -55; the elevation of the surface of the dense sand and gravel at each boring location is shown on Plate 1.

Piles in groups, if any, should be spaced at least three feet on centers. If so spaced, no reduction in the downward capacity of the piles due to group action need be considered in design.

The settlement of the walls, supported on driven piling in the manner recommended, will be depend on the loads imposed but should be well within acceptable limits.

5.2.2.2 Lateral Resistance

The soils adjacent to an 14-inch-square concrete pile may be assumed to resist horizontal thrusts applied at the top of a pile. The available lateral resistance is presented in the following table.

	<u>1/4-inch Deflection</u> <u>(pounds)</u>	<u>1/2-inch Deflection</u> <u>(pounds)</u>
Free Head	3,500	7,000
Fixed Head	10,000	20,000

The lateral resistance of other sizes of piles would be proportional to the width. The presented capacities are for total loads, including wind, seismic, and impact loads.

In calculating the maximum bending moment in a pile, due to the lateral load applied at the top of the pile, the lateral load may be multiplied by an assumed moment arm of four feet. For design, it may be assumed that the maximum bending moment will occur at the pile cap or at the adjacent grade, whichever is lower, and that the

bending moment will decrease to zero at a depth of 20 feet below the bottom of the pile cap or adjacent grade.

Batter piles may also be used to resist lateral loads. Batter piles should also be driven at least five feet into the dense sand and gravel. The axial capacity of a batter pile may be taken as equal to the capacity of a vertical pile driven to the same tip elevation.

5.2.2.3 Installation

We anticipate that the driving resistance will range from very low to high. All piles should be driven to the lengths discussed above, except as may be modified on the basis of the driving criteria defined on Plate 5, Pile Driving Criteria. The piles may run until they encounter a stiffer or denser layer. The driving resistance may be moderately high in the silty sand and sand layers and in the stiffer silt layers. Hard driving may be encountered within the deeper dense sand and gravel, and some predrilling may be required. The diameter of the predrilled hole should not exceed the width of the pile. Predrilling may be performed to within five feet of the design tip elevation of the pile.

Prior to ordering the production piles, an indicator pile program will be required to evaluate the driving resistance prior to ordering production piling. The program can be developed after the design is finalized.

5.2.3 Spread Foundations

5.2.3.1 Bearing Value

Retaining walls less than about six feet in height and extending less than ten feet below the existing ground surface, may be supported on spread footings established in properly compacted fill. Footings extending at least 2 feet below the adjacent grade and underlain by at least three feet of compacted fill may be designed for a soil bearing pressure 1,500 pounds per square foot. The allowable bearing pressure would have to be reduced if the footings are located near the face of a downward slope.

While the actual bearing value of any required fill will depend on the material used and the compaction methods employed, the quoted bearing values will be applicable if acceptable soils are used and are compacted as recommended. The bearing value of the fill should be confirmed during the grading.

5.2.3.2 Lateral Resistance

A coefficient of friction of 0.4 may be used between the footings and the supporting soils. The passive pressure against footings may be assumed to be equal to that developed by a fluid with a density of 250 pounds per cubic foot. The available passive pressure would have to be reduced if the footings are near the face of a downward slope.

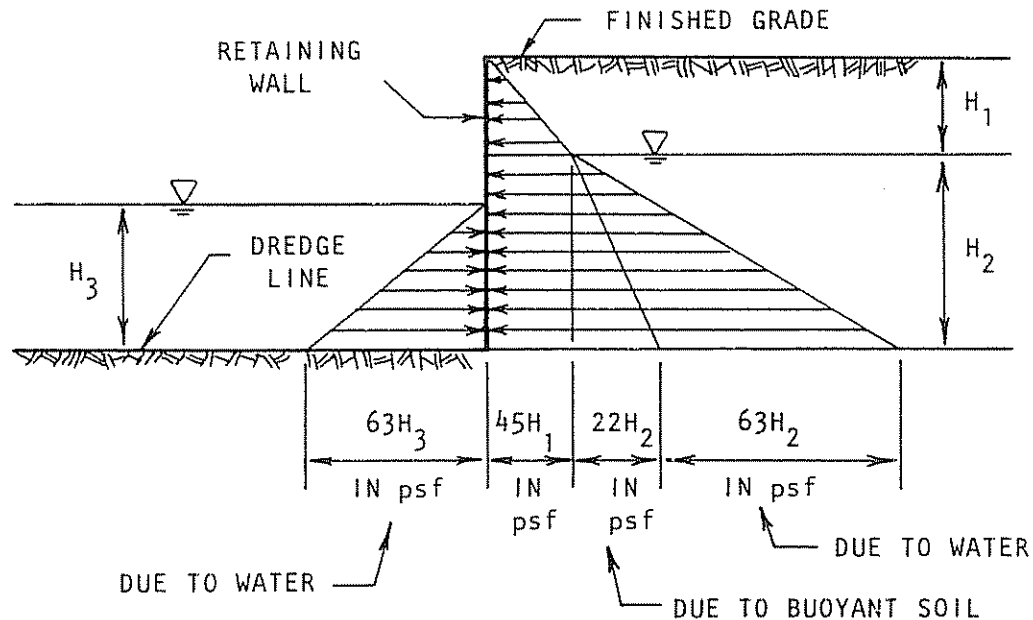
5.2.3.3 Foundation Observation

To verify the presence of satisfactory soils at design elevations, all footing excavations should be observed by personnel of our firm. Inspection of footing excavations may also be required by the appropriate reviewing governmental agencies. The contractor should familiarize himself with the inspection requirements of the reviewing agencies.

5.3 WALLS BELOW GRADE

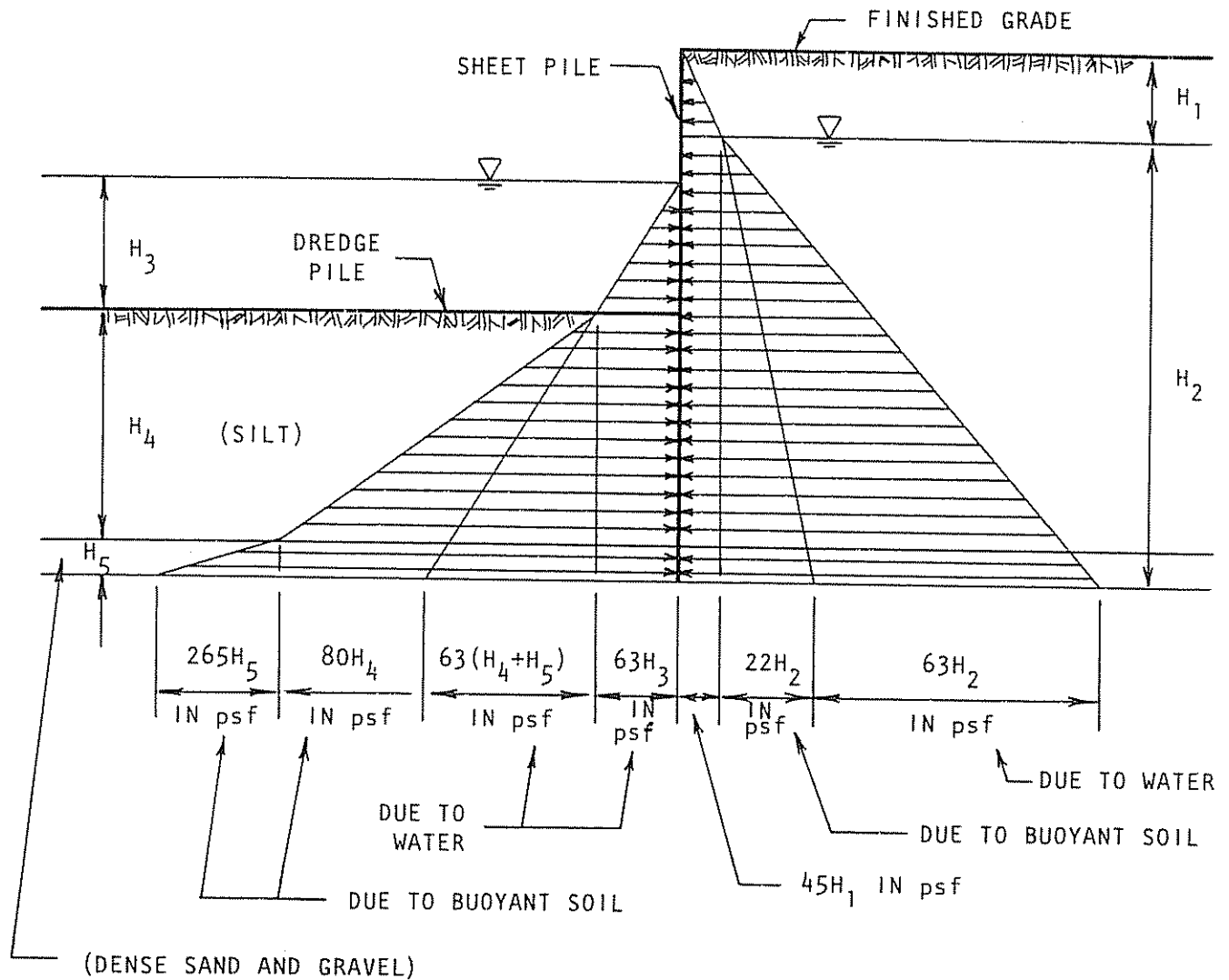
5.3.1 Gravity Retaining Walls

For design of gravity retaining walls, it may be assumed that the lateral pressure of the on-site soils with level backfill above the water level will be equal to that developed by a fluid with a density of 45 pounds per cubic foot. Below the water level, the lateral pressure due to the soil and the water will be equal to that developed by a fluid with a density of 85 pounds per cubic foot. (The soil buoyant pressure would be 22 pounds per cubic foot and the water would be 63 pounds per cubic foot.) The recommended pressure distribution is shown on the following page. (The water pressure on the marina side of the wall may be considered in the design.) If sloping backfill is used, the pressure would be greater; we can provide the necessary data when the configuration is known.



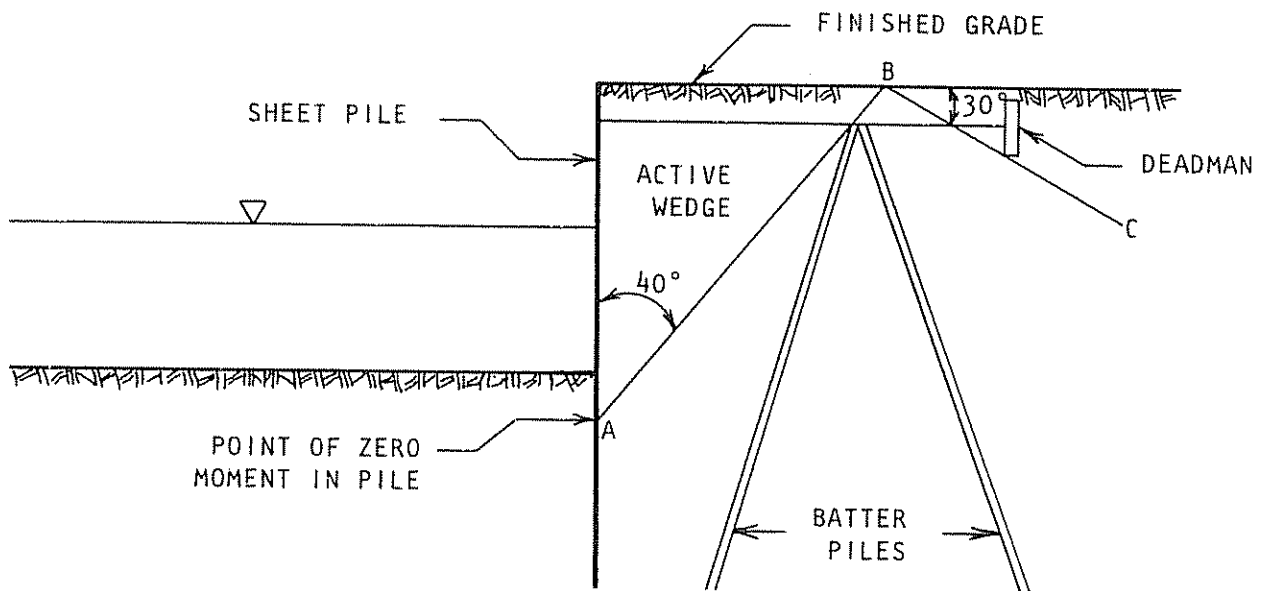
5.3.2 Sheet Pile Walls

The sheet pile walls with level backfill should be designed to resist the lateral pressures imposed by the soils and the ground water as shown on the following page. Sheet piles should extend at least two feet into the dense sand and gravel (see Plate 1 for elevations of dense sand and gravel at the locations explored.) If sloping backfill is used, the pressures would be greater; we can provide the necessary data when the configuration is known.



It may be desired to restrain the upper portion of the sheeting by using tie-rods. The tie-rods should connect to either deadmen or batter piles. The locations of the anchor deadmen or batter piles are illustrated below. The passive resistance of the deadmen may be assumed to be equal to that developed by a fluid with a density of 250 pounds per cubic foot. A coefficient of friction of 0.4 may be used between the deadman and the supporting soils. Lines A-B

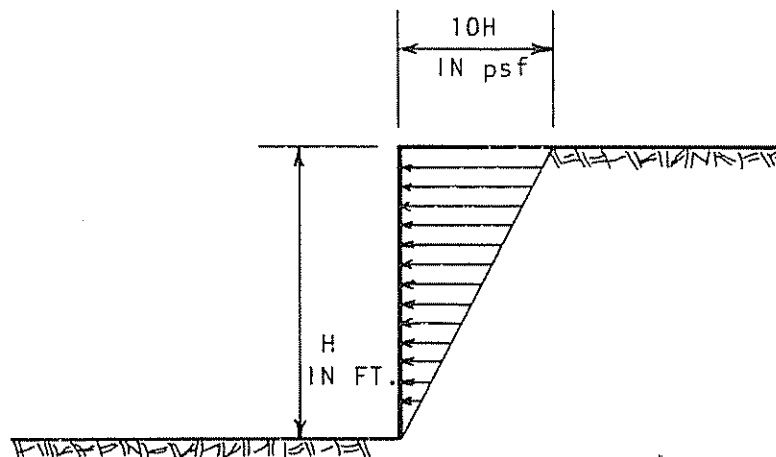
and B-C define the limits of the active and passive wedges, respectively. The entire deadman should be to the right of line B-C. Where a tie-rod is connected to piles, the connection should be to the right of line A-B.



5.3.3 Seismically-Induced Earth Pressure

It is our opinion that the incremental increase in earth pressure due to seismic loading on the walls of the structures will be low and will merely result in a reduction in the factor of safety. If it is desired to maintain the same factor of safety during an

earthquake as under static conditions, we suggest the use of the following seismically induced earth pressure distribution. This pressure distribution would be in addition to the earth pressure and any surcharge.



5.3.4 Backfill

All required backfill for walls below grade should be mechanically compacted in layers not more than eight inches in thickness to at least 90% of the maximum density obtainable by the ASTM Designation D1557-70 method of compaction. Proper compaction of the backfill will be necessary to reduce settlement of the backfill and consequent settlement of overlying slabs and paving. In order to minimize the potential for development of differential hydrostatic pressures on the walls, the backfill should consist of granular, free-draining soils. It may be possible to use the on-site silty sand soils; the silt soils should not be used.

Even at 90% compaction, some settlement may occur within the backfill. Accordingly, provisions should be made for some possible settlement of overlying slabs and paving. Also, we suggest that any utility lines partially supported on the backfill and crossing the walls be designed to accept differential settlement, particularly at the points of entry through the walls.

5.4 GRADING

5.4.1 Excavation

5.4.1.1 Cut Slopes

The allowable permanent slope inclination will vary with the height of the slope. The allowable slope inclinations for various slope heights are presented in the following table:

Slope Height <u>(feet)</u>	Slope Inclination <u>(Horizontal to Vertical)</u>
0 to 10	2:1
10 to 20	3:1
20 to 30	4:1

We recommend that combinations of slopes with gravity retaining walls above not be used. It would be possible to use a slope at the toe of a sheet pile wall. We can provide data for design of sheet pile walls with slopes when the desired configuration is known. The slopes should be revetted to protect against erosion

from tide fluctuations, tidal currents, wave action, prop wash, etc. The revetment should be placed as soon as possible after the slopes are excavated.

5.4.1.2 Dredging

As discussed, dredging excavation methods may be needed below water level. This could involve the soils between depths of about 10 and 30 feet. Based on our borings, the soils in this zone are primarily silts. Although the materials are slightly cohesive we believe that they could be excavated using dredging techniques. However, because of their fine-grained nature, sedimentation of these materials will be slow, and drainage of the dredged materials will require relatively long periods of time.

5.4.1.3 Dewatering

Depending on the final design and selected construction methods for the marina perimeter, dewatering of the site will be required. Well points and/or deep wells could be used to achieve proper dewatering. The lowering of the water table will cause compression of the underlying material. Our analyses indicate that $\frac{1}{4}$ to $\frac{1}{2}$ inch of surface subsidence will occur for each foot the ground water surface is lowered. If the ground water is lowered 15 feet, this will result in about 6 inches of subsidence. Our calculations assume that the site has not been previously dewatered. If the site has been previously dewatered, the anticipated settlement would be less.

The extent of the ground water drawdown away from the excavations will vary depending on the method of dewatering used.

If the site is to be dewatered, supplementary studies to determine the required pump rates and anticipated drawdown should be performed.

5.4.2 Compacted Fill

5.4.2.1 General

The site is underlain by existing hydraulic fill, and some dumped fill. This material is not suitable for foundation support. Retaining walls less than about six feet in height could be supported on spread footings in compacted fill if the existing fill soils to depths of three feet below the footings are excavated and replaced as a compacted fill.

Elsewhere, excavation of the upper soils to a depth of about two feet below the existing grade will be required prior to placing any new fill. After making that excavation, the exposed soils should be carefully observed by competent geotechnical personnel to verify removal of unsuitable material. As discussed, a former dump, referred to as the celery dump, existed on the east portion of the site. The feature was not encountered in the borings. If encountered during grading, special handling of these materials may be necessary.

5.4.2.2 Site Preparation

Where spread footings will be used, the on-site soils should be excavated to a depth at least three feet below footing bottoms and five feet beyond the footings in plan. Elsewhere, where new compacted fill is to be placed, the soils should be excavated to at least two feet below existing grade. Next, the exposed soils should be scarified to a depth of six inches and rolled with heavy compaction equipment. The upper six inches of exposed soils should be compacted to at least 90% of the maximum density obtainable by the ASTM D1557-70 method of compaction. The exposed soils may be wet and soft. In this event, the soils should not be scarified, instead a geotextile and a layer of gravel should be placed to provide a stable working surface for men and equipment.

5.4.2.3 Compaction

After compacting the exposed soils or placing the layer of gravel, the required fill should be placed in loose lifts not more than eight inches in thickness and compacted to at least 90%. It is recommended that the soils be compacted at a moisture content varying no more than 2% below or above optimum moisture content.

5.4.2.4 Material For Fill

The on-site silty sand and sandy silt soils, less any debris or organic matter, may be used in the required fills beneath footings. The on-site clayey silt soils should not be used. Any required imported fill should consist of relatively non-expansive soils.

The Expansion Index of the select soil should be less than 35. The imported material should contain sufficient fines (binder material) so as to result in a stable subgrade.

5.4.2.5 Geotechnical Observation

The excavation of the upper soils and the compaction of all required fill should be observed and tested by our firm. Any imported fill material should be approved for use prior to importing.

5.4.2.6 Subsidence

The settlement due to the placement of the existing fill at the site is complete. If additional fill is placed on the site during development of the marina, there will be additional settlement of the areas receiving fill. We estimate that the areal settlement due to the placement of additional fill will be up to about two inches per foot of fill placed. We estimate that 80% of this settlement will occur within about one year after completion of this fill. The rate of settlement can be effectively increased by placing a surcharge fill. We can develop surcharge criteria if necessary.

5.5 FOUNDATIONS FOR STRUCTURES ADJACENT TO MARINA PERIMETER

It is anticipated that structures may be constructed adjacent to the perimeter of the marina in the future. Major structures adjacent to the perimeter of the marina should be supported on piling.

6.0 BIBLIOGRAPHY

- Association of Engineering Geologists, 1973, "Geology and Earthquake Hazards Planners Guide to the Seismic Safety Element," Special Publication.
- Association of Engineering Geologists, 1973, "Geology, Seismicity, and Environmental Impact," Special Publication.
- California Department of Public Works, 1952, "West Coast Basin Reference," Draft of Report of Referee, Case No. 506806.
- California Department of Water Resources, 1961, Bulletin 104, "Planned Utilization of the Ground Water Basins of the Coastal Plain of Los Angeles County," App. A, Ground Water Geology. California Department of Water Resources, 1964, "Bulletin 116-2, "Crustal Strain and Fault Movement Investigation."
- California Department of Water Resources, 1977, "Bulletin 130-75, "Hydrologic Data: 1975, Vol. II, Southern California.
- California Division of Mines and Geology, 1954, "Geology of Southern California," Bulletin 170.
- California Division of Mines and Geology, 1974, "A Review of the Geology and Earthquake History of the Newport-Inglewood Structural Zone, Southern California," Special Report 114.
- California Division of Oil and Gas, 1974, "California Oil and Gas Fields, Volume II", Report No. TR12.
- California Division of Oil and Gas, 1986, "Playa del Rey Field," Map 120.
- California Division of Oil and Gas, 1988, "73rd Annual Report of the State Oil and Gas Supervisor."
- Castle, R.O., and Yerkes, R.F., 1976, "Recent Subsurface Movements in the Baldwin Hills, Los Angeles, California," U.S. Geological Survey Professional Paper 882.
- Converse, Ward, Davis, Dixon, 1981, "Comprehensive Geotechnical Report, Playa Vista Parcel, Marina del Rey Area."

- Federal Emergency Management Agency, 1985, "Flood Insurance Rate Map, Los Angeles County, California," Community-Panel Number 065043 0905C.
- Greensfelder, R.W., 1974, "Maximum Credible Rock Acceleration from Earthquakes in California," California Division of Mines and Geology Map Sheet 23.
- Houston, J.R., and Garcia, A.W., 1974, "Type 16 Flood Insurance Study: Tsunami Predictions for Pacific Coastal Communities," Vickburg, U.S. Army Waterways Experiment Station.
- Los Angeles, City of, 1974, "Seismic Safety Plan."
- Los Angeles County Department of Regional Planning, 1982, "Local Coastal Plan, Phase II - Land Use Plan, Marina del Rey/Ballona," Los Angeles County, California.
- Los Angeles County, 1974, "Seismic Safety Element, Los Angeles County General Plan."
- Richter, C.F., 1958, "Elementary Seismology," W.H. Freeman & Company.
- Poland, J.F., Garrett, A.A., and Sinnott, A., 1959, "Geology, Hydrology and Chemical Character of Ground Waters in the Torrance-Santa Monica Area, California," U.S. Geological Survey, Water Supply Paper 1461.
- U.S. Geological Survey, 1965, "Venice 7.5' Quadrangle Map," photorevised 1981.
- Yerkes, R.F., McColloch, T.H., Schoellhammer, J.E., and Vedder, J.G., 1969, "Geology of the Los Angeles Basin, an Introduction," U.S. Geological Survey, Professional Paper 420-A.



CHKD

dmh

W.P.

O.E. BK

dmh

DR.

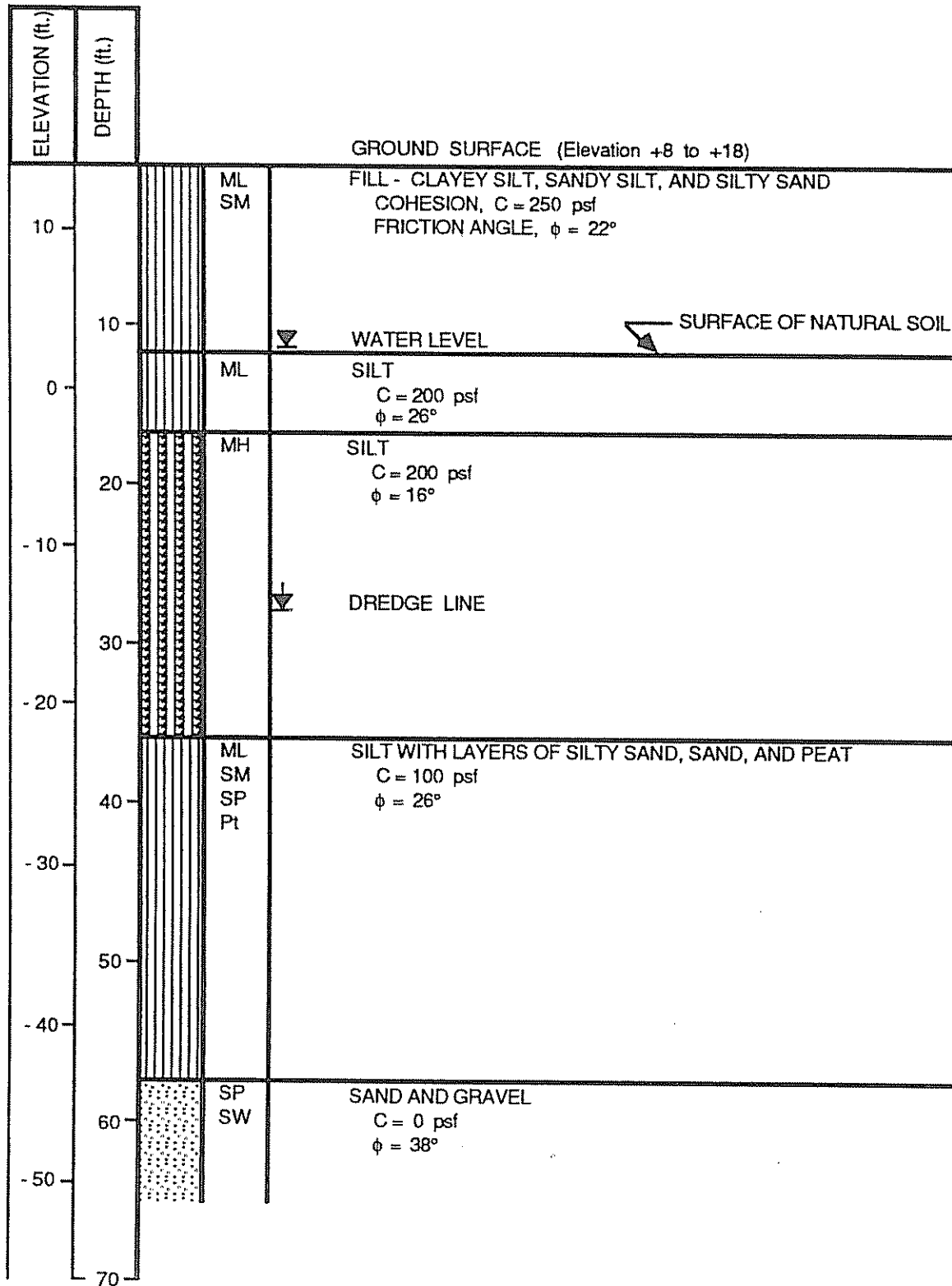
F.T.

DATE

3/20/89

AE-88473

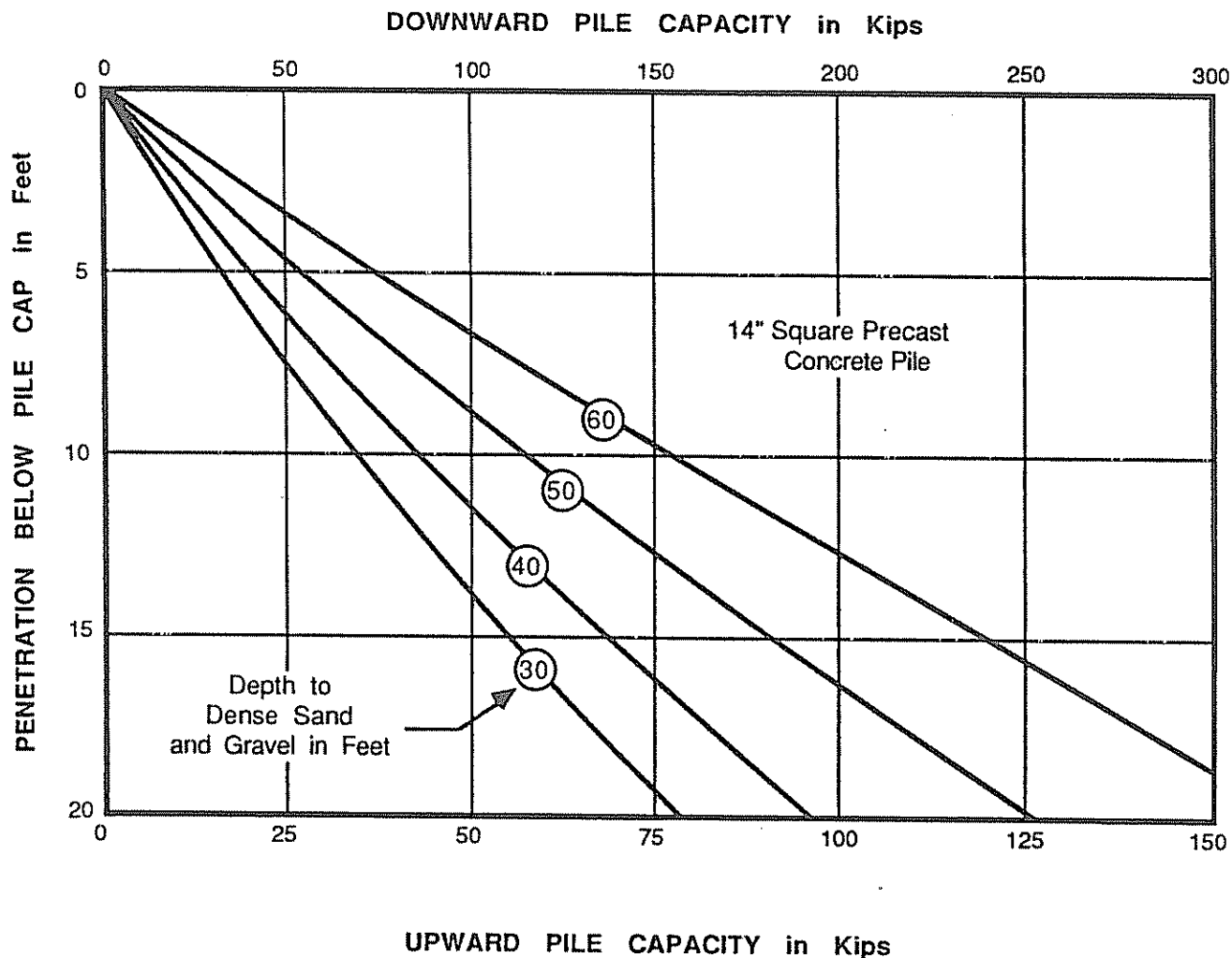
AE-88473



GENERALIZED SOIL PROFILE

LEROY CRANDALL AND ASSOCIATES

PLATE 3



NOTES :

- (1) The indicated values refer to the total of dead plus live loads; a one-third increase may be used when considering wind or seismic loads.
- (2) Piles in groups should be spaced a minimum of 3 feet on centers.
- (3) The indicated values are based on the strength of the soils; the actual pile capacities may be limited to lesser values by the strength of the piles.

DRIVEN PILE CAPACITIES

CHKD

BK

O.E.

dmh

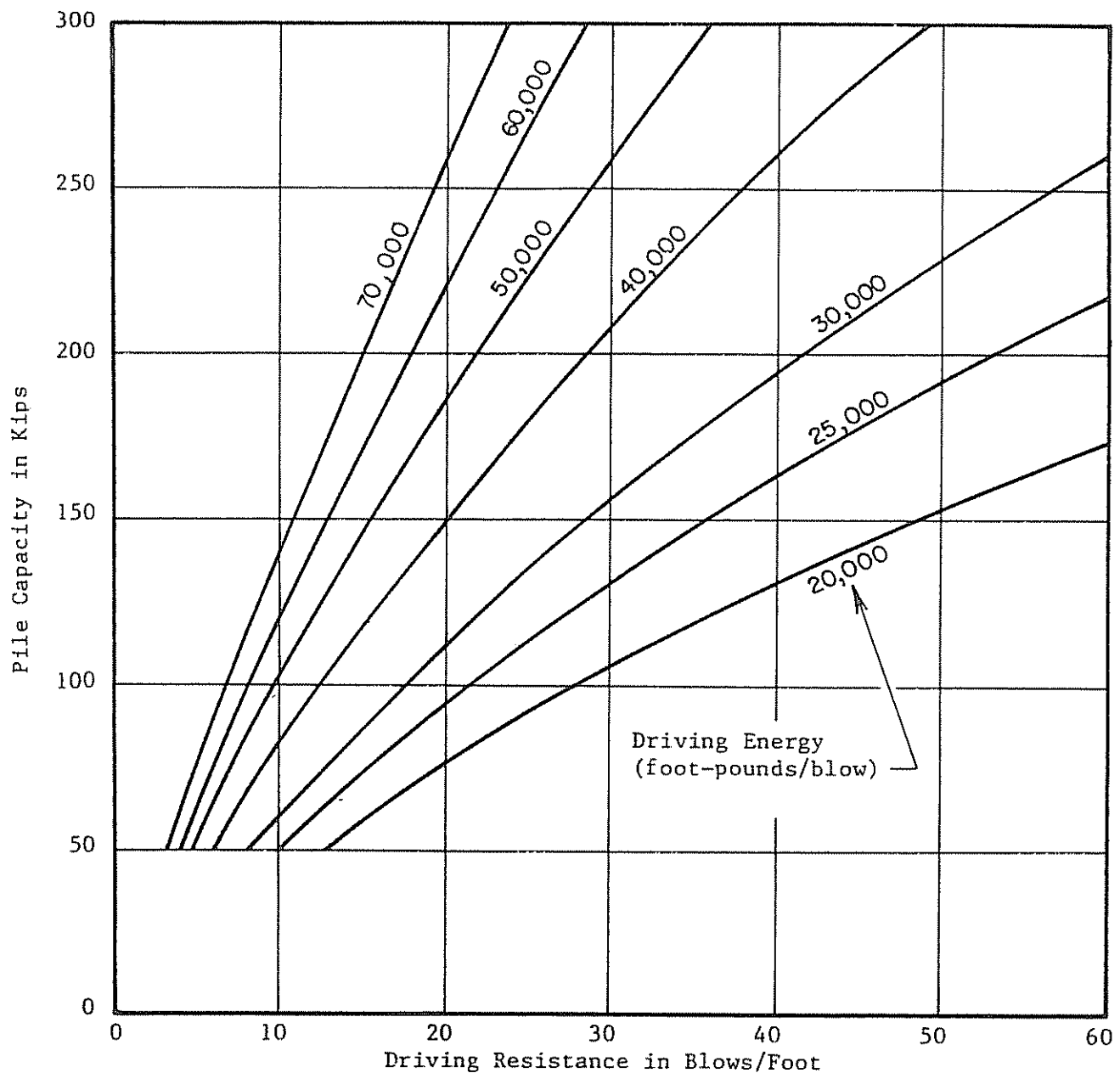
W.P.

3/31/89

DATE

AE-88473

JOB



NOTES:

- 1) The above driving resistance should be obtained for the last foot of driving at design pile length. If the driving resistance at the design length is less than above, the piles should be lengthened until the desired driving resistance is obtained.
- 2) As an alternate to lengthening when low driving resistance is obtained, the piles may be allowed to set overnight and the number of blows to drive the pile one inch the following day should be determined. If the restarting resistance is at least two times the above criteria, the pile may be considered satisfactory.
- 3) If driving resistance of three times the above criteria is encountered within five feet of design length, the pile driving may be stopped.

PILE DRIVING CRITERIA

LeROY CRANDALL AND ASSOCIATES

FORM 42 JUL 1964



APPENDIX A
EXPLORATIONS

The soil conditions beneath the site were explored by drilling 20 borings at the locations shown on Plate 1. Five cone penetration tests were performed to supplement the exploration borings. The cone penetration test results are presented in Appendix B.

Borings 1 through 7, 11, 14, and 16 through 20 were drilled to depths of about 60 feet below the existing grade using 5-inch-diameter rotary wash-type drilling equipment with drilling mud to prevent caving. The mud was removed from some of the borings following completion of the drilling to permit measurement of the water level. Borings 8, 9, 19, 12, 13, and 15 were drilled to depths of between 16 and 20 feet below the existing grade using 16- and 18-inch bucket-type drilling equipment. Bucket-type borings were also drilled to depths of between 11½ and 16 feet below the existing grade adjacent to Borings 1, 6, 16, and 18 to provide information on the water levels. Drilling mud or casing was not used to extend the bucket borings to the depths drilled. Caving and squeezing in of the boring walls occurred during drilling as noted on the boring logs.

The soils encountered were logged by our field technician, and undisturbed and loose samples were obtained for laboratory inspection and testing. The logs of the borings are presented on Plates A-1.1 through A-1.20; the depths at which undisturbed samples were obtained are indicated to the left of the boring logs. The energy required to drive the sampler twelve inches is indicated on the logs. Standard penetration tests were performed in selected borings; the results are indicated on the boring logs. The soils are classified in accordance with the Unified Soil Classification System described on Plate A-2.

LABORATORY TESTS

The field moisture content and dry density of the soils encountered were determined by performing tests on the undisturbed samples. The results of the tests are shown to the left of the boring logs.

The liquid limit and plasticity index of selected samples were determined. In order to assess whether or not the soils were organic, the liquid limit test was repeated on some of the samples after they were oven dried. The results of the tests are presented on the boring logs.

Direct shear tests were performed on selected undisturbed samples to determine the strength of the soils. The tests were performed at field and increased moisture contents and at various different surcharge pressures. Tests were also performed on remolded samples compacted to 90%. The yield-point values determined from the direct shear tests are presented on Plates A-3.1 and A-3.2, Direct Shear Test Data.

Confined consolidation tests were performed on 23 undisturbed samples to determine the compressibility of the soils. The samples were tested at field moisture content. The results of the tests are presented on Plates A-4.1 through A-4.12, Consolidation Test Data.

To determine the particle size distribution of the soils and to aid in classification, mechanical and hydrometer analyses were performed on six samples. The results of the analyses are presented on Plates A-5.1 through A-5.3, Particle Size Distribution.

The optimum moisture content and maximum dry density of the soils were determined by performing compaction tests on samples from Borings 8 and 9. The tests were performed in accordance with the ASTM Designation

D1557-70 method of compaction. The results of the tests are presented on Plate A-6, Compaction Test Data.

Permeability tests were performed on ten undisturbed samples to determine the coefficient of permeability of the soils. The samples were tested under confining pressures of between 200 and 1,000 pounds per square foot. The test results are presented on Plate A-7, Permeability Test Data.

Soil corrosivity tests were performed on eleven samples. The tests were performed for us by M.J. Schiff & Associates. The results are presented on Plates A-8.1 through A-8.6.

-oOo-

JOB AE-88473

DATE 1/19/89

F.T.

FH

DR.

dmh

O.E. JC

W.P.

dmh

CHKD

BK

Note: The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated.
It is not warranted to be representative of subsurface conditions at other locations and times.

							BORING 1	
							DATE DRILLED: January 5, 1989	
							EQUIPMENT USED: 5" - Diameter Rotary Wash	
							ELEVATION 14.9'	
ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.		
							SM	FILL - SILTY SAND - fine, brown
							ML	FILL - CLAYEY SILT - dark grey
10	5		17.8	91	9		SM	FILL - SILTY SAND - fine, lenses of Clayey Silt, brownish grey
			22.2	101	17			Some seashells
5	10	6					ML	FILL - SANDY SILT - lenses of Clayey Silt, some seashells, grey
			41.2	77	1		ML	↓ SURFACE OF NATURAL SOIL CLAYEY SILT - some seashells, grey
0	15	6						
			47.3	74	3			
-5	20						ML	SANDY SILT - grey
		11						
-10	25		62.4	60	2		MH	CLAYEY SILT - dark grey (LL = 68; PI = 21)
		2						
-15	30		79.9	53	2			Organic odor
		3						
-20	35		59.4	65	2			
		18					ML	CLAYEY SILT - brownish grey
-25	40							

(CONTINUED ON FOLLOWING PLATE)

LOG OF BORING

LeROY CRANDALL AND ASSOCIATES

PLATE A - 1.1a

3 AE-88473

DATE 1/19/89

F.T. FH

DR. dmh

O.E. JC

W.P. dmh

CHKD

BK

Note : The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated. It is not warranted to be representative of subsurface conditions at other locations and times.

ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.
-30	45	9	31.5	89	3	
-35	50		29.0	88	3	
-40	55		21.6	106	18	
-45	60		34.2	87	6	

DATE DRILLED: January 5, 1989
EQUIPMENT USED: 5" - Diameter Rotary Wash

Bluish grey

SILTY SAND - fine, bluish grey

CLAYEY SILT - dark grey

NOTE: Drilling mud used in drilling process. Boring grouted with cement-bentonite mixture. Water level not established.

Drilled 18" - Diameter Bucket Boring 5' south of Rotary Wash Boring to a depth of 11-1/2' on 1/12/89 for water level determination. Water seepage encountered at a depth of 9-1/2'. Caving below 9-1/2' (to 2' in diameter).

LOG OF BORING

LeROY CRANDALL AND ASSOCIATES

PLATE A - 1.1b

Note : The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated.
It is not warranted to be representative of subsurface conditions at other locations and times.

							BORING 2	
ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.	DATE DRILLED: January 9, 1989 EQUIPMENT USED: 5" - Diameter Rotary Wash	
							ELEVATION	14.1
							ML	FILL - SANDY SILT - some seashells, brown Large amount of seashells, greyish brown
10	5		37.8	80	2			
			12.9	93	4		SM	FILL - SILTY SAND - fine, light brown
			59.3	66	1			Layers of Sandy Silt, some seashells, dark grey
5			26.6	99	4			Grey
	10		43.6	76	< 1		ML	FILL - CLAYEY SILT - grey (LL = 43; PI = 10; oven dry LL = 39)
0			34.0	88	3		ML	SURFACE OF NATURAL SOIL SANDY SILT - slightly Clayey, brown and grey
	15							Some seashells
-5	20							Large amount of seashells
							MH	CLAYEY SILT - organic odor, grey
-10	25		77.9	55	1			
-15	30		48.4	71	2			Dark grey
-20	35		54.3	69	2			
-25							ML	CLAYEY SILT - dark grey (LL = 39; PI = 5)
	40		35.4	85	4			

(CONTINUED ON FOLLOWING PLATE)

LOG OF BORING

LeROY CRANDALL AND ASSOCIATES

PLATE A - 1.2a

B AE-88473

DATE 1/19/89

F.T.

BG

DR.

dmh

O.E. JC

W.P.

dmh

CHKD

BK

Note: The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated.
It is not warranted to be representative of subsurface conditions at other locations and times.

BORING 2 (Continued)

DATE DRILLED: January 9, 1989
EQUIPMENT USED: 5" - Diameter Rotary Wash

ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD.PEN.TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.
-30	45		33.8	86	4	
-35	50		52.3	65	3	
-40	55		19.1	111	18	ML
						SM
-45	60		19.5	104	54	SP

Layer of Peat

Traces of Peat

SANDY SILT - bluish grey

SILTY SAND - fine, some Gravel, traces of Peat, bluish grey

SAND - fine to medium, few Gravel and Cobbles, greyish brown

NOTE: Drilling mud used in drilling process. Mud removed to a depth of 35' at completion of drilling. Water level measured at 7' after removal of mud. Boring grouted with cement-bentonite mixture.

LOG OF BORING

LEROY CRANDALL AND ASSOCIATES

PLATE A - 1.2b

JOB AE-88473

DATE 1/19/89

F.T. FH

DR. 3mh

O.E. JC

W.P. dmh

CHKD

Bk

Note: The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated.
It is not warranted to be representative of subsurface conditions at other locations and times.

							BORING 3	
							DATE DRILLED: January 9, 1989	
							EQUIPMENT USED: 5" - Diameter Rotary Wash	
							ELEVATION 13.4	
ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-klps/ft.)	SAMPLE LOC.		
10	5		4.3	93	5	SM	FILL - SILTY SAND - fine, light brown	
						ML	FILL - CLAYEY SILT - brownish grey	
5			16.0	90	2	SM	FILL - SILTY SAND - fine, light brown	
						ML	FILL - CLAYEY SILT - dark grey	
10			45.8	76	<1			
0		6				ML	SURFACE OF NATURAL SOIL SANDY SILT - grey	
15			54.0	71	1		Layer of Clayey Silt with seashells	
-5						ML	CLAYEY SILT - greyish brown	
20		5				ML	SANDY SILT - some seashells, dark grey	
			37.6	83	2			
-10						MH	CLAYEY SILT - organic odor, dark grey	
25		2						
			77.5	53	1			
-15								
30		4						
-20			50.2	73	1			
35							Layer of Silty Sand	
						ML	CLAYEY SILT - dark grey	
-25		9						
40			32.8	90	2			

(CONTINUED ON FOLLOWING PLATE)

LOG OF BORING

LeROY CRANDALL AND ASSOCIATES

PLATE A - 1.3a

AE-88473

DATE 1/19/89

F.T.

FH

DR.

dmh

O.E. JC

W.P.

dmh

CHKD

BY

Note : The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated.
It is not warranted to be representative of subsurface conditions at other locations and times.

ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.
-30		9				
-45			34.8	87	3	
-50		5				
-55			19.5	110	11	
-60			17.8	112	32	

DATE DRILLED: January 9, 1989
EQUIPMENT USED: 5" - Diameter Rotary Wash

Layer of Peat

SILTY SAND - fine to medium, some Gravel, traces of
Peat, grey
SAND - medium to coarse, about 15% Gravel, light grey

NOTE: Drilling mud used in drilling process. Boring grouted with
cement-bentonite mixture. Water level not established.

LOG OF BORING

LeROY CRANDALL AND ASSOCIATES

PLATE A - 1.3b

By

CHKD

dmh

W.P.

O.E.

dmh

DR.

FH

F.T.

DATE

AE-88473

JOB

Note : The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated.
It is not warranted to be representative of subsurface conditions at other locations and times.

							BORING 4	
							DATE DRILLED: January 9, 1989	
							EQUIPMENT USED: 5" - Diameter Rotary Wash	
							ELEVATION 12.1	
ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.		
10		14.4	91	3		SM	FILL - SILTY SAND - fine, lenses of Clayey Silt, brown and grey	
5		35.3	84	1		MH	FILL - CLAYEY SILT - dark grey	
5		63.0	63	< 1			Some rootlets	
10		39.5	82	< 1		ML	FILL - CLAYEY SILT - brown	
0		33.6	88	4		ML	SURFACE OF NATURAL SOIL CLAYEY SILT - lenses of Sandy Silt, light grey	
15		37.6	84	< 1		SM	SILTY SAND - fine, light brown	
-5		37.9	84	1			Grey	
20		85.8	49	1		OH	CLAYEY SILT - some seashells, organic odor, dark grey (LL = 109; PI = 30; oven dry LL = 68)	
-15		46.5	71	1			About 20% seashells, brownish grey	
30		23.5	104	3		ML	CLAYEY SILT - some seashells, bluish grey	
-20		35.5	88	3			Dark gray	
35								
-25								
40								

(CONTINUED ON FOLLOWING PLATE)

LOG OF BORING

LeROY CRANDALL AND ASSOCIATES

PLATE A - 1.4a

3 AE-88473

DATE 1/19/89

F.T. FH

DR. dmh

W.P. dmh

O.E. JC

CHKD

BYE

Note : The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated. It is not warranted to be representative of subsurface conditions at other locations and times.

BORING 4 (Continued)

DATE DRILLED: January 9, 1989
EQUIPMENT USED: 5" - Diameter Rotary Wash

ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD.PEN.TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.
-30						
-45			81.3	49	3	PT
-35						ML
-50			22.7	104	5	
-40						SP
-55			12.5	129	40	
-45						
-60			8.2	137	50	

Traces of Peat
PEAT - brown
CLAYEY SILT - traces of Peat, dark grey
SAND - fine to coarse, about 30% Gravel and Cobbles, grey

NOTE: Drilling mud used in drilling process. Boring grouted with cement-bentonite mixture. Water level not established.

LOG OF BORING

bk

JOB AE-88473 DATE 1/20/89 F.T. BG DR. dmin O.E. JC W.P. dmh CHKD

BORING 5

DATE DRILLED: January 9, 1989
EQUIPMENT USED: 5" - Diameter Rotary Wash

ELEVATION 10.9

Note : The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated.
It is not warranted to be representative of subsurface conditions at other locations and times.

ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.	
10							ML FILL - SANDY SILT - lenses of Clayey Silt, greyish brown and dark grey
	5		48.7	73	1		SM FILL - SILTY SAND - fine, light brown
5			57.5	66	< 1		MH FILL - CLAYEY SILT - dark grey (LL = 50; PI = 13)
							↓ SURFACE OF NATURAL SOIL
	10		36.8	84	4		ML CLAYEY SILT - traces of organic matter, dark grey
0		5					Some Sand
	15		37.8	84	1		SM SILTY SAND - fine, brownish grey
- 5							MH CLAYEY SILT - organic odor, bluish grey
	20	1					
- 10			56.6	66	1		Few shells (LL = 68; PI = 24)
	25						ML CLAYEY SILT - dark grey
- 15		14					
	30		25.1	101	5		Lenses of Sandy Silt
- 20							
	35	24					SP SAND - fine to coarse, few Gravel, bluish grey
- 25							ML CLAYEY SILT - bluish grey
40							

(CONTINUED ON FOLLOWING PLATE)

LOG OF BORING

AE-88473

DATE

1/20/89

F.T.

BG

DR.

dmh

O.E.

JC

W.P.

dmh

CHKD

Note : The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated.
It is not warranted to be representative of subsurface conditions at other locations and times.

ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD.PEN.TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.
-30			29.2	89	6	
-35	45	8				
-40	50	53	17.7	114	54	
-45	55		9.4	128	39	
-50	60					
-55	65					

DATE DRILLED: January 9, 1989
EQUIPMENT USED: 5" - Diameter Rotary Wash

Traces of Peat

SAND - fine to coarse, some Silt, some Gravel and
Cobbles, grey

Large amount of Gravel and Cobbles

NOTE: Drilling mud used in drilling process. Boring grouted with
cement-bentonite mixture. Water level not established.

LOG OF BORING

LeROY CRANDALL AND ASSOCIATES

PLATE A - 1.5b

JOB AE-88473 DATE 1/20/89 F.T. FH DR. dmh O.E. JC W.P. dmh CHKD

BORING 6

DATE DRILLED: January 10, 1989
 EQUIPMENT USED: 5" - Diameter Rotary Wash
 ELEVATION 8.1

Note: The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated.
 It is not warranted to be representative of subsurface conditions at other locations and times.

ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.	
							ML
	5		39.6	83	2		FILL - CLAYEY SILT - lenses of Sandy Silt, dark grey
	5		53.7	70	< 1		
	0		50.7	70	< 1		
	10		42.2	80	< 1		↓ SURFACE OF NATURAL SOIL
	-5		52.2	69	1		ML CLAYEY SILT - grey
	-5						ML SANDY SILT - brownish grey
	-15						MH CLAYEY SILT - some seashells, bluish grey
	-10		70.5	57	< 1		Dark grey
	-20		70.4	57	< 1		
	-15						ML SANDY SILT - layers of Clayey Silt, bluish grey
	-25		23.5	103	2		
	-30		20.9	107	2		
	-25						SM SILTY SAND - fine, bluish grey
	-35		16.5	114	11		ML SANDY SILT - bluish grey
	-30						PT PEAT - brown to dark brown
	-40		59.7	64	2		ML CLAYEY SILT - large amount of Peat, dark brown

(CONTINUED ON FOLLOWING PLATE)

LOG OF BORING

AE-88473

DATE

1/20/89

F.T.

FH

DR.

dmh

O.E.

JC

W.P.

dmh

CHKD

BK

Note: The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated. It is not warranted to be representative of subsurface conditions at other locations and times.

ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD.PEN.TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.
-35						
-45			20.2	108	14	SM
-40						SP
-50			11.3	126	32	SW
-45						
-55			10.0	130	36	
-50						
-60						

BORING 6 (Continued)

DATE DRILLED: January 10, 1989
EQUIPMENT USED: 5" - Diameter Rotary Wash

SILTY SAND - fine to medium, bluish grey

SAND - fine, grey

SAND - well graded, about 20% Gravel, grey

NOTE: Drilling mud used in drilling process. Boring grouted with cement-bentonite mixture. Water level not established.

Drilled 18" - Diameter Bucket Boring 5' south of Rotary Wash Boring to a depth of 12-1/2' on 1/12/89 for water level determination. Water seepage encountered at a depth of 11'. Hole squeezing in below 11'.

LOG OF BORING

LeROY CRANDALL AND ASSOCIATES

PLATE A - 1.6b

JOB AE-88473

DATE

1/20/89

F.T.

BG

DR.

dmh

O.E.

JC

W.P.

dmh

CHKD

BK

Note : The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated.
It is not warranted to be representative of subsurface conditions at other locations and times.

ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.
10			20.3	96	6	
	5		29.0	88	2	
	5		37.6	86	1	
	10		46.5	76	1	
	0		31.9	83	1	
	15		35.0	87	4	
	-5		32.6	87	1	
	20		80.1	53	1	
	-10					
	25		78.0	54	2	
	-15					
	30		36.6	84	2	
	-20					
	35		46.6	72	2	
	-25					
	40		33.6	88	7	

DATE DRILLED: January 5, 1989
EQUIPMENT USED: 5" - Diameter Rotary Wash

ELEVATION 12.3

BORING 7

ML

FILL - SANDY SILT - brown

Lenses of Clayey Silt
Some seashells, brown and grey

ML

FILL - CLAYEY SILT - some Sand, light grey
Large amount of seashells

Brownish grey

ML

↓ SURFACE OF NATURAL SOIL
CLAYEY SILT - lenses of Sandy Silt, light grey

MH

CLAYEY SILT - some organic matter, light grey

Bluish grey

(CONTINUED ON FOLLOWING PLATE)

LOG OF BORING

LeROY CRANDALL AND ASSOCIATES

PLATE A - 1.7a

B AE-88473

DATE 1/20/89

F.T. BG

DR. dmh

O.E. JC

W.P. dmh

CHKD

BK

Note : The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated.
It is not warranted to be representative of subsurface conditions at other locations and times.

ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD.PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.
-30						
	45		36.8	81	5	
-35						
	50		23.8	103	10	
-40						ML
	55		26.6	96	6	
-45						ML
	60		22.5	102	40	SP

BORING 7 (Continued)

DATE DRILLED: January 5, 1989
EQUIPMENT USED: 5" - Diameter Rotary Wash

(LL = 56; PI = 23)

SANDY SILT - few seashells, bluish grey

Black
CLAYEY SILT - grey

SAND - fine to medium, grey

NOTE: Drilling mud used in drilling process. Mud removed to a depth of 60' at completion of drilling. Water level measured at 7' after removal of mud. Boring grouted with cement-bentonite mixture.

LOG OF BORING

LeROY CRANDALL AND ASSOCIATES

PLATE A - 1.7b

JOB AE-88473

DATE 1/20/89

F.T.

TC

DR.

dmh

O.E.

JC

W.P.

dmh

CHKD

BK

Note: The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated.
It is not warranted to be representative of subsurface conditions at other locations and times.

ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.
10			24.8	92	< 1	
5			33.8	89	< 1	
5			57.4	70	< 1	
10			48.1	73	< 1	
0						
15			35.8	86	< 1	
5						
20						

DATE DRILLED: January 11, 1989
EQUIPMENT USED: 16" - Diameter Bucket

ELEVATION 12.9

BORING 8

ML
FILL - SANDY SILT - brownish grey
Some lumps of Clayey Silt
ML
FILL - CLAYEY SILT - grey
(LL = 37; PI = 8)
MH
FILL - CLAYEY SILT - bluish grey
Brownish grey
SURFACE OF NATURAL SOIL
CLAYEY SILT - light brown

NOTE: Water seepage encountered at 13'. Water level measured at 15' at completion of drilling and at 13' 20 minutes later. Caving below 13'.

LOG OF BORING

LeROY CRANDALL AND ASSOCIATES

PLATE A - 1.8

B AE-88473

DATE 1/20/89

F.T. TC

DR.

dmh

O.E. JC

W.P.

dmh

CHKD

Note: The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated. It is not warranted to be representative of subsurface conditions at other locations and times.

							BORING 9	
							DATE DRILLED: January 11, 1989	
							EQUIPMENT USED: 16" - Diameter Bucket	
							ELEVATION 12.0	
ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.		
10			19.2	96	< 1	SM	FILL - SILTY SAND - fine, light brown	
			41.6	82	< 1	ML	FILL - SANDY SILT - greyish brown	
						ML	FILL - CLAYEY SILT - dark grey	
5			64.0	62	< 1	MH	FILL - CLAYEY SILT - dark grey	
5			47.9	72	< 1			
10			55.6	68	< 1	ML	SURFACE OF NATURAL SOIL CLAYEY SILT - some Sand, some cemented nodules, dark brown	
0								
15								
-5								
20								

NOTE: Water seepage encountered at 13'-1/2'. Water level measured at 13' 20 minutes after completion of drilling. Caving below 12'. Hole squeezed in below 14' after drilling to 17'.

LOG OF BORING

LeROY CRANDALL AND ASSOCIATES

PLATE A - 1.9

JOB AE-88473

DATE 1/20/89

F.T. TC

DR. dmh

O.E. JC

W.P. dmh

CHKD

BK

Note : The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated.
It is not warranted to be representative of subsurface conditions at other locations and times.

ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD.PEN.TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.
10			32.9	89	< 1	ML
			62.5	61	< 1	MH
5	5		83.5	53	< 1	
			32.7	86	< 1	ML
0	10		35.0	88	< 1	ML
-5	15		37.4	87	< 1	ML
	20					

BORING 10

DATE DRILLED: January 11, 1989
EQUIPMENT USED: 16" - Diameter Bucket

ELEVATION 10.6

FILL - SANDY SILT - brown

FILL - CLAYEY SILT - bluish grey and brown

Some organic matter

Lenses of Sandy Silt

FILL - CLAYEY SILT - brownish grey

↓ SURFACE OF NATURAL SOIL
SANDY SILT - brownish grey

CLAYEY SILT - bluish grey

NOTE: Water seepage encountered at a depth of 11'. Caving
below 11'. Hole squeezed in below 12-1/2' after drilling
to 16'.

LOG OF BORING

LeROY CRANDALL AND ASSOCIATES

PLATE A - 1.10

BK

CHKD

dmh

W.P.

JC

O.E.

dmh

DR.

FH

F.T.

1/20/89

DATE

AE-88473

JOB

BORING 11

DATE DRILLED: January 10, 1989
EQUIPMENT USED: 5" - Diameter Rotary Wash

ELEVATION 10.3

Note: The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated. It is not warranted to be representative of subsurface conditions at other locations and times.

ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.	
10						ML	FILL - CLAYEY SILT - greyish brown
			14.2	94	4		
5	5					SM	FILL - SILTY SAND - fine, greyish brown
			39.6	83	4	ML	FILL - CLAYEY SILT - some organic matter, brown
						ML	FILL - SANDY SILT - light grey
0	10		41.7	83	2		↓ SURFACE OF NATURAL SOIL
		3				MH	CLAYEY SILT - traces of organic matter, light greyish brown
							Greyish brown
-5	15		48.1	74	< 1		Some seashells, organic odor, grey
		0					
							Bluish grey
-10	20		77.6	52	< 1		
		5					
-15	25		19.1	109	1		Some Sand, dark grey
		33				SP SM	SAND - fine to coarse, some Silt, about 10% Gravel, dark grey
-20	30		25.7	99	7	ML	SANDY SILT - bluish grey
		14				MH	CLAYEY SILT - bluish grey
-25	35						
40			46.5	75	2		

(LL = 67; PI = 32; oven dry LL = 62)

(CONTINUED ON FOLLOWING PLATE)

LOG OF BORING

LeROY CRANDALL AND ASSOCIATES

PLATE A - 1.11a

JOB AE-88473

DATE 1/20/89

F.T.

FH

DR.

dmh

O.E. JC

W.P.

dmh

CHKD

Note: The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated.
It is not warranted to be representative of subsurface conditions at other locations and times.

ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-klps/ft.)	SAMPLE LOC.
-30		11				
-35	45		26.6	97	7	
-40	50	58				
-45	55		7.6	119	36	
60			21.9	105	28	

BORING 11 (Continued)

DATE DRILLED: January 10, 1989
EQUIPMENT USED: 5" - Diameter Rotary Wash

Traces of Peat

Some Sand

SAND - fine to medium, grey

SAND - well graded, about 30% Gravel, grey

SAND - fine to medium, some Gravel, light grey

NOTE: Drilling mud used in drilling process. Boring grouted with cement-bentonite mixture. Water level not established.

LOG OF BORING

LeROY CRANDALL AND ASSOCIATES

PLATE A - 1.11b

Note : The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated.
It is not warranted to be representative of subsurface conditions at other locations and times.

							BORING 12	
							DATE DRILLED: January 12, 1989	
							EQUIPMENT USED: 18" - Diameter Bucket	
							ELEVATION 13.2	
ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.		
10			20.1	85	2		ML	FILL - CLAYEY SILT - layers of Sandy Silt, light brown and brown
	5		30.3	83	< 1		ML	Some organic matter FILL - SANDY SILT - layers of Clayey Silt, brown and dark grey
			22.7	91	< 1			
	5		27.7	87	< 1			
							MH	FILL - CLAYEY SILT - dark grey
	10		45.6	75	< 1			
	0							
	15		53.4	69	< 1		MH	SURFACE OF NATURAL SOIL CLAYEY SILT - some Sand, few seashells, brownish grey
	-5							
20			40.2	81	< 1			Bluish grey

NOTE: Water seepage encountered at a depth of 13'. Water level measured at 16' 10 minutes after completion of drilling. Caving below 13' (to 2' in diameter).

LOG OF BORING

LeROY CRANDALL AND ASSOCIATES

PLATE A - 1.12

BK

JOB AE-88473

DATE 1/20/89

三三

五

DR.

dmb

O.E. JC

W.P.

dmh

CHKD

29

Note : The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated. It is not warranted to be representative of subsurface conditions at other locations and times.

ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.
10			39.4	83	< 1	
			21.3	83	< 1	
5	5		62.5	65	< 1	
			35.8	87	< 1	
0	10		34.0	87	< 1	
-5	15		30.5	92	1	
-20	20					

DATE DRILLED: January 12, 1989
EQUIPMENT USED: 18" - Diameter Bucket

ELEVATION 11.6

BORING 13

ML	FILL - SANDY SILT - light brown
ML	FILL - CLAYEY SILT - bluish grey
ML	FILL - SANDY SILT - light brown
MH	FILL - CLAYEY SILT - bluish grey
	Layers of Silty Sand and Sandy Silt, grey and dark grey
	↓ SURFACE OF NATURAL SOIL
ML	CLAYEY SILT - some organic matter, bluish grey
	Brownish grey
MH	CLAYEY SILT - bluish grey

NOTE: Water seepage encountered at a depth of 12'. Water level measured at 14-1/2' 10 minutes after completion of drilling. Caving below 12' (to 2' in diameter).

LOG OF BORING

LeROY CRANDALL AND ASSOCIATES

PLATE A - 1.13

AE-88473

DATE

1/20/89

F.T.

BG

DR.

dmh

O.E.

JC

W.P.

dmh

CHKD

BK

BORING 14

DATE DRILLED: January 10, 1989
EQUIPMENT USED: 5" - Diameter Rotary Wash

ELEVATION 11.8

ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.
10			19.9	92	5	
	5		24.3	86	5	
5			25.6	91	3	
	10		51.6	70	2	
0			47.8	78	3	
	15					
-5			37.9	83	3	
	20		64.6	67	1	
-10						
	25		43.5	74	1	
-15						
	30		32.8	91	1	
-20						
	35		27.1	96	1	
-25						
	40		23.8	99	5	

ML FILL - CLAYEY SILT - some rootlets, brownish grey
Light brown
ML FILL - SANDY SILT - greyish brown
Some organic matter
Dark brown to black, organic odor, some seashells
↓ SURFACE OF NATURAL SOIL
ML CLAYEY SILT - light grey
ML SILT - brownish grey
MH CLAYEY SILT - few seashells, organic odor, bluish grey
Lenses of Sandy Silt, light grey
Dark grey
ML CLAYEY SILT - some Peat, dark grey
(LL = 38; PI = 11)

(CONTINUED ON FOLLOWING PLATE)

LOG OF BORING

LeROY CRANDALL AND ASSOCIATES

PLATE A - 1.14a

Note: The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated.
It is not warranted to be representative of subsurface conditions at other locations and times.

JOB AE-88473

DATE

1/20/89

F.T.

BG

DR.

dmh

O.E. JC

W.P.

dmh

CHKD

BK

Note : The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated.
It is not warranted to be representative of subsurface conditions at other locations and times.

ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD.PEN.TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.
-30			45.4	76	3	
-45			29.1	93	5	
-35						
-50			35.9	84	4	
-40						
-55			80.2	52	4	
-45						
-60			89.6	45	4	
-50						
-65			31.1	93	7	

BORING 14 (Continued)

DATE DRILLED: January 10, 1989
EQUIPMENT USED: 5" - Diameter Rotary Wash

Bluish grey

Dark grey

Layers of Peat, dark brownish grey and brown

Dark grey

NOTE: Drilling mud used in drilling process. Boring grouted with
cement-bentonite mixture. Water level not established.

LOG OF BORING

LeROY CRANDALL AND ASSOCIATES

PLATE A - 1.14b

B AE-88473

DATE 1/20/89

F.T.

TC

DR.

dmh

O.E. JC

W.P.

dmh

CHKD

BK

Note : The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated.
It is not warranted to be representative of subsurface conditions at other locations and times.

							BORING 15	
							DATE DRILLED: January 11, 1989	
							EQUIPMENT USED: 16" - Diameter Bucket	
							ELEVATION 10.7	
ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.		
10			24.2	92	2		ML	FILL - SANDY SILT - greyish brown
			30.9	81	< 1			Lenses of Clayey Silt
5	5		40.0	80	< 1		ML	FILL - CLAYEY SILT - greyish brown
			44.2	79	< 1			Some organic matter
								Layer of Silty Sand
0	10						ML	SURFACE OF NATURAL SOIL
			51.9	69	< 1			SANDY SILT - grey
-5	15		43.9	79	< 1		MH	CLAYEY SILT - light grey (LL = 59; PI = 20)
								Brownish grey
20								

NOTE: Water seepage encountered at a depth of 12'. Water level measured at 12-1/2' 10 minutes after completion of drilling. Caving below 12'. Hole squeezed in below 13-1/2' after drilling to 16'.

LOG OF BORING

LeROY CRANDALL AND ASSOCIATES

PLATE A - 1.15

JOB AE-88473 DATE 1/20/89 F.T. BG DR. dmh W.P. dmh CHKD *BK*

BORING 16

DATE DRILLED: January 10, 1989
EQUIPMENT USED: 5" - Diameter Rotary Wash

ELEVATION 11.3

Note: The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated.
It is not warranted to be representative of subsurface conditions at other locations and times.

ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.	
10			43.6	73	1		ML FILL - CLAYEY SILT - brownish grey
			18.4	95	5		ML FILL - SANDY SILT - light brown
5			21.4	84	1		ML FILL - CLAYEY SILT - brown
5			18.5	110	7		ML FILL - SANDY SILT - few Gravel, brownish grey
0			48.5	71	2		↓ SURFACE OF NATURAL SOIL MH CLAYEY SILT - some Sand, traces of organic matter, dark grey
-5		3					Brownish grey
-10			71.7	56	2		Few seashells, organic odor, bluish grey
-15		4					Dark grey
-20			26.9	97	3		Layer of Sandy Silt, traces of Peat, dark greyish brown
-25		23					Traces of Peat
-30			42.8	77	2		CLAYEY SILT - bluish grey
-35							
-40		15					

(CONTINUED ON FOLLOWING PLATE)

LOG OF BORING

B AE-88473

DATE 1/20/89

F.T.

BG

DR.

dmh

O.E.

JC

W.P.

dmh

CHKD

Note: The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated.
It is not warranted to be representative of subsurface conditions at other locations and times.

ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD.PEN.TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.
-30			23.8	100	8	
-45		8				
-35			37.3	85	8	
-50		15				
-40						
-55			32.5	86	6	
-45						
-60			10.5	129	72	
-50						SP
-65						

BORING 16 (Continued)

DATE DRILLED: January 10, 1989
EQUIPMENT USED: 5" - Diameter Rotary Wash

Some Sand

Dark grey

Traces of Peat

SAND - fine to coarse, some Gravel and Cobbles, light grey

NOTE: Drilling mud used in drilling process. Boring grouted with cement-bentonite mixture. Water level not established.

Drilled 18" - Diameter Bucket Boring 5' east of Rotary Wash Boring to a depth of 13' on 1/12/89 for water level determination. Water seepage encountered at a depth of 11-1/2'. No caving.

LOG OF BORING

LeROY CRANDALL AND ASSOCIATES

PLATE A - 1.16b

BK

CHKD

dmh

W.P.

JC

O.E.

dmh

DR.

BG

F.T.

1/20/89

DATE

AE-88473

JOB

BORING 17

DATE DRILLED: December 30, 1988
EQUIPMENT USED: 5" - Diameter Rotary Wash

ELEVATION 18.2

Note: The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated. It is not warranted to be representative of subsurface conditions at other locations and times.

ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.
15	5	19.3	110	14		CL
		14.5	114	13		
10	10	15.3	115	5		
		20.6	107	4		
5	15	27.0	96	7		ML
		35.0	86	2		
0	20	11.8	117	13		SM
						MH
-5	25	6				
-10	30	63.8	60	3		
-15	35	28.1	94	18		SM
-20	40	18				
		46.8	77	3		MH

FILL - SANDY CLAY - reddish brown

Lenses of Silty Sand, some organic matter, brown and light grey

Lenses of Clayey Silt, pieces of brick, reddish brown and brown

FILL - SANDY SILT - grey

FILL - SILTY SAND - fine, some vegetation, hydrocarbon odor, traces of hydrocarbon, dark brown to black

▽ SURFACE OF NATURAL SOIL

CLAYEY SILT - brownish grey

Large amount of seashells, organic odor, grey

SILTY SAND - fine, grey

CLAYEY SILT - some seashells, grey

(CONTINUED ON FOLLOWING PLATE)

LOG OF BORING

AE-88473

DATE 1/20/89

F.T. BG

DR. dmh

O.E. JC

W.P. dmh

CHKD

BK

Note : The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated.
It is not warranted to be representative of subsurface conditions at other locations and times.

ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD.PEN.TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.
-25		8				
45			23.7	94	45	
-30		10				
50			19.9	109	16	
-35			20.6	109	5	
55						
-40			27.6	94	5	
60			33.4	90	5	

BORING 17 (Continued)

DATE DRILLED: December 30, 1988
EQUIPMENT USED: 5" - Diameter Rotary Wash

Bluish grey
SILTY SAND - fine, grey
CLAYEY SILT - some seashells, bluish grey
Some Sand
SANDY SILT - slightly Clayey, grey
CLAYEY SILT - some cemented lumps, bluish grey

NOTE: Drilling mud used in drilling process. Mud removed to a depth of 40' at completion of drilling. Water level measured at 15-1/2' 4 days after removal of mud. Boring grouted with sand and cement slurry.

LOG OF BORING

LeROY CRANDALL AND ASSOCIATES

PLATE A - 1.17b

BK

JOB AE-88473 DATE 1/20/89 F.T. FH DR. dmh W.P. dmh CHKD

Note : The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated. It is not warranted to be representative of subsurface conditions at other locations and times.

							BORING 18	
							DATE DRILLED: January 11, 1989	
							EQUIPMENT USED: 5" - Diameter Rotary Wash	
							ELEVATION 12.7	
ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.		
10			35.4	87	2		ML	FILL - SANDY SILT - greyish brown
								Lenses of Clayey Silt
5			9.0	88	1		ML	FILL - CLAYEY SILT - bluish grey
5			59.1	62	<1			Dark grey (LL = 45; PI = 14)
10			38.0	84	<1			Brownish grey
0			56.1	66	<1		MH	↓ SURFACE OF NATURAL SOIL CLAYEY SILT - greyish brown
15							ML	SANDY SILT - lenses of Clayey Silt, brown and grey
-5			38.6	84	1		MH	CLAYEY SILT - some seashells, organic odor, grey
20			46.8	74	1			
-10								
25			74.6	53	<1			Brownish grey
-15								Some Sand, dark grey
30			32.1	90	1			No Sand, bluish grey
-20								
35			51.0	68	<1			
-25							ML	CLAYEY SILT - bluish grey
40			22.8	104	6			

(CONTINUED ON FOLLOWING PLATE)

LOG OF BORING

DB AE-88473

DATE

1/20/89

F.T.

FH

DR.

dmh

O.E.

JC

W.P.

dmh

CHKD

Note: The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated. It is not warranted to be representative of subsurface conditions at other locations and times.

ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.
-30						
	45		22.9	88	5	SM
-35						ML
	50		39.4	78	3	
-40						
	55		32.0	92	2	
-45						
	60		42.6	78	2	

BORING 18 (Continued)

DATE DRILLED: January 11, 1989
EQUIPMENT USED: 5" - Diameter Rotary Wash

SILTY SAND - fine, grey

CLAYEY SILT - dark grey

NOTE: Drilling mud used in drilling process. Boring grouted with cement-bentonite mixture. Water level not established.

Drilled 18" - Diameter Bucket Boring 5' east of Rotary Wash Boring to a depth of 16' on 1/12/89 for water level determination. Water seepage encountered at a depth of 11'. Caving below 11' (to 2' in diameter).

LOG OF BORING

LeROY CRANDALL AND ASSOCIATES

PLATE A - 1.18b

JOB AE-88473 DATE 1/20/89 F.T. BG DR. dmh O.E. JC W.P. dmh CHKD *BK*

BORING 19

DATE DRILLED: January 11, 1989
EQUIPMENT USED: 5" - Diameter Rotary Wash

ELEVATION 14.9

Note: The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated.
It is not warranted to be representative of subsurface conditions at other locations and times.

ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.	
			22.1	92	6		SM
			5.0	102	11		
10	5		29.5	92	8		ML
			43.4	81	2		ML
5	10		43.4	76	1		
			40.4	80	1		
0	15						MH
		3					
-5	20		42.1	77	1		
		1					
-10	25						
			26.5	98	12		SM
-15	30						MH
		1					
-20	35		58.6	62	2		
-25	40						

FILL - SILTY SAND - fine, lenses of Clayey Silt, brown and grey
Large amount of seashells

Light brown

FILL - SANDY SILT - lenses of Clayey Silt, some seashells, grey and brown

FILL - CLAYEY SILT - greyish brown

Some Sand, some vegetation, grey and black

↓ SURFACE OF NATURAL SOIL
CLAYEY SILT - brownish grey

Few seashells, organic odor, dark grey

SILTY SAND - fine, few seashells, organic odor, dark grey

CLAYEY SILT - brownish grey

Few seashells

(CONTINUED ON FOLLOWING PLATE)

LOG OF BORING

LeROY CRANDALL AND ASSOCIATES

PLATE A - 1.19a

B AE-88473

DATE 1/20/89

F.T.

BG

DR.

dmh

O.E.

JC

W.P.

dmh

CHKD

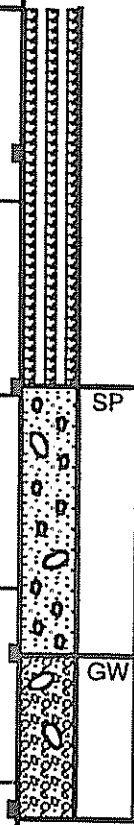
OK

Note: The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated. It is not warranted to be representative of subsurface conditions at other locations and times.

BORING 19 (Continued)

DATE DRILLED: January 11, 1989
EQUIPMENT USED: 5" - Diameter Rotary Wash

ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.
-30	45	4	46.6	73	2	
-35	50	4	47.6	76	6	
-40	55	25	11.0	122	27	
-45	60		9.0	120	24	
-50	65					



Lenses of Silty Sand, traces of Peat, dark grey

GRAVELLY SAND - fine to coarse, about 40% Gravel and Cobbles, lenses of Sandy Silt, traces of Peat, black

GRAVEL - well graded, some Sand, few Cobbles, black

NOTE: Drilling mud used in drilling process. Boring grouted with cement-bentonite mixture. Water level not established.

LOG OF BORING

LeROY CRANDALL AND ASSOCIATES

PLATE A - 1.19b

BK

JOB AE-88473 DATE 1/20/89 F.T. FH DR. dmh W.P. dmh CHKD

BORING 20

DATE DRILLED: January 11, 1989
EQUIPMENT USED: 5" - Diameter Rotary Wash

ELEVATION 12.3

Note : The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated.
It is not warranted to be representative of subsurface conditions at other locations and times.

ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.	
10			29.2	94	7		ML FILL - SANDY SILT - light brown
5							ML FILL - CLAYEY SILT - brownish grey
5			24.0	96	3		ML FILL - SANDY SILT - lenses of Clayey Silt, grey and brown
10			41.1	80	1		ML SURFACE OF NATURAL SOIL SANDY SILT - some organic matter, dark grey
0		5					
15			32.8	90	4		Brownish grey
-5							
		0					MH CLAYEY SILT - some seashells, organic odor, brown and dark grey
20							
-10			48.7	72	<1		Dark grey
25		0					
-15			57.4	64	<1		
30		0					
-20			54.4	68	1		Grey
35		8					
-25							
40			20.7	109	8		SM SILTY SAND - fine, dark grey

(CONTINUED ON FOLLOWING PLATE)

LOG OF BORING

DB AE-88473

DATE 1/20/89

F.T. FH

DR.

dmh

O.E. JC

W.P. dmh

CHKD

Note: The log of subsurface conditions shown hereon applies only at the specific boring location and at the date indicated.
It is not warranted to be representative of subsurface conditions at other locations and times.

ELEVATION (ft.)	DEPTH (ft.)	"N" VALUE STD. PEN. TEST	MOISTURE (% of dry wt.)	DRY DENSITY (lbs./cu. ft.)	DRIVE ENERGY (ft.-kips/ft.)	SAMPLE LOC.
-30		18				
-45			31.8	90	4	
-35		8				ML
-50						
-40						
-55			35.1	85	3	
-45						
-60			33.1	83	2	

BORING 20 (Continued)

DATE DRILLED: January 11, 1989
EQUIPMENT USED: 5" - Diameter Rotary Wash

Layers of Clayey Silt

CLAYEY SILT - lenses of Silty Sand, dark grey

NOTE: Drilling mud used in drilling process. Boring grouted with cement-bentonite mixture. Water level not established.

LOG OF BORING

LeROY CRANDALL AND ASSOCIATES

PLATE A - 1.20b

MAJOR DIVISIONS			GROUP SYMBOLS	TYPICAL NAMES
COARSE GRAINED SOILS (More than 50% of material is LARGER than No. 200 sieve size)	GRAVELS (More than 50% of coarse fraction is LARGER than the No 4 sieve size)	CLEAN GRAVELS (Little or no fines)	 GW	Well graded gravels, gravel-sand mixtures, little or no fines.
			 GP	Poorly graded gravels or gravel-sand mixtures, little or no fines.
		GRAVELS WITH FINES (Appreciable amt of fines)	 GM	Silty gravels, gravel-sand-silt mixtures
			 GC	Clayey gravels, gravel-sand-clay mixtures.
	SANDS (More than 50 % of coarse fraction is SMALLER than the No 4 sieve size)	CLEAN SANDS (Little or no fines)	 SW	Well graded sands, gravelly sands, little or no fines
			 SP	Poorly graded sands or gravelly sands, little or no fines.
		SANDS WITH FINES (Appreciable amt. of fines)	 SM	Silty sands, sand-silt mixtures.
			 SC	Clayey sands, sand-clay mixtures.
FINE GRAINED SOILS (More than 50% of material is SMALLER than No 200 sieve size)	SILTS AND CLAYS (Liquid limit LESS than 50)	 ML	ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity
		 CL	CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays.
		 OL	OL	Organic silts and organic silty clays of low plasticity.
	SILTS AND CLAYS (Liquid limit GREATER than 50)	 MH	MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts.
		 CH	CH	Inorganic clays of high plasticity, fat clays.
		 OH	OH	Organic clays of medium to high plasticity, organic silts
		HIGHLY ORGANIC SOILS		 Pt

BOUNDARY CLASSIFICATIONS: Soils possessing characteristics of two groups are designated by combinations of group symbols.

P A R T I C L E S I Z E L I M I T S							
SILT OR CLAY	SAND			GRAVEL		COBBLES	BOULDERS
	FINE	MEDIUM	COARSE	FINE	COARSE		
	NO. 200	NO. 40	NO. 10	NO. 4	$\frac{3}{16}$ in.	3 in.	(12 in.)
	U S S T A N D A R D S I E V E S I Z E						

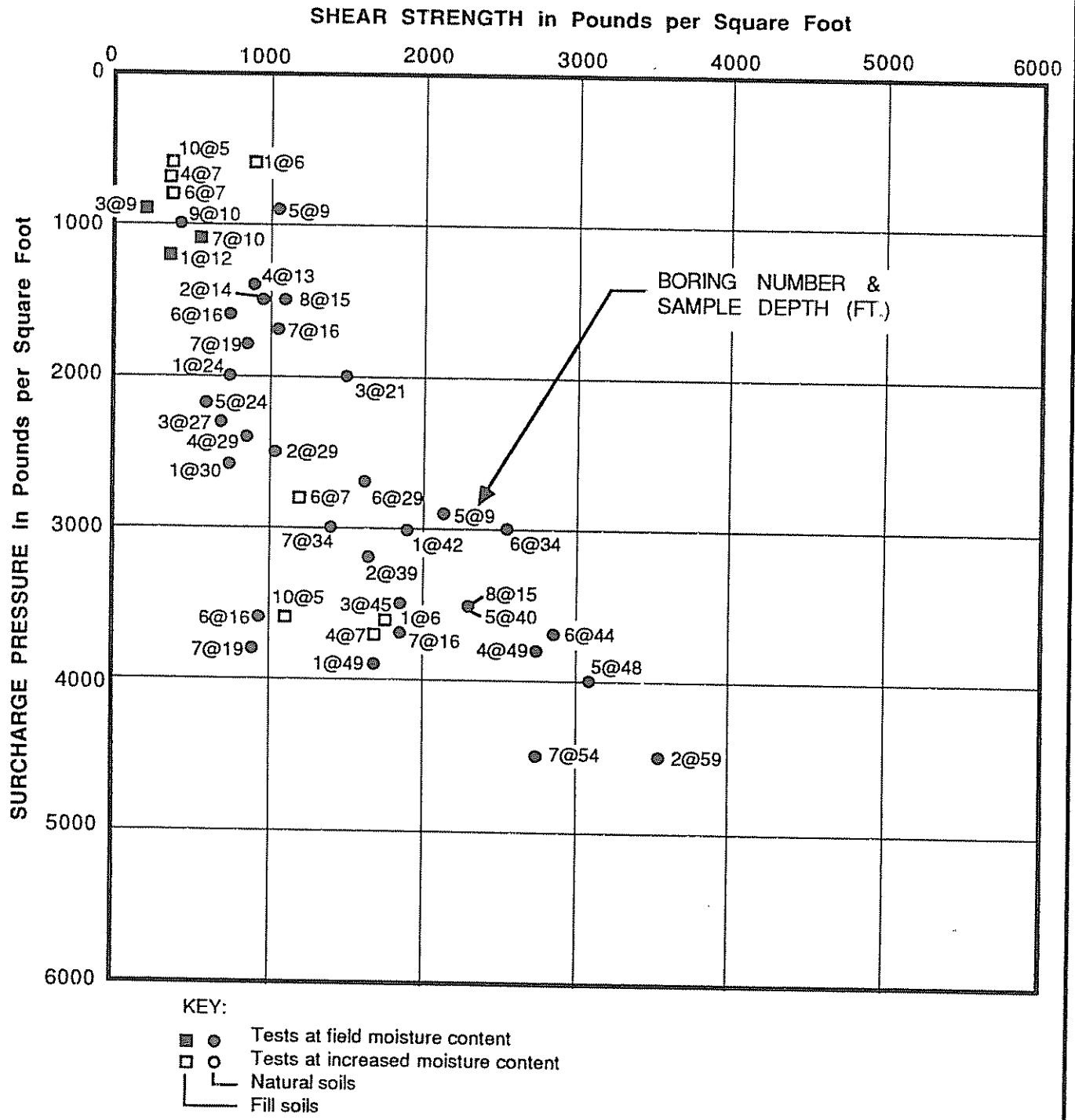
UNIFIED SOIL CLASSIFICATION SYSTEM

Reference :
The Unified Soil Classification System, Corps of Engineers, U. S. Army Technical Memorandum No. 3-357, Vol I, March, 1953. (Revised April, 1960)

LEROY CRANDALL & ASSOCIATES

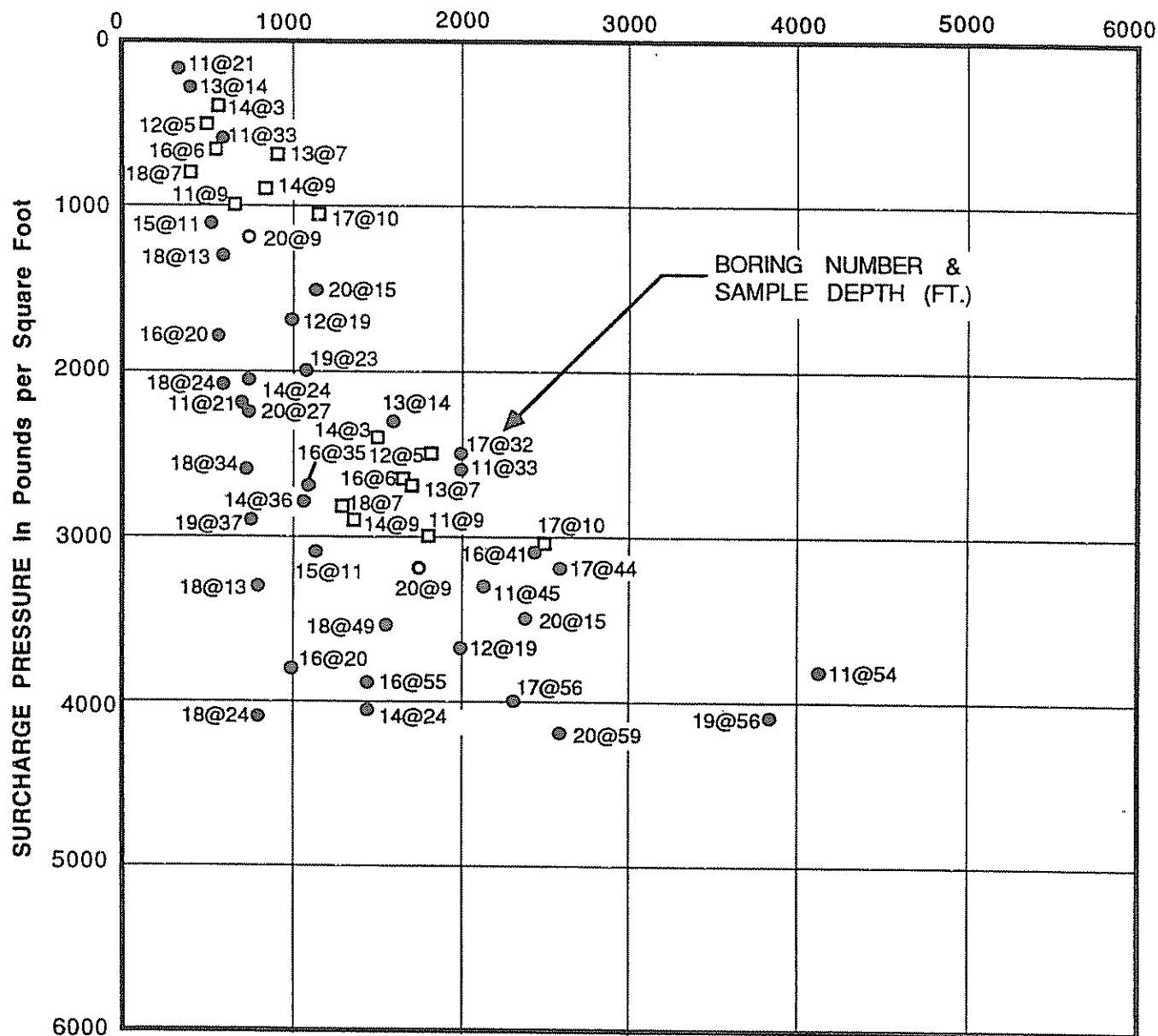
PLATE A-2

JOB AE-88473 DATE 1/31/89 DR lp O.E. JC CHKD. BK



DIRECT SHEAR TEST DATA

SHEAR STRENGTH in Pounds per Square Foot



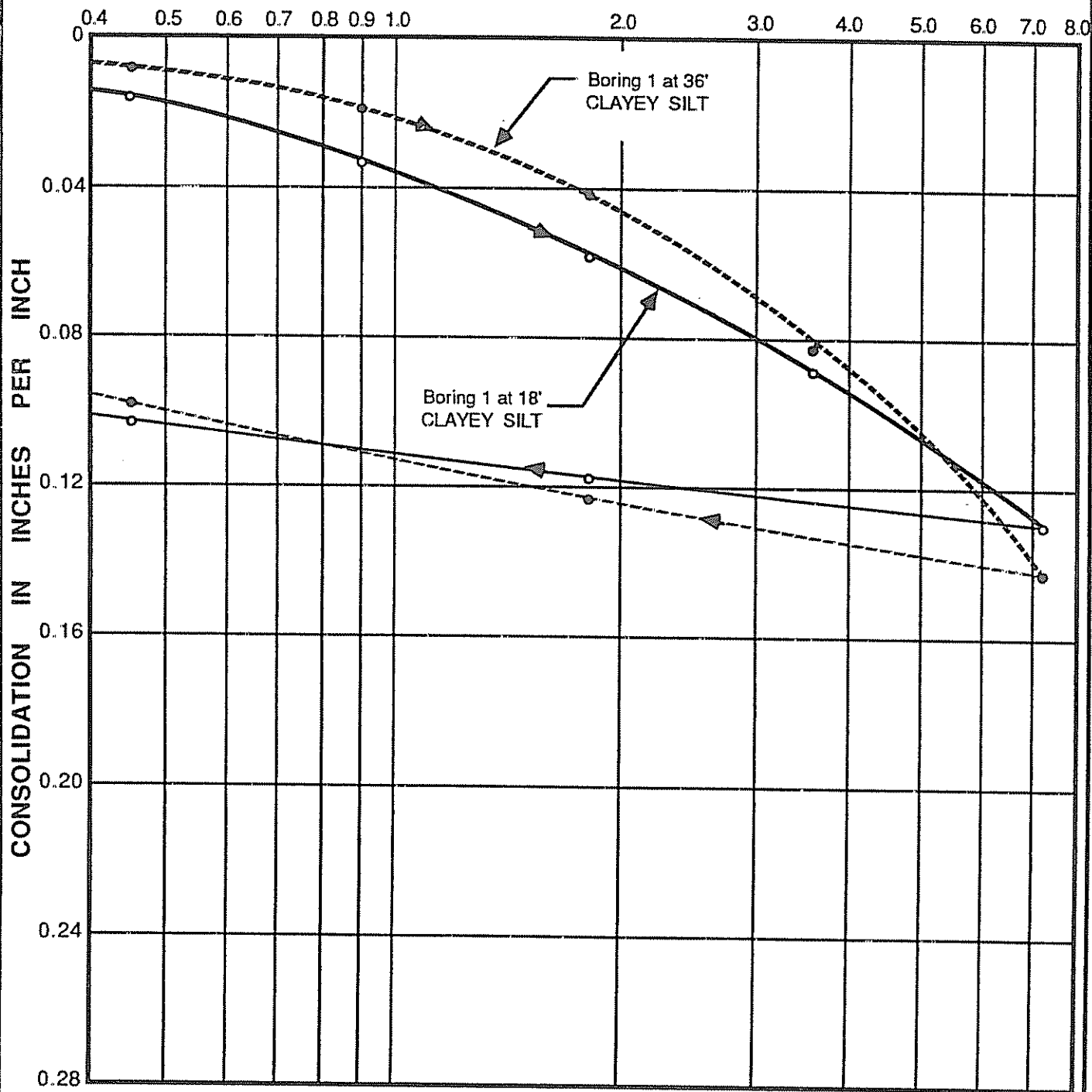
DIRECT SHEAR TEST DATA

LeROY GRANDALL AND ASSOCIATES

PLATE A - 3.2

JOB AE-88473 DATE 1/31/89 DR lp O.E. JC CHKD. *OK*

LOAD IN KIPS PER SQUARE FOOT



NOTE: Samples tested at field moisture content.

CONSOLIDATION TEST DATA

LeROY CRANDALL AND ASSOCIATES

PLATE A-4.1

CHKD

O.E. JC

dmh

W.P.

dmh

DR.

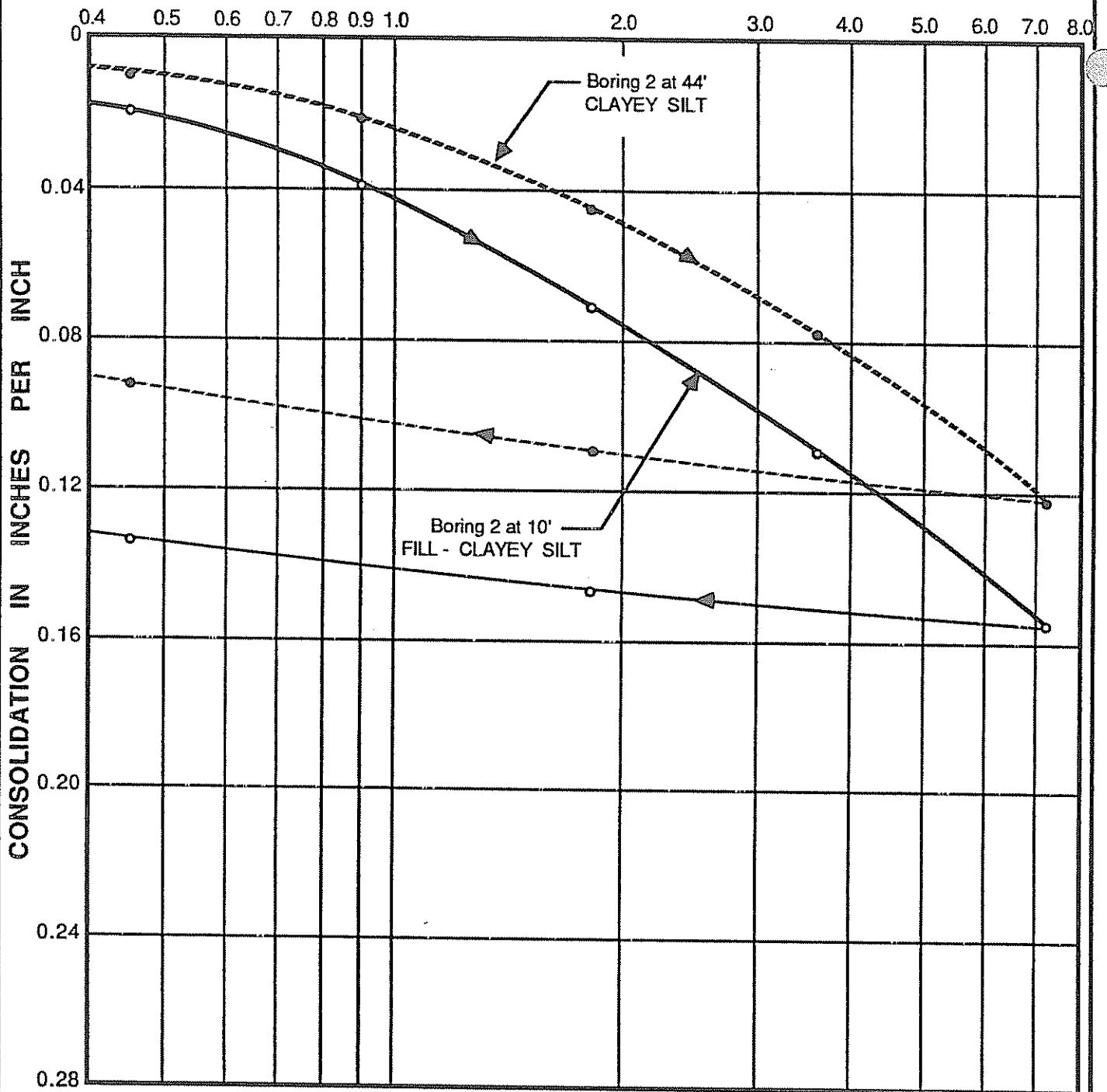
1/25/89

DATE

AE-88473

JOB

LOAD IN KIPS PER SQUARE FOOT



NOTE: Samples tested at field moisture content.

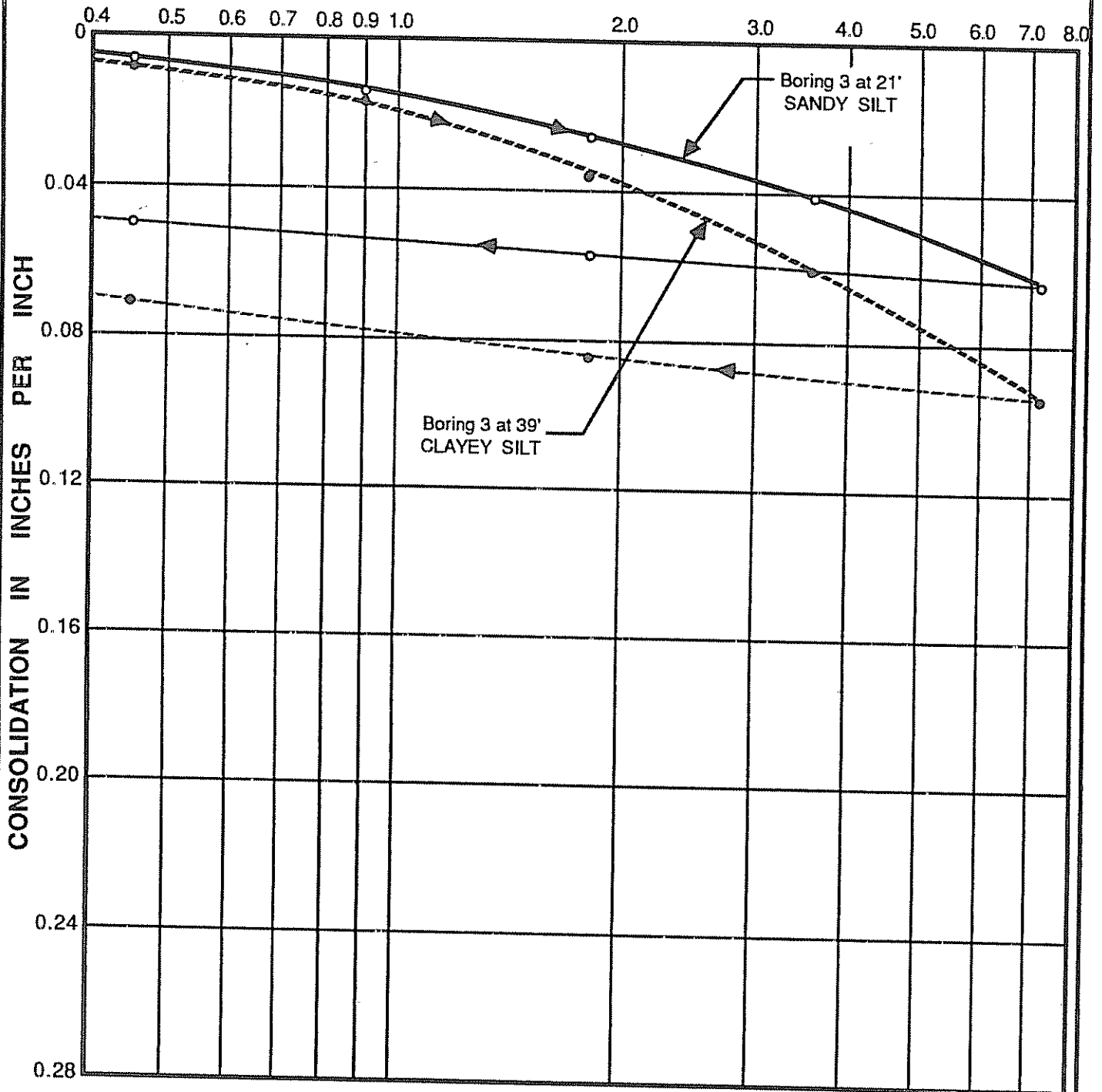
CONSOLIDATION TEST DATA

LeROY CRANDALL AND ASSOCIATES

PLATE A-4.2

JOB AE-88473 DATE 1/25/89 DR. dmh W.P. dmh O.E. JC CHKD

LOAD IN KIPS PER SQUARE FOOT



NOTE: Samples tested at field moisture content.

CONSOLIDATION TEST DATA

LeROY CRANDALL AND ASSOCIATES

PLATE A - 4.3

BK

CHKD

O.E. JC

dmh

W.P.

dmh

DR.

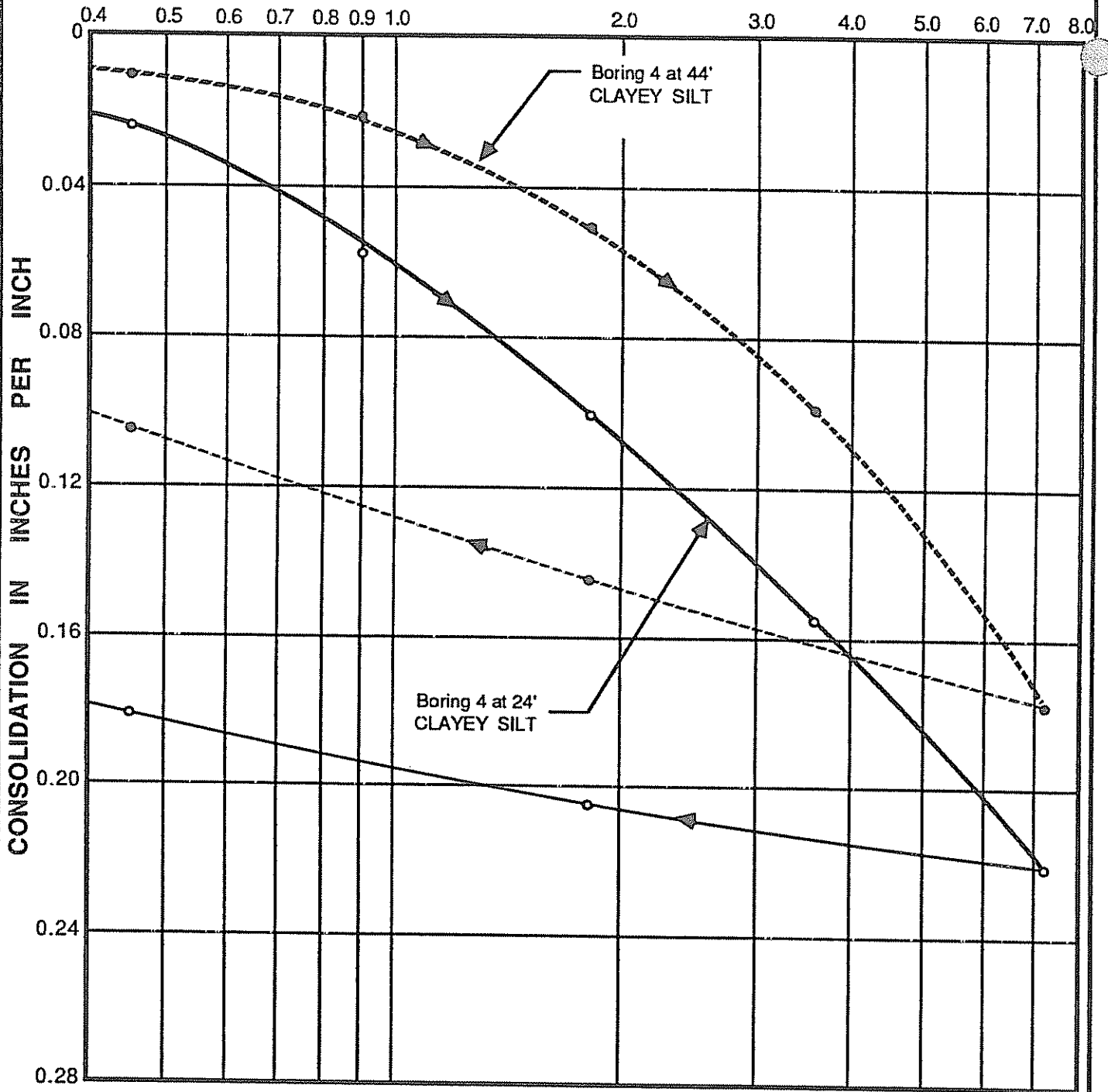
DATE

1/25/89

AE-88473

JOB

LOAD IN KIPS PER SQUARE FOOT



NOTE: Samples tested at field moisture content.

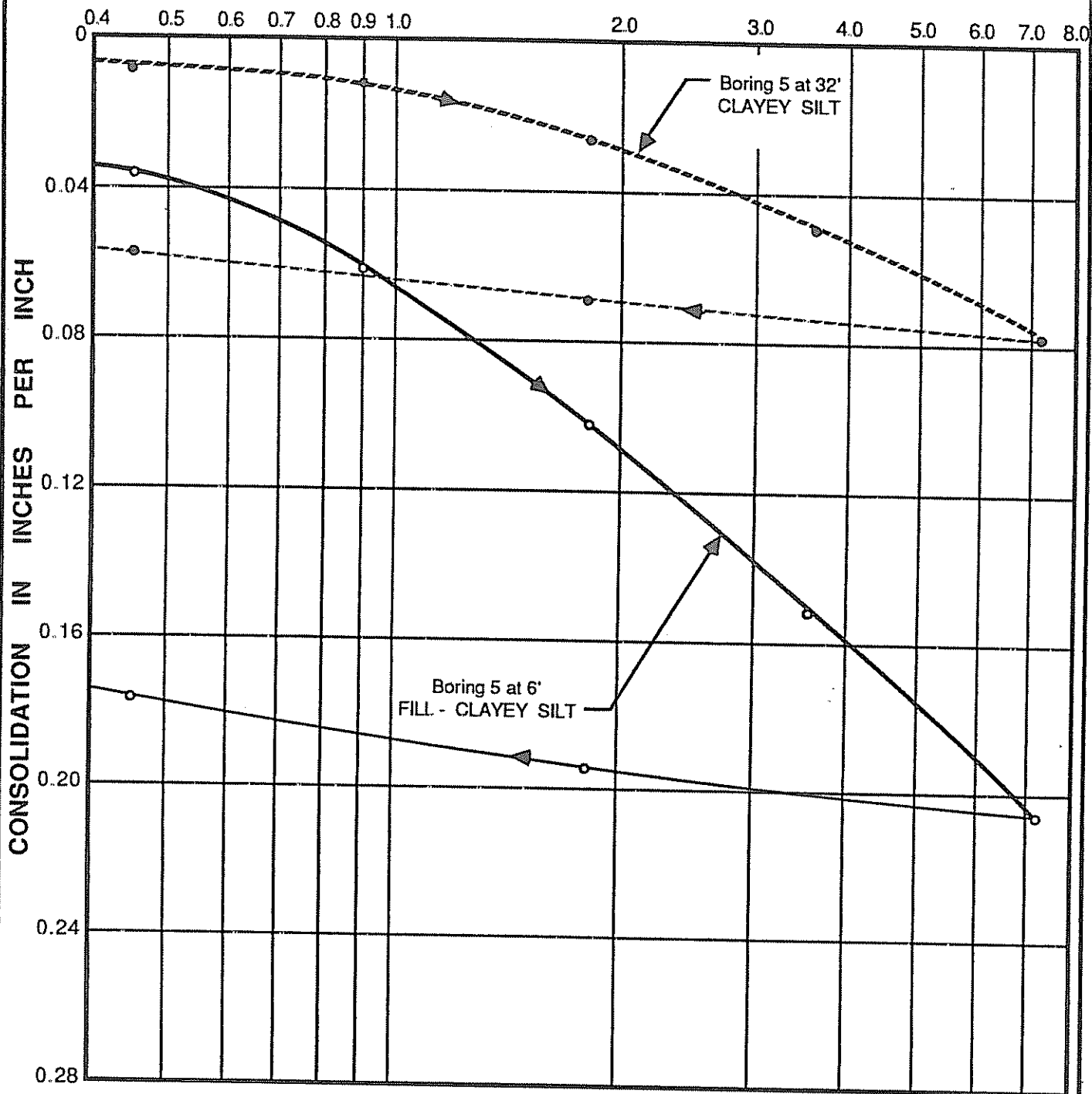
CONSOLIDATION TEST DATA

LeROY CRANDALL AND ASSOCIATES

PLATE A-4.4

JOB AE-88473 DATE 1/25/89 DR. dmh W.P. dmh O.E. JC CHKD BK

LOAD IN KIPS PER SQUARE FOOT



NOTE: Samples tested at field moisture content.

CONSOLIDATION TEST DATA

LeROY CRANDALL AND ASSOCIATES

PLATE A-4.5

CHKD

O.E.

JC

dmh

W.P.

dmh

DR.

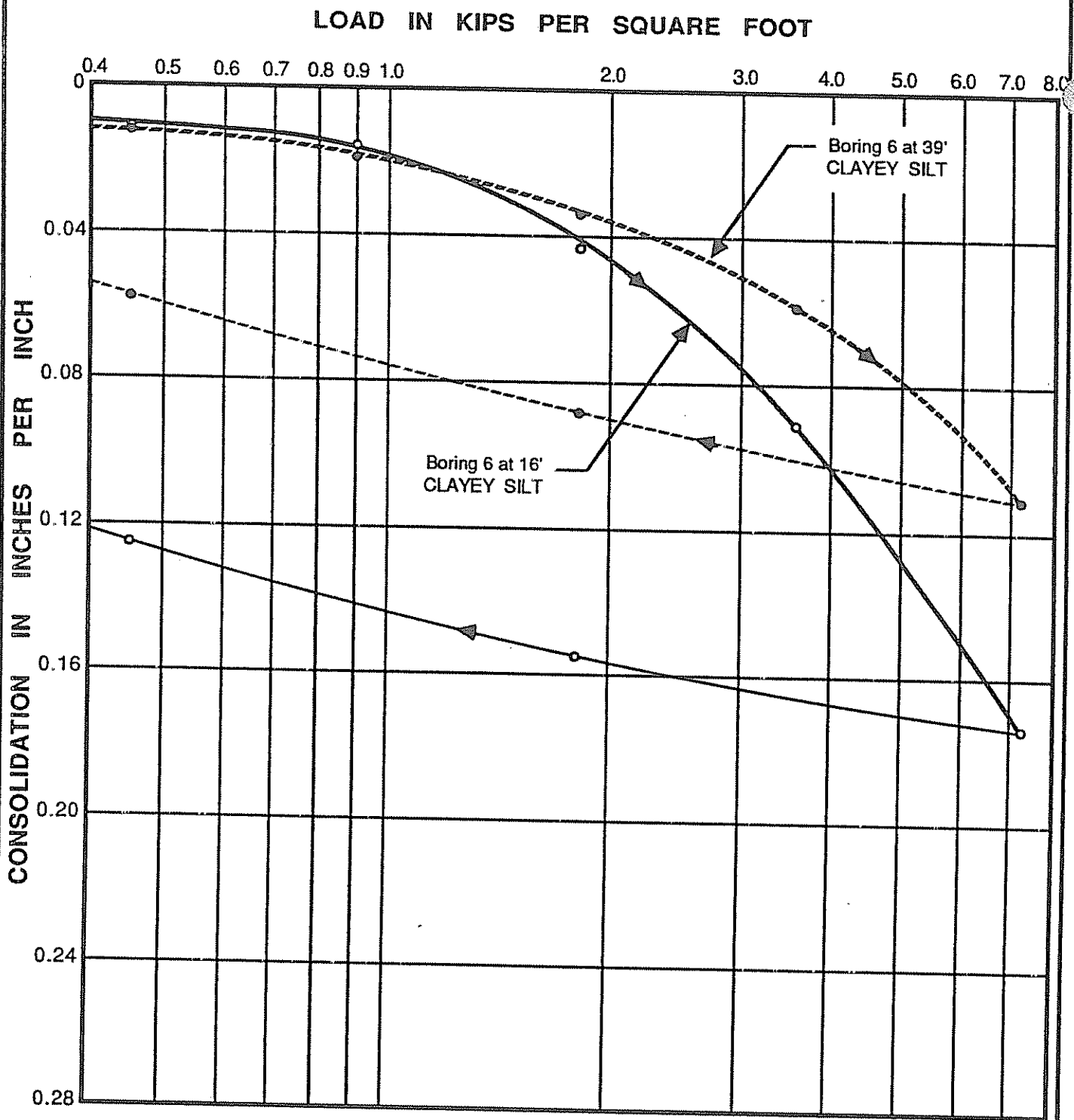
DATE

1/25/89

AE-88473

JOB

JOB AE-88473 DATE 1/25/89 DR. dmh W.P. dmh O.E. JC CHKD *BK*



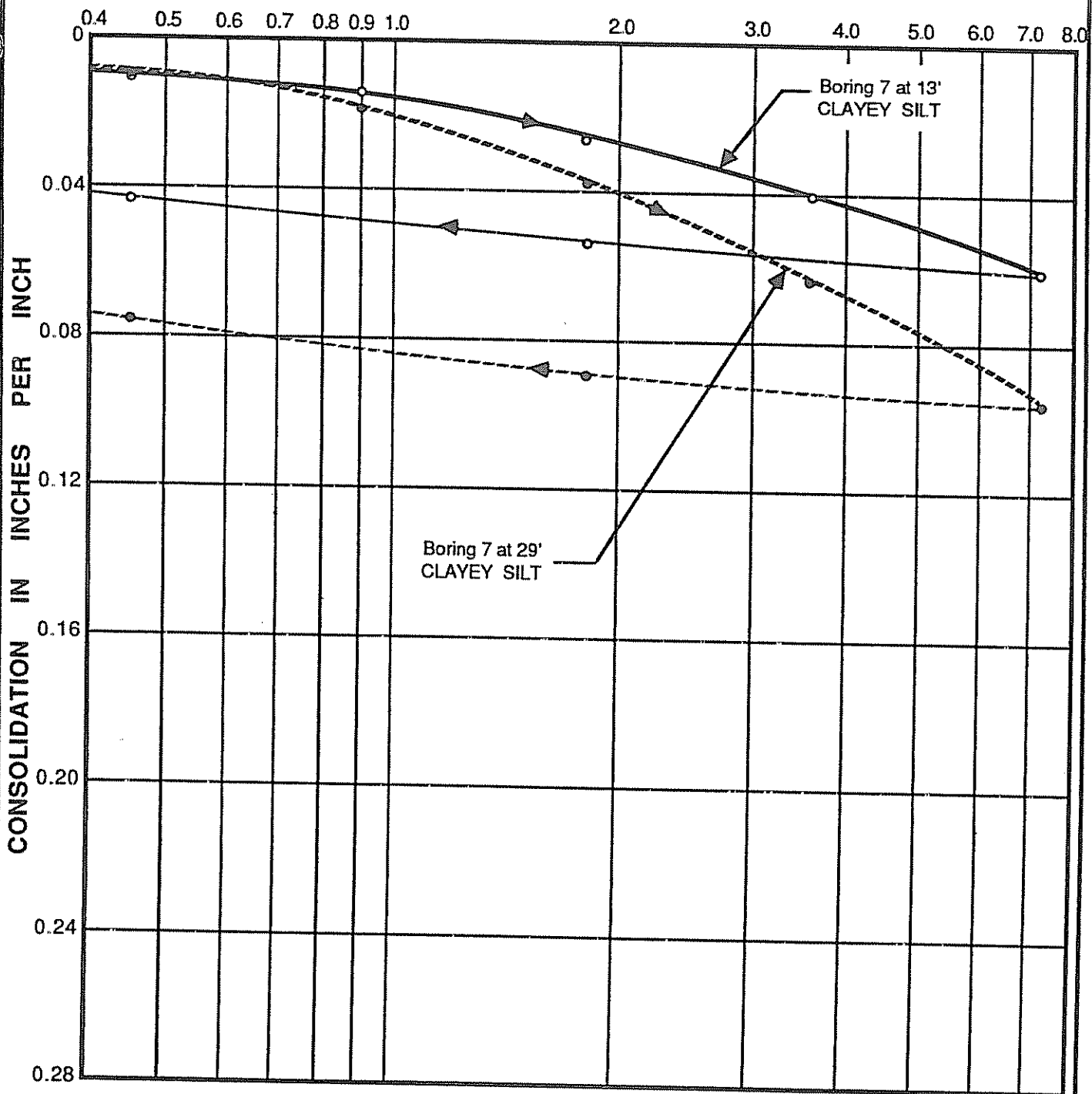
NOTE: Samples tested at field moisture content.

CONSOLIDATION TEST DATA

LeROY CRANDALL AND ASSOCIATES

PLATE A-4.6

LOAD IN KIPS PER SQUARE FOOT



NOTE: Samples tested at field moisture content.

CONSOLIDATION TEST DATA

LeROY CRANDALL AND ASSOCIATES

PLATE A-4.7

JOB AE-88473

DATE 1/25/89

DR. dmh

W.P. dmh

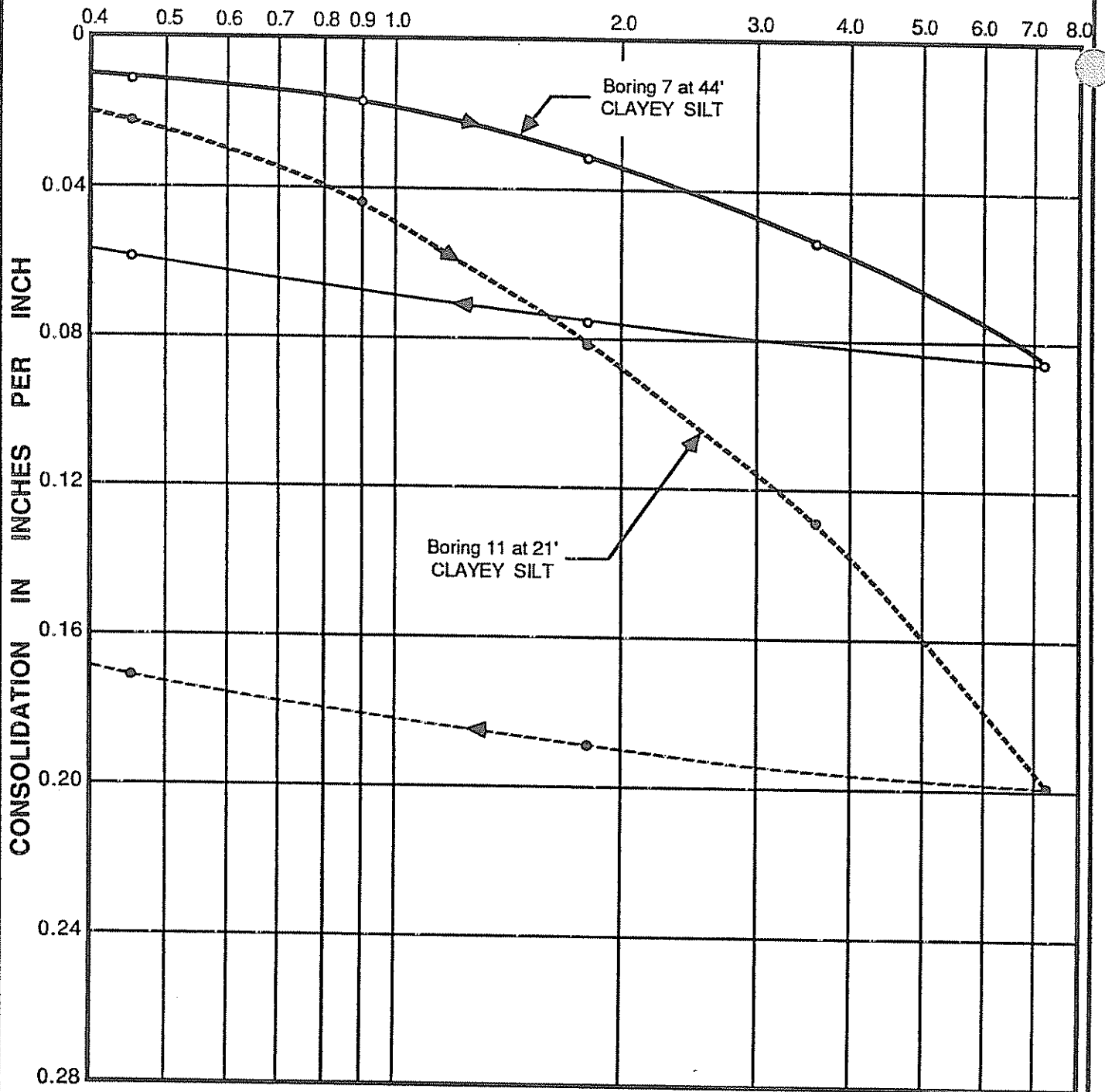
O.E. JC

CHKD

CHKD

BK

LOAD IN KIPS PER SQUARE FOOT



NOTE: Samples tested at field moisture content.

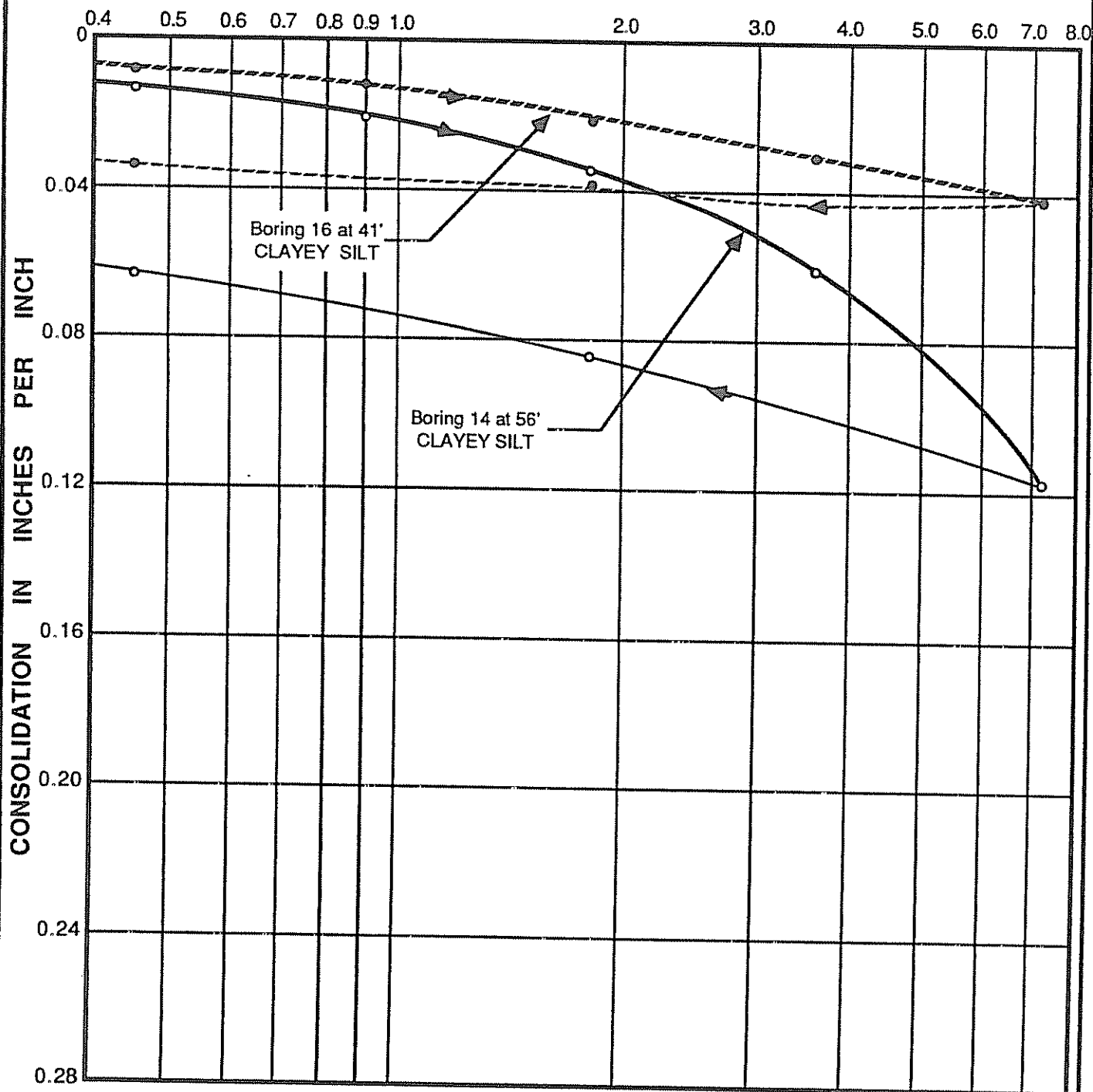
CONSOLIDATION TEST DATA

LeROY CRANDALL AND ASSOCIATES

PLATE A-4.8

JOB AE-88473 DATE 1/25/89 DR. dmh W.P. dmh O.E. JC CHKD BK

LOAD IN KIPS PER SQUARE FOOT



NOTE: Samples tested at field moisture content.

CONSOLIDATION TEST DATA

LeROY CRANDALL AND ASSOCIATES

PLATE A - 4.9

CHKD

O.E. JC

dmh

dmh

DR.

1/25/89

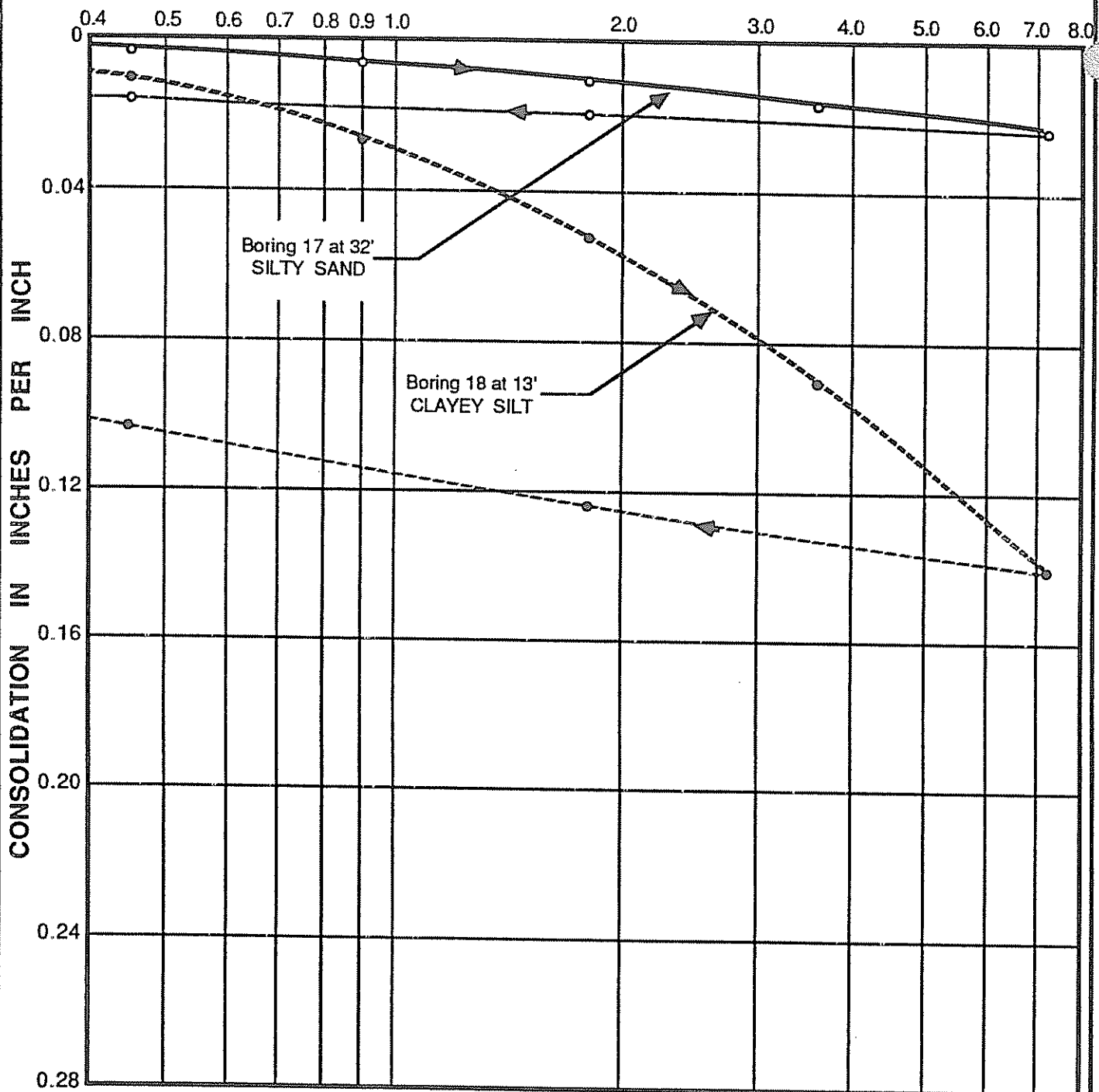
DATE

AE-88473

JOB

JOB AE-88473 DATE 1/25/89 DR. dmh W.P. dmh O.E. JC CHKD *JK*

LOAD IN KIPS PER SQUARE FOOT



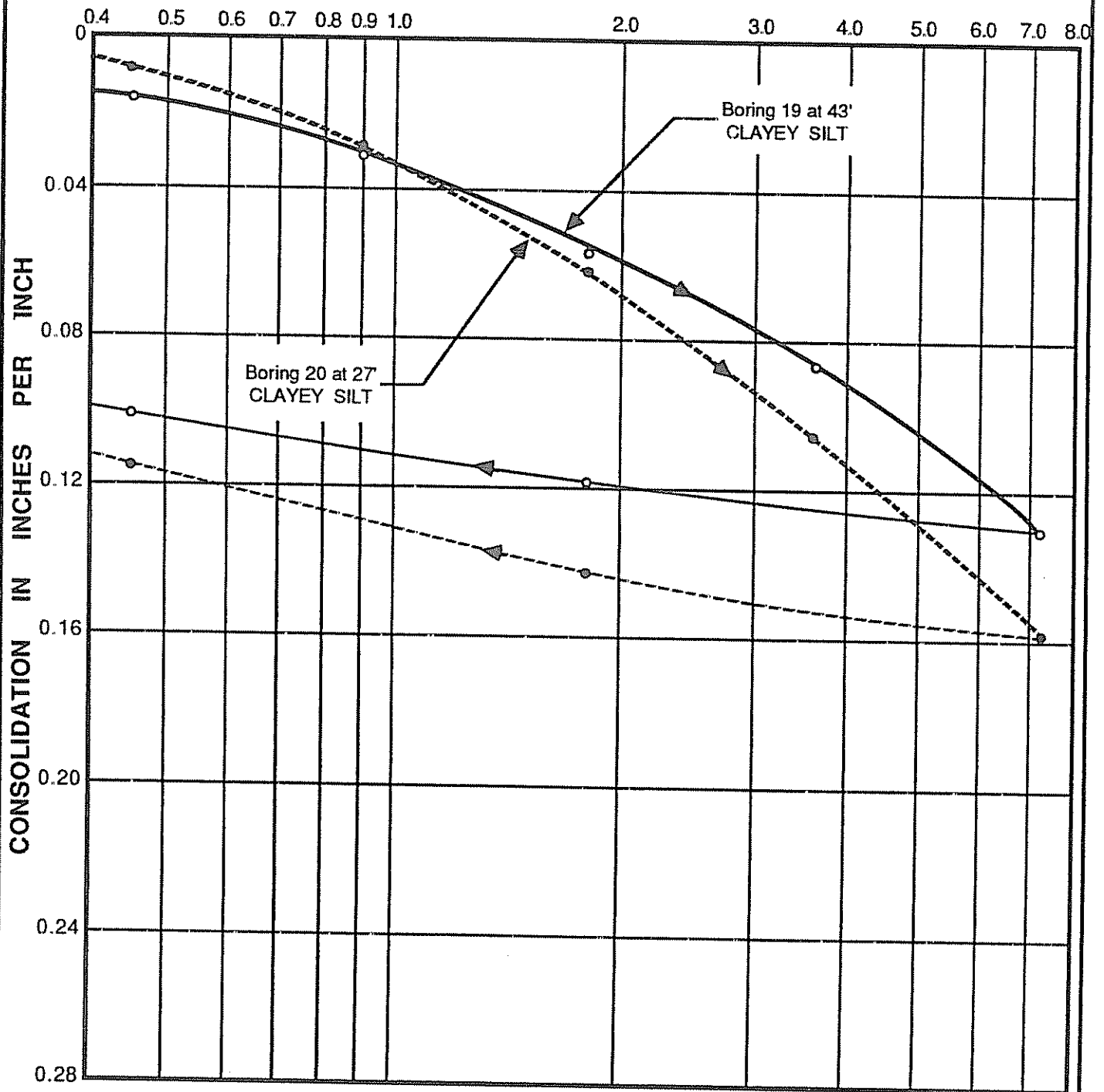
NOTE: Samples tested at field moisture content.

CONSOLIDATION TEST DATA

LeROY CRANDALL AND ASSOCIATES

PLATE A-4.10

LOAD IN KIPS PER SQUARE FOOT



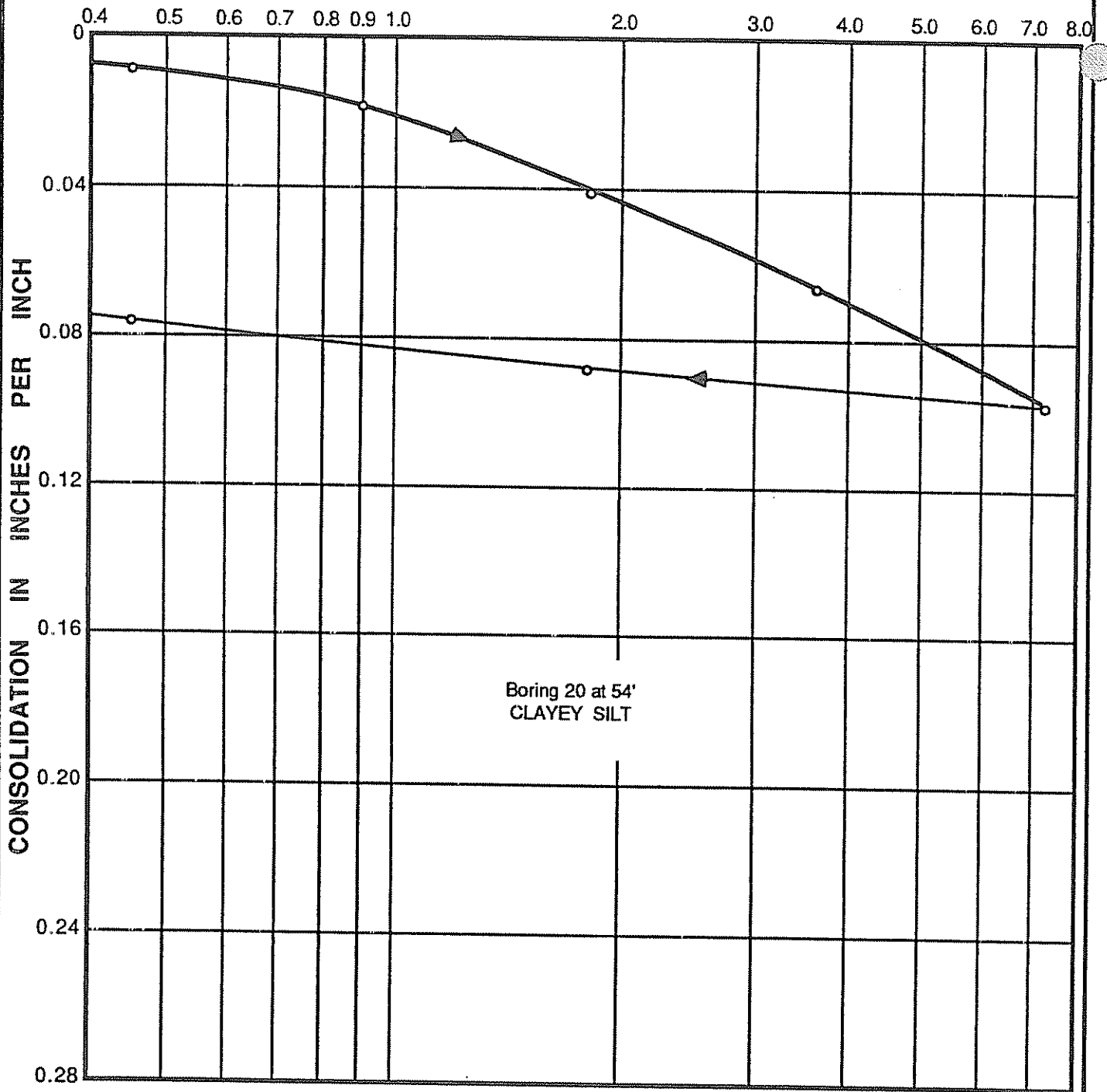
NOTE: Samples tested at field moisture content.

CONSOLIDATION TEST DATA

LeROY CRANDALL AND ASSOCIATES

PLATE A - 4. 11

LOAD IN KIPS PER SQUARE FOOT



Boring 20 at 54'
CLAYEY SILT

NOTE: Sample tested at field moisture content.

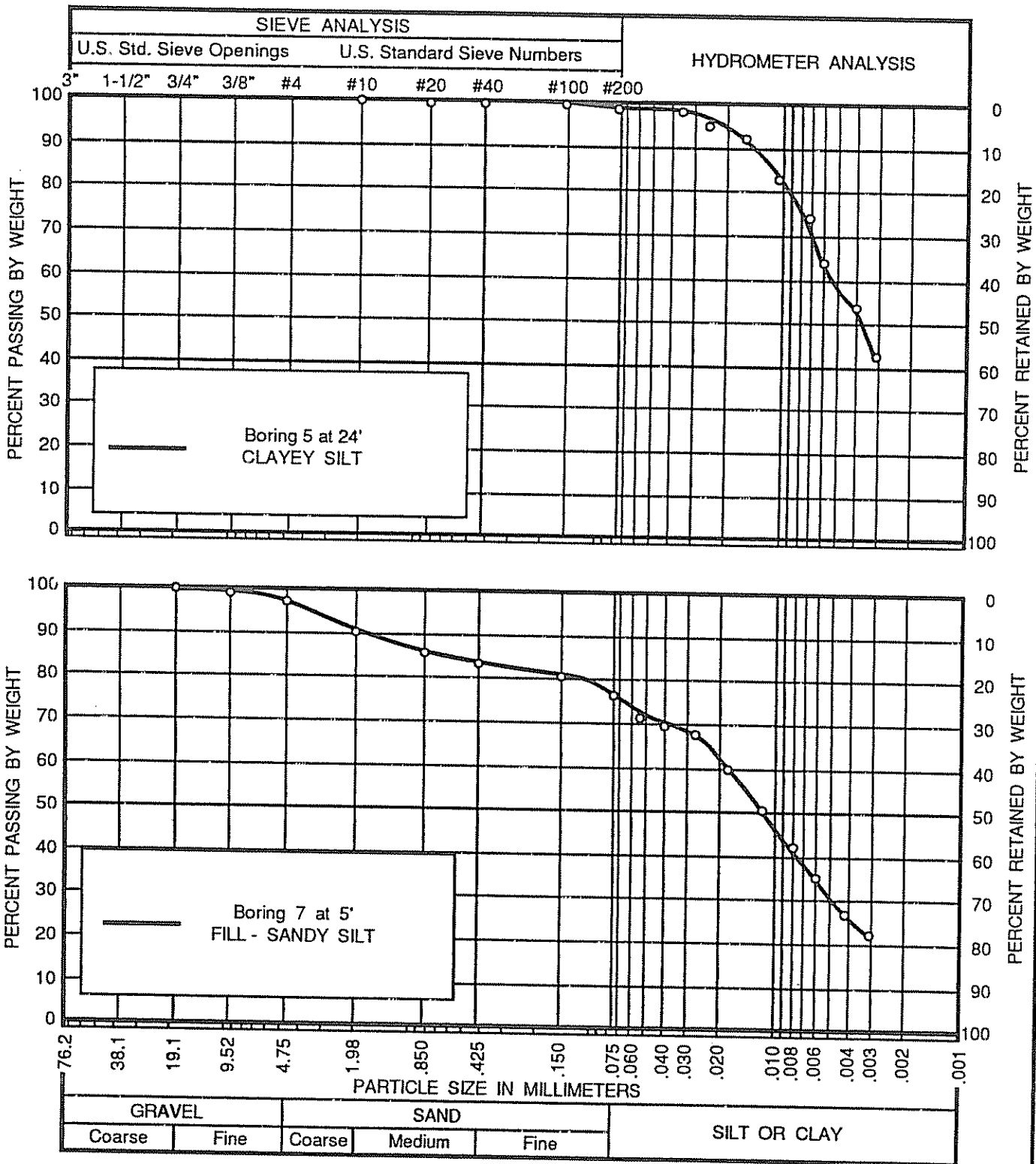
CONSOLIDATION TEST DATA

LeROY CRANDALL AND ASSOCIATES

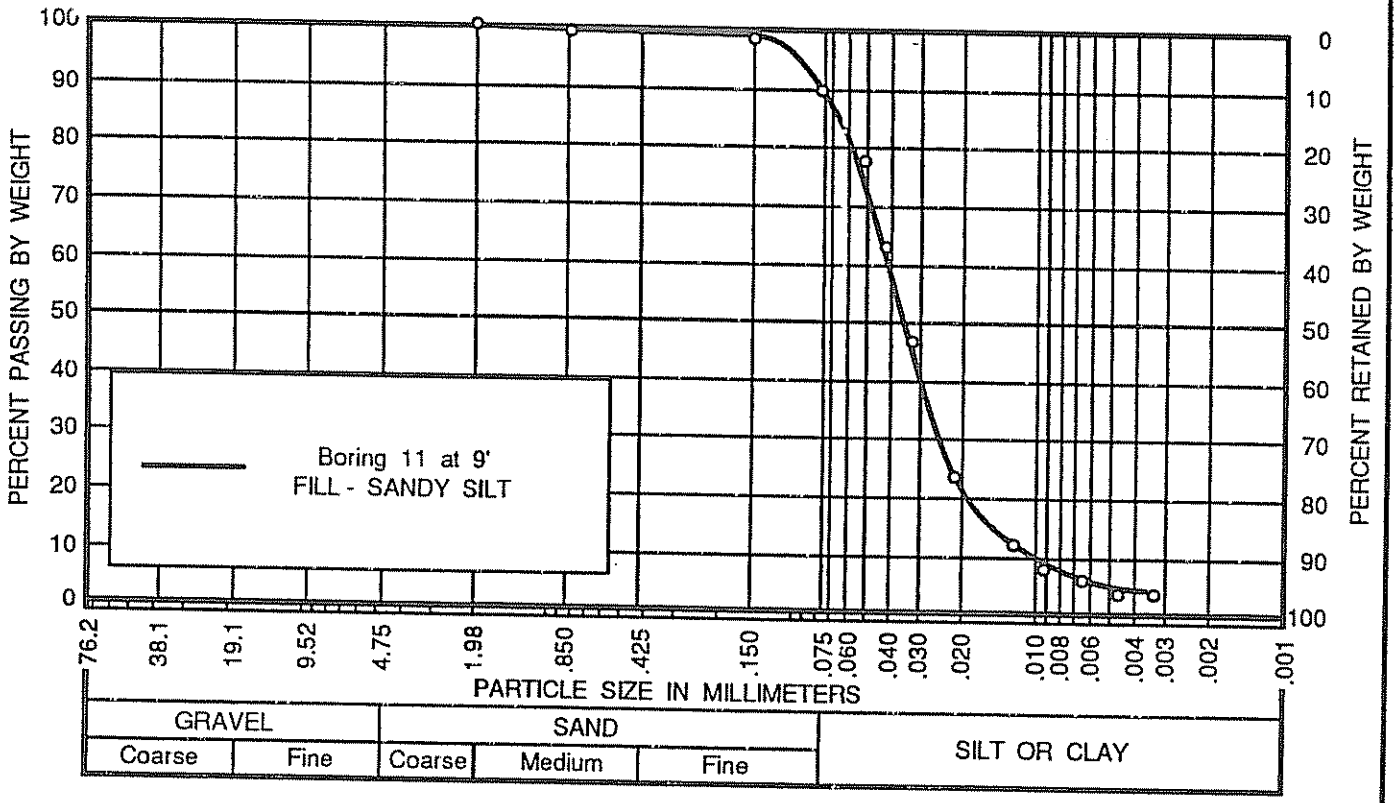
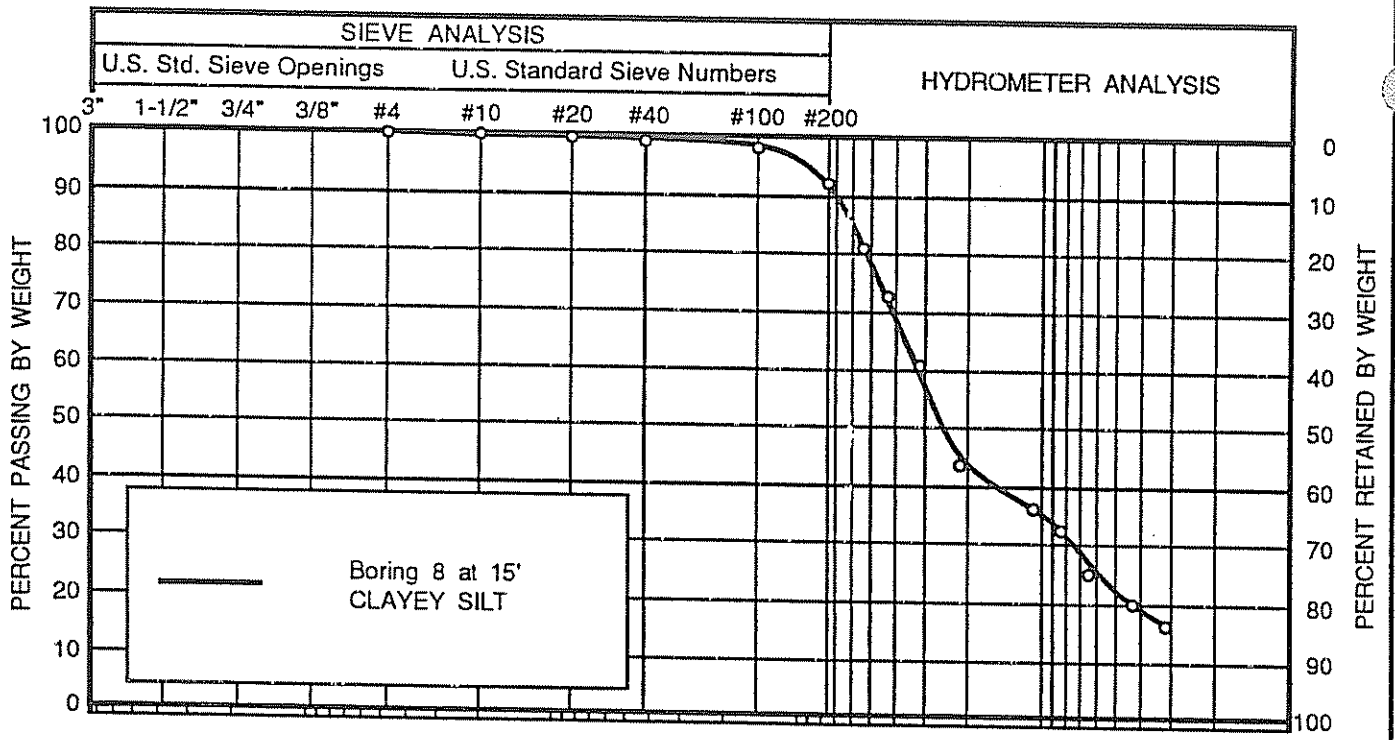
PLATE A - 4.12

JOB AE-88473 DATE 1/25/89 DR. dmh W.P. dmh O.E. JC CHKD *BE*

JOB AE-88473 DATE 3/23/89 BY dmh O.E. BK CHKD. *[Signature]*

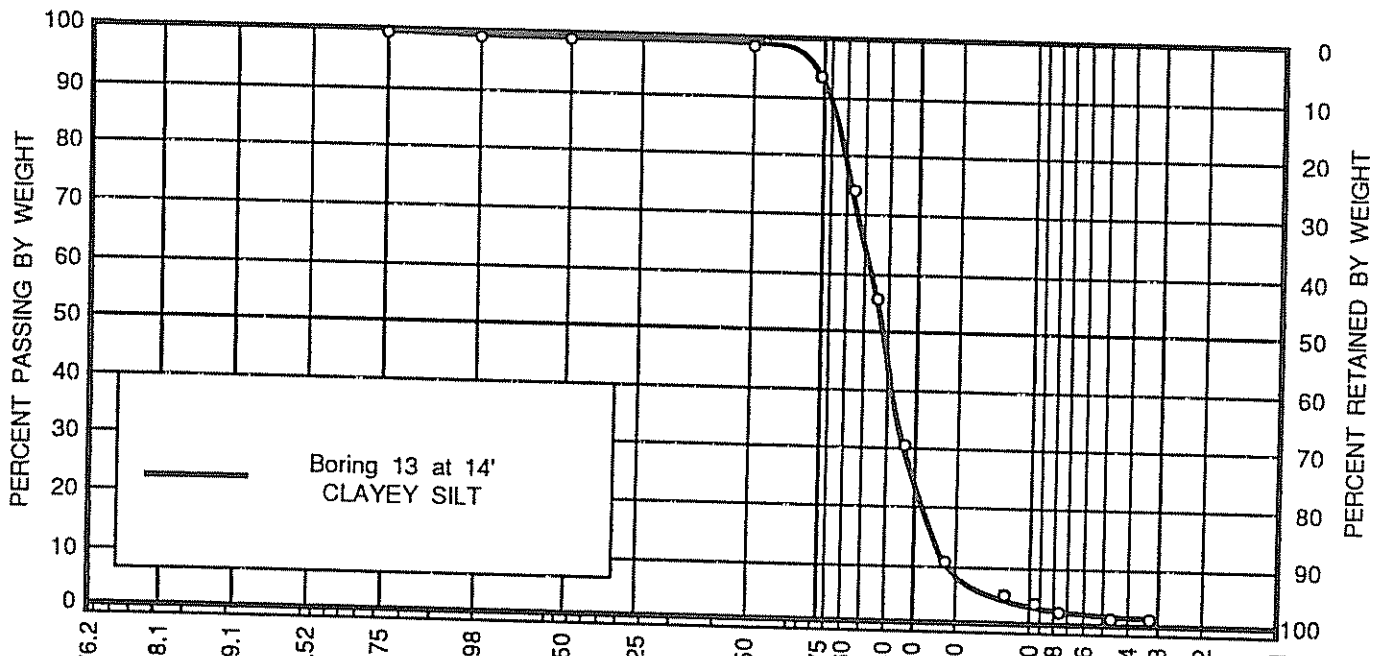
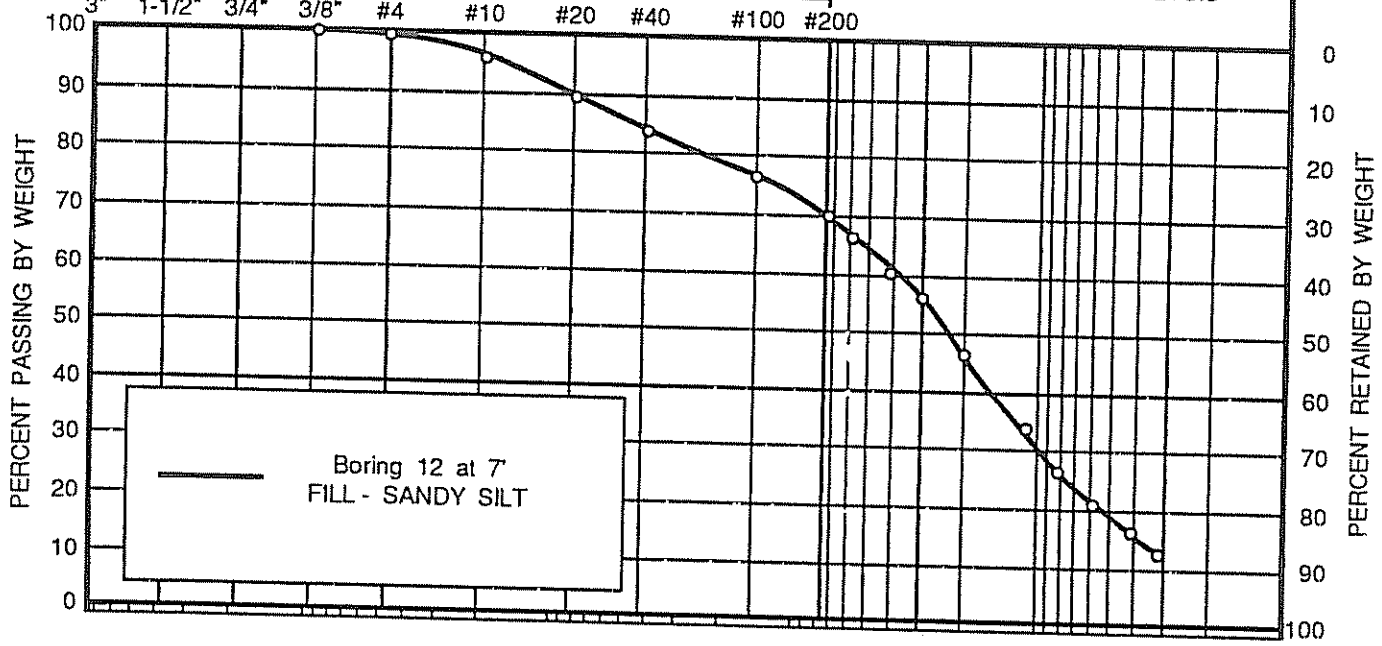


JOB AE-88473 DATE 3/23/89 BY dmh O.E. BK CHKD. *PC*



PARTICLE SIZE DISTRIBUTION

SIEVE ANALYSIS											
U.S. Std. Sieve Openings						U.S. Standard Sieve Numbers					
3"	1-1/2"	3/4"	3/8"	#4	#10	#20	#40	#100	#200	HYDROMETER ANALYSIS	



GRAVEL					SAND			SILT OR CLAY		
Coarse	Fine				Coarse	Medium	Fine			

PARTICLE SIZE DISTRIBUTION

JOB AE-88473 DATE 3/23/89 BY dmh O.E. BK CHKD.

BOREING NUMBER
AND SAMPLE DEPTH :

8 at 0' to 4'

9 at 4' to 8'

SOIL TYPE :

FILL - SANDY SILT

FILL - CLAYEY SILT

MAXIMUM DRY DENSITY :
(lbs./cu. ft.)

116

110

OPTIMUM MOISTURE CONTENT :
(% of dry wt.)

14

19

TEST METHOD: ASTM Designation D1557 - 70

COMPACTION TEST DATA

LeROY CRANDALL AND ASSOCIATES

PLATE A-6

CHKD

JC

O.E.

dmh

W.P.

DATE 1/26/89

JOB AE-88473

PERMEABILITY TEST DATA

BORING NUMBER AND SAMPLE DEPTH	SOIL TYPE	SURCHARGE PRESSURE (psf)	COEFFICIENT OF PERMEABILITY	
			Ft./Yr.	Cm./Sec.
1 at 24'	CLAYEY SILT	900	0.1	1×10^{-7}
2 at 14'	SANDY SILT	200	11.1	1×10^{-5}
4 at 19'	SILTY SAND	500	3.6	3×10^{-6}
5 at 16'	SILTY SAND	400	1.9	2×10^{-6}
6 at 29'	SANDY SILT	1000	0.1	1×10^{-7}
11 at 33'	SANDY SILT	1000	7.5	7×10^{-6}
12 at 14'	CLAYEY SILT	200	0.1	1×10^{-7}
14 at 20'	CLAYEY SILT	600	0.1	1×10^{-7}
16 at 28'	SANDY SILT	1000	0.1	9×10^{-8}
19 at 23'	CLAYEY SILT	900	0.3	3×10^{-7}

M. J. SCHIFF & ASSOCIATES

Consulting Corrosion Engineers

1291 NORTH INDIAN HILL BOULEVARD
CLAREMONT, CALIFORNIA 91711
714/626-0967
FAX 714/621-1419

February 28, 1989

LeROY CRANDALL & ASSOCIATES
900 Grand Central Avenue
Glendale, California 91201-3009

Attention: Mr. Boris Korin

Re: Soil Corrosivity Tests
Playa Vista Development
Los Angeles, California
Your #AE-88473, MJS&A #89012

Gentlemen:

Laboratory tests have been completed on 11 soil samples we selected from your exploratory borings for the subject marina on Lincoln Boulevard at Ballona Creek. Development of the marina will require the installation of retaining walls. The walls could consist of either gravity walls or driven sheet piling. The purpose of these tests was to determine if these soils may have deleterious effects on the sheet piling, concrete structures and foundations, steel piles and underground utilities.

The electrical resistivity of each sample was measured in its as-received condition and again with distilled water added to create the standardized condition of saturation. Resistivities are at about their lowest value when the soil is saturated. The samples were chemically analyzed for the major anions and cations, and pH was measured. Sulfides and oxidation-reduction (redox) potentials were also measured on samples containing peat or a sulfur odor. Results are shown in Table 1.

One of the most useful factors in determining soil corrosivity is electrical resistivity. The electrical resistivity of a soil is a measure of its resistance to the flow of electrical current. Corrosion of buried metal is an electrochemical process in which the amount of metal loss due to corrosion is directly proportional to the flow of electrical current (DC) from the metal into the soil. A soil's resistivity decreases and therefore its corrosivity increases primarily as its moisture and chemical contents increase.

A commonly accepted correlation between electrical resistivity and corrosivity toward ferrous metals is:

<u>Soil Resistivity</u> <u>in ohm-centimeters</u>	<u>Corrosivity Category</u>
0 to 1,000	severely corrosive
1,000 to 2,000	corrosive
2,000 to 10,000	moderately corrosive
over 10,000	mildly corrosive

Electrical resistivities measured in the laboratory with as-received moisture content were in moderately corrosive to severely corrosive categories. However, most of the samples were in the severely corrosive category. When saturated, all resistivities were in the severely corrosive category. Some soils were at or near their most corrosive moisture content as-received. The wide variation in soil resistivity can create concentration type corrosion cells that increase corrosion rates above what would be expected from the soil characteristics alone.

pH values varied from 7.2 to 8.3 which is neutral to moderately alkaline. This is not significant in evaluating corrosivity in this case.

The chemical content of the samples was very high, except for the samples from borings 14 and 17. As might be expected, sodium chloride was the predominant compound. Chloride ions are particularly corrosive to ferrous metals and in such high concentrations can overcome the corrosion inhibiting effect of concrete on reinforcing steel.

Sulfates were fairly high in a few samples. Sulfates at concentrations above about 1000 ppm are moderately aggressive towards concrete.

Sulfides, which are aggressive to copper, showed a strong reaction in a qualitative test on two deep samples containing organic material. The low or negative redox potentials indicate reducing conditions in which anaerobic, sulfide producing bacteria thrive.

We classify this site as severely corrosive to ferrous metals, possibly aggressive toward copper, and moderately deleterious toward concrete. The following corrosion control measures are recommended.

Underground steel utilities should be blasted and given a high quality protective coating such as extruded polyethylene, 50 mil tape coating system over primer, or hot applied coal tar enamel or tape.

Buried steel piping should be electrically insulated from dissimilar metals, cement-mortar or concrete coated steel, and above ground steel pipe. Underground steel pipe should be bonded for electrical continuity if rubber gasketed, mechanical, grooved end, or other nonconductive type joints are used.

Cathodic protection is strongly recommended for underground steel utilities.

The use of nonmetallic materials should be considered.

It is assumed that prestressed concrete piles will contain at least 8 sacks of type 2 prestress cement per cubic yard of concrete and a water/cement ratio not exceeding 0.45. With such a mix, concrete cover should be at least 2 inches thick over embedded steel from a corrosion viewpoint. Solid steel lifting lugs are recommended to prevent groundwater from wicking into the pile interior. If wire rope lifting lugs are used, they should be carefully drilled out 1.5 inches deep and the hole filled with epoxy.

Steel piles are most susceptible to corrosion in the upper portions where oxygen is available to support corrosion. Further, the portion of steel piles embedded in concrete pile caps forms a dissimilar environment corrosion cell with the steel in the soil. In this cell the steel in soil is anodic (corroding) to the steel in the cap (protected).

Steel piles may be protected by coating them with coal tar epoxy. Although this tough coating may be damaged somewhat during driving, it will provide a great deal of protection. Some corrosion can be tolerated without appreciably affecting the pile's structural strength. The coating need not be applied to those portions of the piles driven into undisturbed soil permanently below the water table where oxygen is not available.

A coating is important on portions of the piles to be embedded in concrete in order to minimize the dissimilar environment effect on non-embedded steel. Significant coating damage should be repaired after driving where the piles will be embedded in concrete and on several lineal feet of adjacent pile. It would be desirable to prevent metal to metal contact between the piles and reinforcing steel.

The interior of pipe piles may be protected by filling with concrete or sealing both ends.

For protection of reinforcing steel, dense concrete mixes with a water/cement ratio not exceeding 0.45, will minimize concrete permeability. A silica fume additive can be used to increase the resistivity of the concrete and reduce chloride and oxygen diffusion into the concrete. A calcium nitrite corrosion inhibiting additive, such as W. R. Grace DCI, can be used to reduce rebar corrosion. Fusion bonded epoxy coating of the reinforcing steel, in combination with waterproofing of the soil side concrete, possibly in conjunction with cathodic protection, may also be considered.

Concrete in contact with these soils should be made with type 2 cement.

Concrete cover over reinforcing steel in concrete contacting these soils should be at least 2 inches thick if placed against forms and 3 inches thick if placed against the earth.

Steel tie-back rods for retaining walls may be protected by using corrosion inhibiting grease and installing in nonmetallic sleeves or conduits to prevent soil contact, by encasing rods in concrete or grout, cathodic protection, or a combination of these.

Steel sheet piling should be coated with a suitable dielectric coating, such as coal tar epoxy, and can be cathodically protected. If cathodically protected, the piling should be made electrically continuous.

Cast or ductile iron pipe, valves, and fittings should be encased in an 8 mil polyethylene tube or wrap per AWWA Standard C105/ANSI 21.5. Underground iron pipe should be electrically insulated from dissimilar metals, cement-mortar or concrete coated iron, and above ground iron pipe.

Due to the high chlorides and dissimilarities in the shallow soils, buried copper tubing should be coated with plastic pipe wrapping tape over primer and cathodically protected. Insulation over buried hot water copper tubing should be carefully sealed. If cathodically protected, insulating joints should be installed just above the floor in all risers from underground.

No special precautions are required for plastic utilities placed underground from a corrosion viewpoint. However, any iron valves or fittings should be protected as mentioned above.

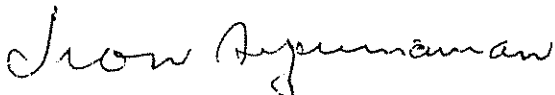
Sand would be better than the existing soils for bedding and backfill of metallic piping from a corrosion standpoint.

Where metallic pipelines penetrate concrete structures such as building floors or walls, plastic sleeves, rubber seals, or other dielectric material should be used to prevent pipe contact with the concrete and reinforcing steel.

On any type of pipe, bare steel appurtenances such as bolts, joint harnesses, or flexible couplings should be coated with a coal tar or rubber based mastic after assembly.

The scope of this study was limited to a determination of soil corrosivity and its general effects on materials likely to be used for construction. If the architect and/or engineers desire more specific information, designs, specifications, or review of design, we will be happy to work with them as a separate phase of this project.

Respectfully submitted,
M. J. SCHIFF & ASSOCIATES



Leon Arzumanian

SM
Enc: Table 1
L27

Table 1 - LABORATORY TESTS ON SOIL SAMPLES

Page 1 of 2

Boring and Depth	Soil Type	Soil Resistivity		pH		-----Chemical Analysis in mg/kg (ppm) of dry soil-----										Sulfide S	Sulfate SO ₄	Redox MV
		As Rec'd	Sat'd	Ca	Mg	Na	HCO ₃	Chloride	Sulfate	Sulfide	Sulfate	Chloride	Sulfate	Sulfate	Sulfate			
B1	3.5' fill	1,400	270	7.9	200	48	1,368	610	920	1,800								
B2	29.5' clay	94	90	8.3	160	24	6,222	854	9,275	580								
B3	15.5' silt	98	79	7.6	160	72	2,944	1,464	4,036	195								
B4	4.5' fill	100	100	7.5	240	48	3,232	1,708	3,540	1,225								
B6	34.5' sand-silt	610	390	7.3	trace	trace	920	610	991	75								
B14	36.5' clay	1,200	910	7.7	trace	trace	230	122	354	trace	present							

Playa Vista Development
 Los Angeles, California
 Your #AE-88473, MJS&A #89012
 F8

Table 1 - LABORATORY TESTS ON SOIL SAMPLES

Page 2 of 2

Boring and Depth	Soil Type	Soil Resistivity		-----Chemical Analysis in mg/kg (ppm) of dry soil-----										Sulfide S	Sulfate SO ₄	Redox MV
		As Rec'd	Sat'd	pH	Ca	Mg	Na	HCO ₃	Chloride	Sulfate	Sulfide	Sulfate	Chloride			
B15	4.5' silt-clay	360	120	7.5	600	96	3,496	610	5,876	825						
B17	1.5' fill	2,800	570	7.2	40	trace	472	122	779	55						
B18	24.5' clay	82	65	7.8	600	72	7,383	3,294	9,912	2,150	present					-10
B19	6.5' fill	86	86	7.9	160	trace	1,380	976	1,274	975						
B20	3.5' fill	320	110	7.5	240	48	5,428	1,342	6,726	2,150						

Carbonate = 0 for all samples

Playa Vista Development
 Los Angeles, California
 Your #AE-88473, MJS&A #89012
 F8

APPENDIX B
CONE PENETRATION TESTS

The five Cone Penetration Tests (CPTs) were performed to depths of approximately 66 to 72 feet below grade at the locations shown on Plate 1. The testing was performed for us by Pioneer Drilling Company.

The CPT consists of pushing a cone-tipped probe into a soil deposit and recording the resistance of the soil to that penetration. The test was conducted in general accordance with ASTM Test Designation D-3441. The test equipment consists of a cone assembly equipped with a piezometer at the tip, a series of hollow sounding rods, a hydraulic frame to push the cone and rods into the soil, an electronic data processing unit, and a truck to transport the test equipment and provide the needed thrust capacity.

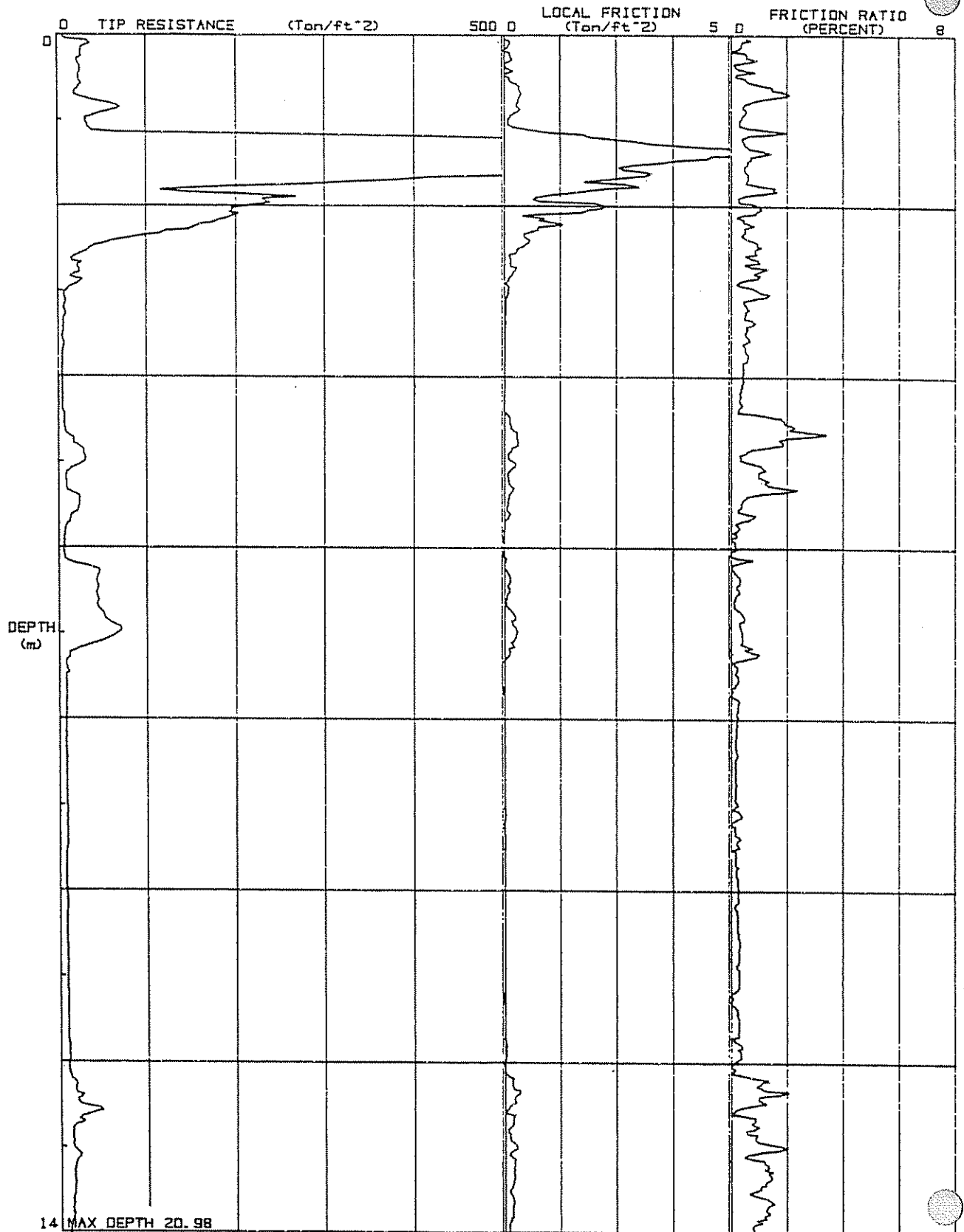
The probe ("penetrometer") consists of a conical tip with a 60 degree apex angle and a cylindrical friction sleeve. The interior of the device is instrumented with strain gauges allowing simultaneous measurements of cone and sleeve resistance during penetration. The pressure due to the head of ground water table above the tip of the cone and any excess pore water pressures generated due to penetration of the cone into the soils can be measured by an electronic piezometer installed at the tip of the cone. Electric signals from the strain gauges and the piezometer are transmitted by cable to the data processing unit. The cone assembly used on this project has a cross-sectional area of 10 square centimeters, and a sleeve surface area of 150 square centimeters. The penetrometer system has a total thrust capacity of about 20 tons.

Output quantities for the cone tip penetration resistance and (local) sleeve friction are simultaneously recorded in units of tons per square foot (tsf) versus depth in units of meters (m). The pore pressure (in units of pounds per square inch) is also recorded versus depth. The recording apparatus is also designed to calculate and record the ratio of friction resistance to tip resistance (known as the friction ratio) and the ratio of pore pressure to tip resistance (known as the pore pressure ratio). Plots of these five quantities are presented on Plates B-1.1 through B-1.5. The cone tip resistance and friction ratio were evaluated using published literature in order to classify the subsurface soils. The interpreted results of the tests are presented on Plates B-2.1 through B-2.5.

-oOo-

C.P.T. - 1

CLIENT: HOWARD HUGHES PROPERTIES
 JOB NO. AE-88473
 DATE: JANUARY 30, 1989
 ELEVATION: 14.9 FT *

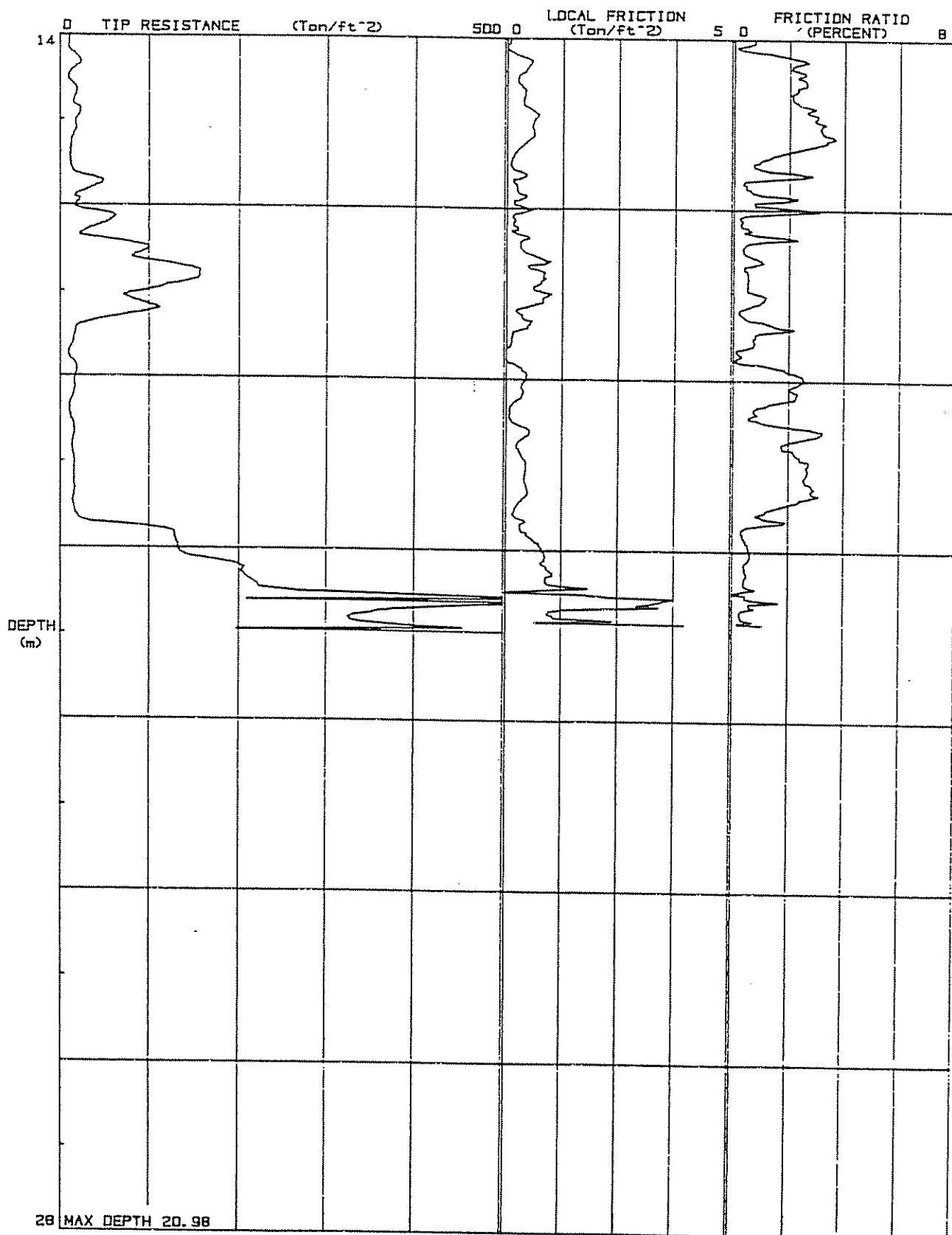


* Elevations provided by Psomas and Associates.

(CONTINUED)

C.P.T. - 1

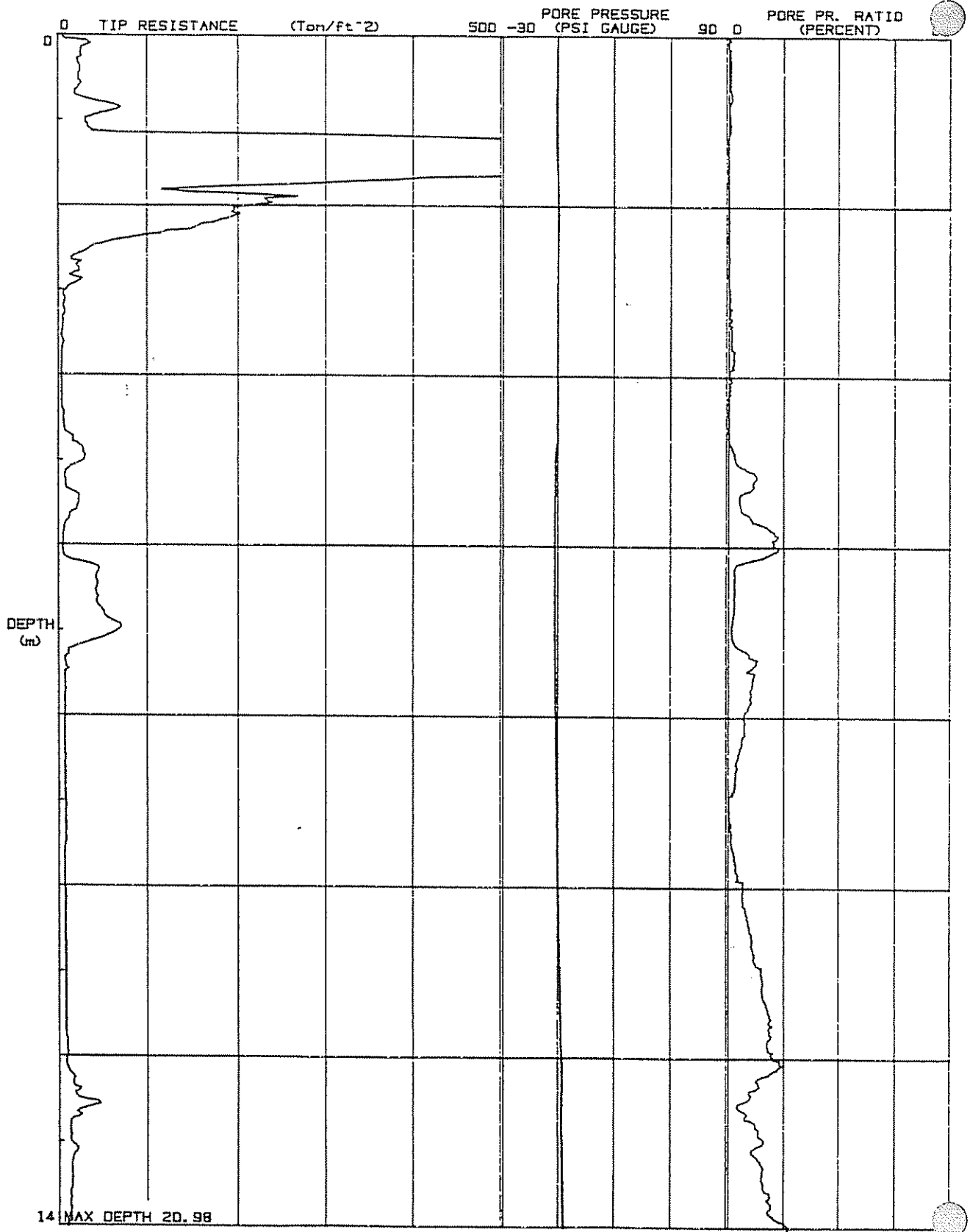
CLIENT: HOWARD HUGHES PROPERTIES
 JOB NO. AE-88473
 DATE: JANUARY 30, 1989
 ELEVATION: 14.9 FT



(CONTINUED)

C.P.T. - 1

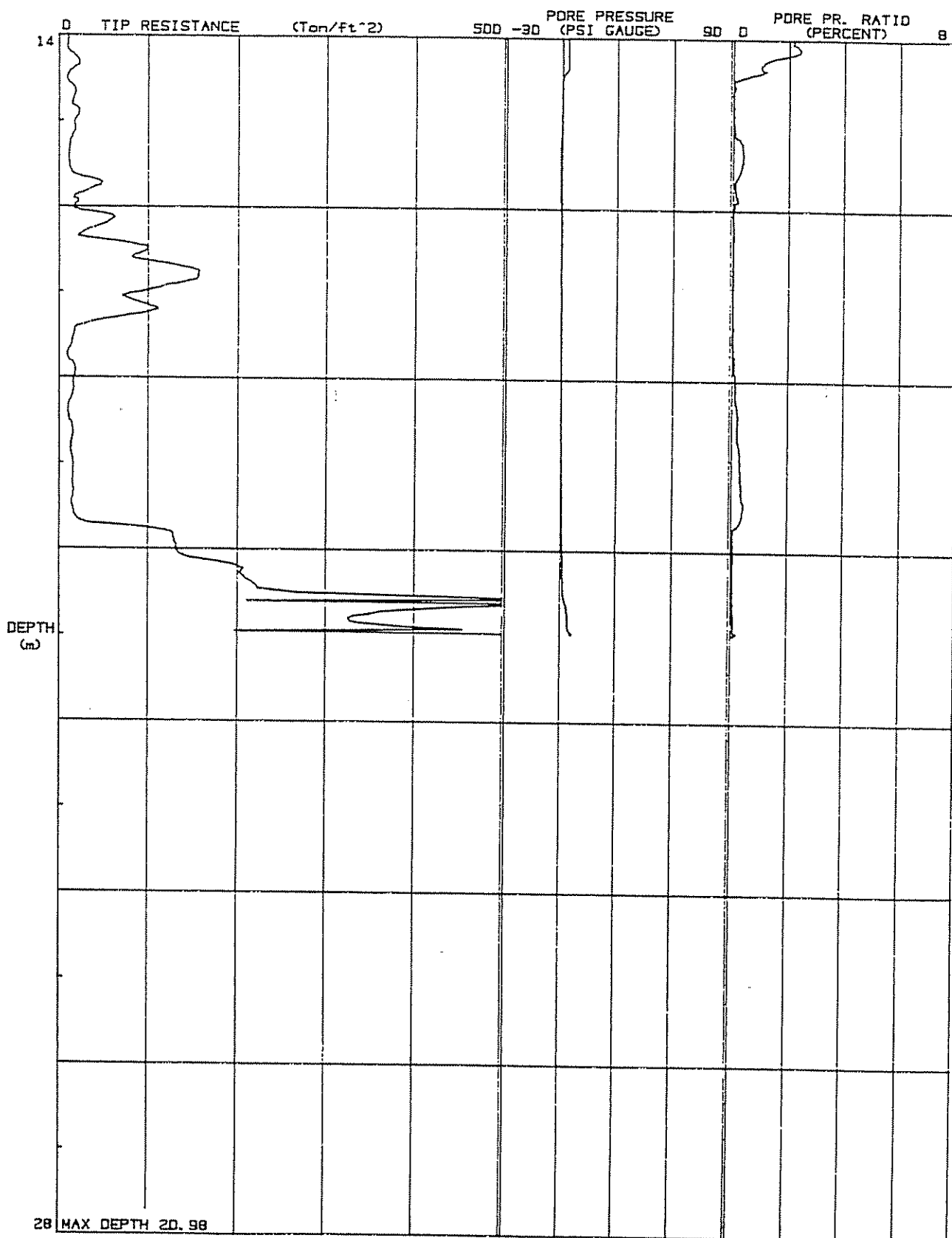
CLIENT: HOWARD HUGHES PROPERTIES
JOB NO. AE-88473
DATE: JANUARY 30, 1989
ELEVATION: 14.9 FT



(CONTINUED)

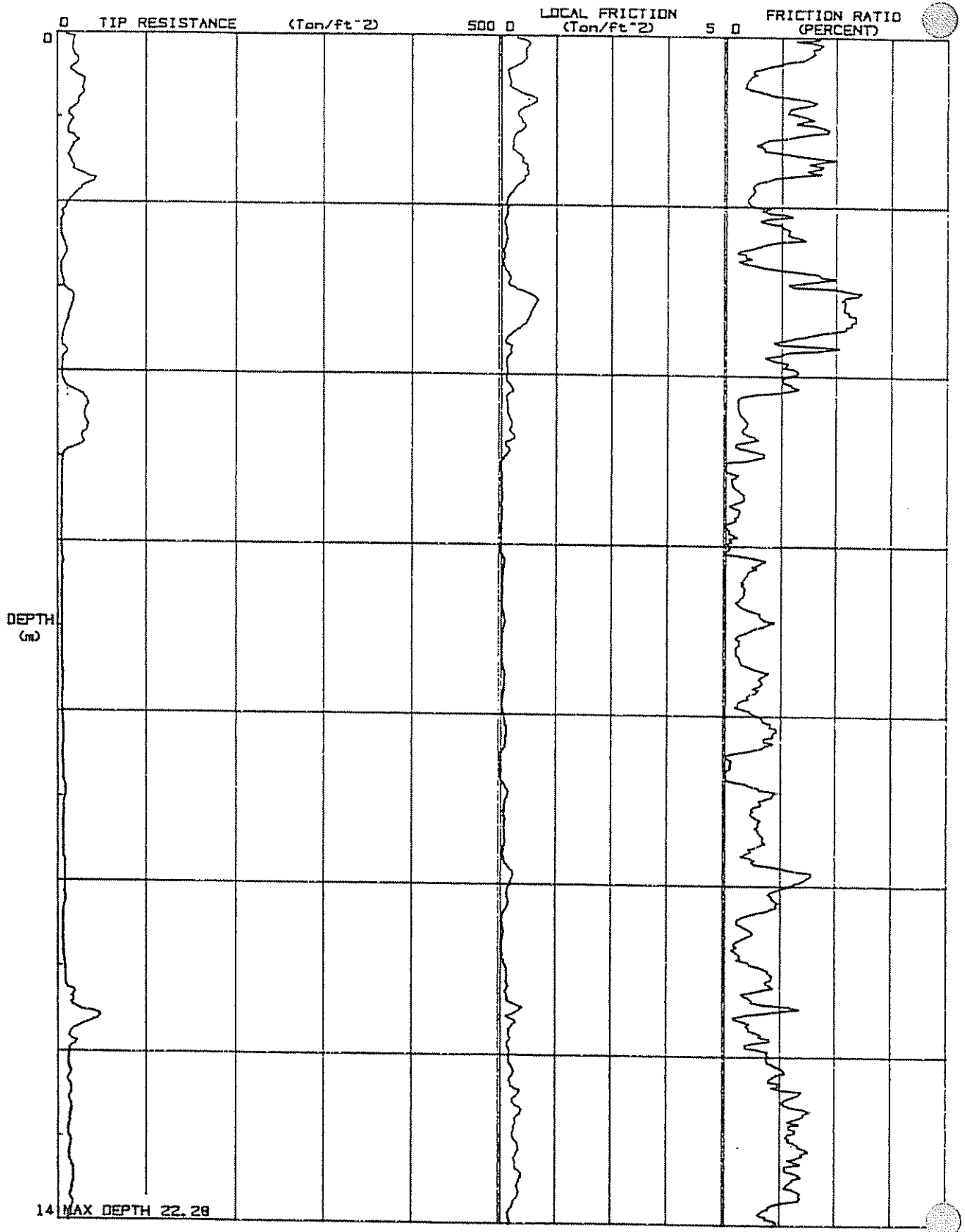
C.P.T. - 1

CLIENT: HOWARD HUGHES PROPERTIES
 JOB NO. AE-88473
 DATE: JANUARY 30, 1989
 ELEVATION: 14.9 FT



C.P.T. - 14

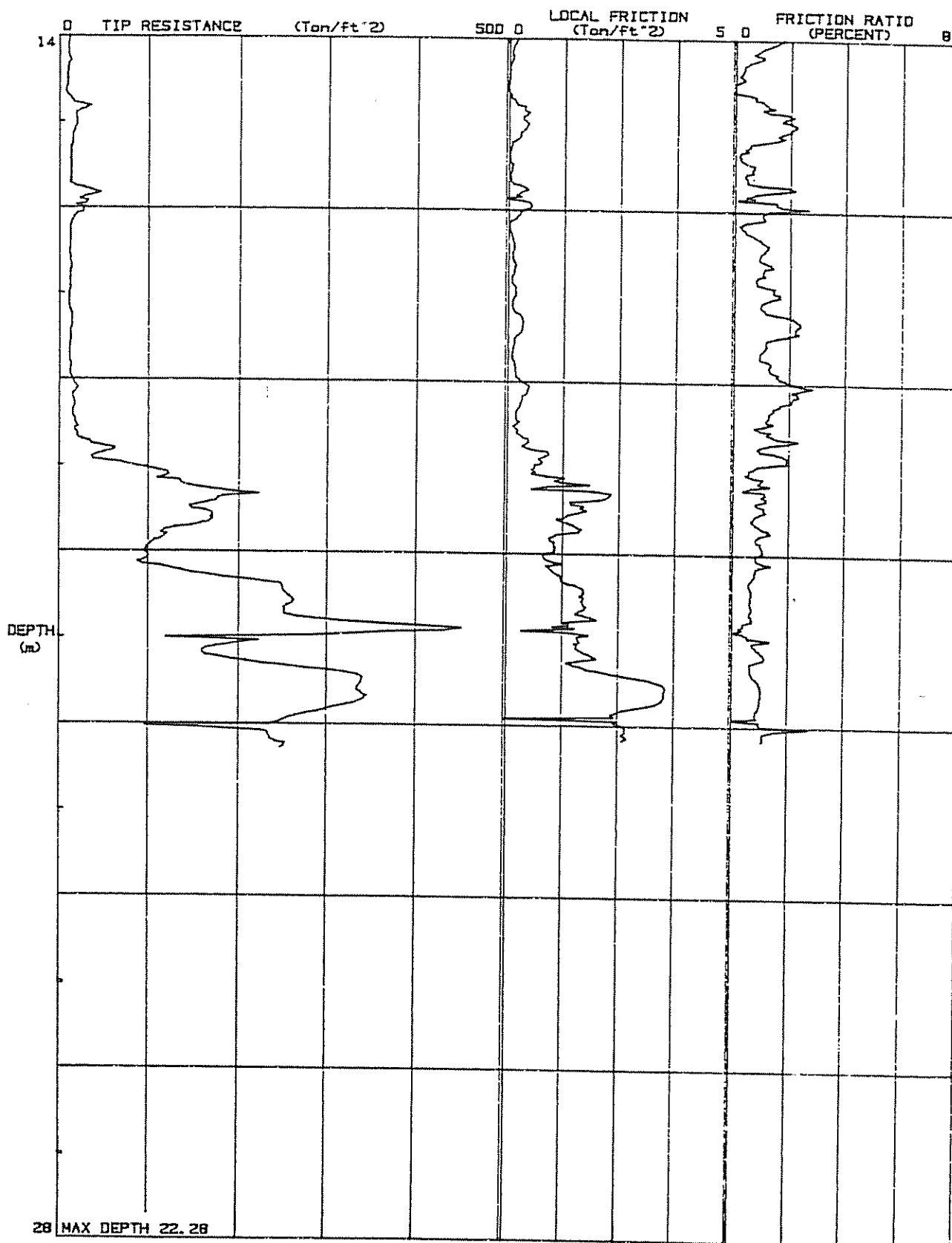
CLIENT: HOWARD HUGHES PROPERTIES
JOB NO: AE-88473
DATE: JANUARY 30, 1989
ELEVATION: 11.8 FT



(CONTINUED)

C.P.T. - 14

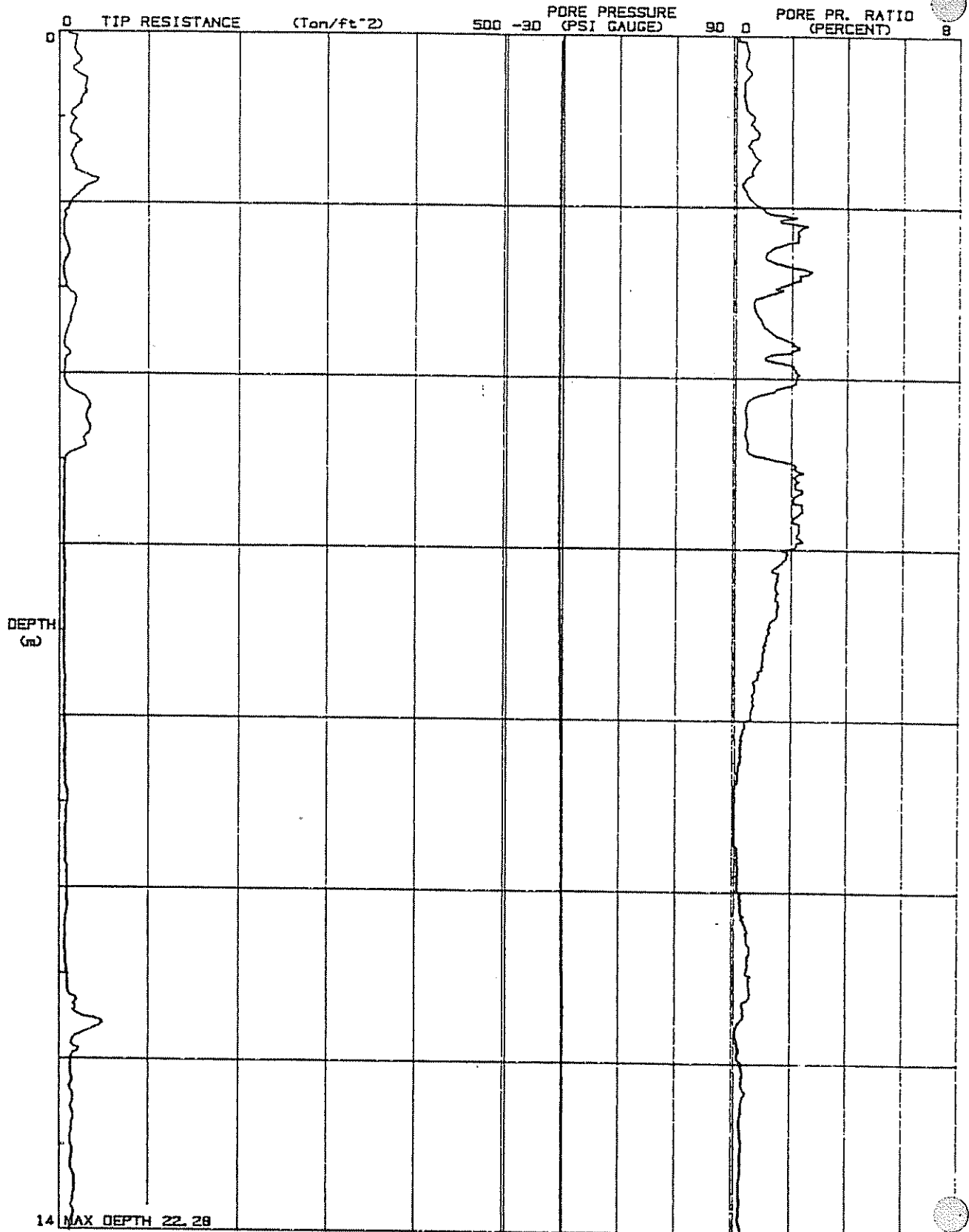
CLIENT: HOWARD HUGHES PROPERTIES
JOB NO. AE-88473
DATE: JANUARY 30, 1989
ELEVATION: 11.8 FT



(CONTINUED)

C.P.T. - 14

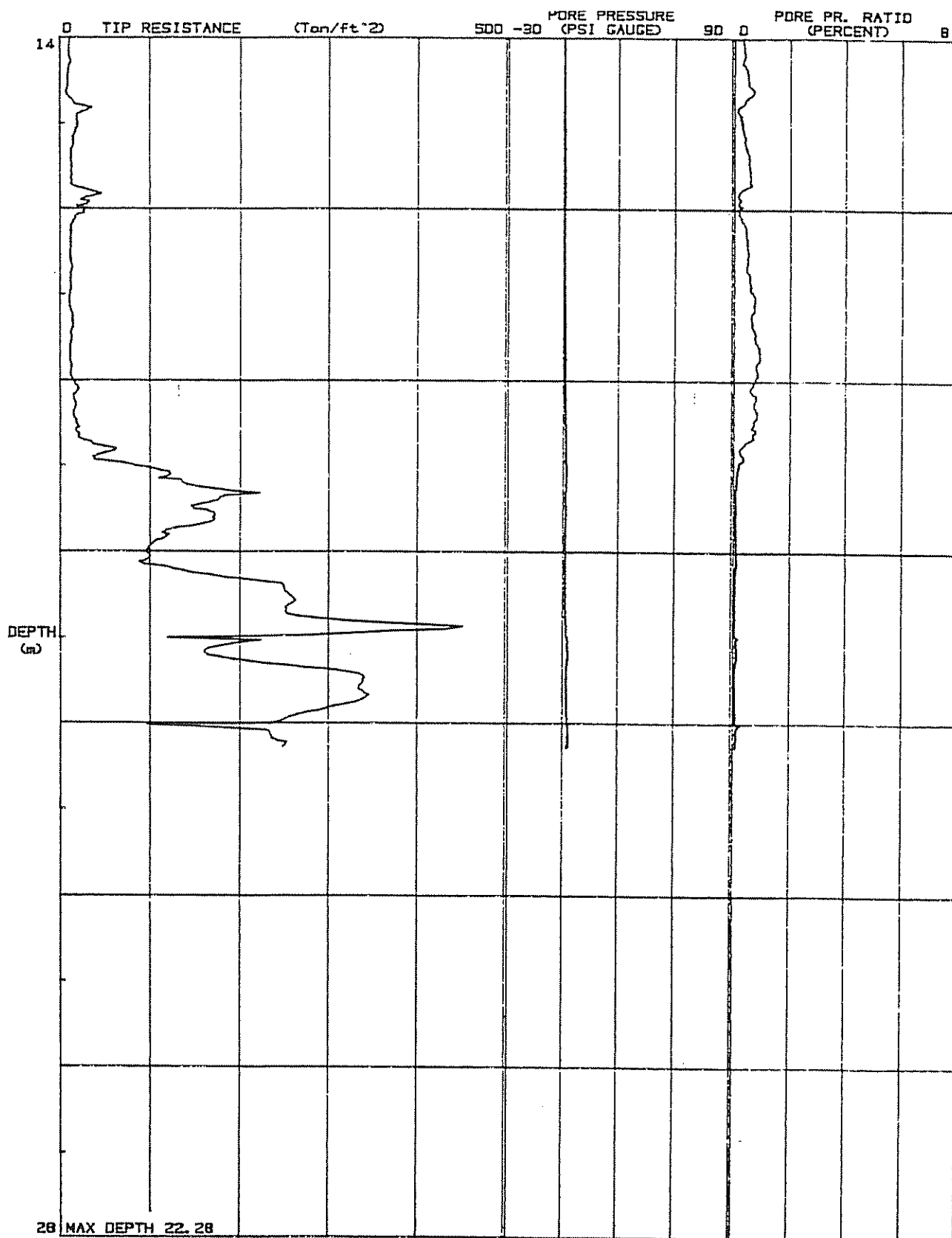
CLIENT: HOWARD HUGHES PROPERTIES
JOB NO: AE-88473
DATE: JANUARY 30, 1989
ELEVATION: 11.8 FT



(CONTINUED)

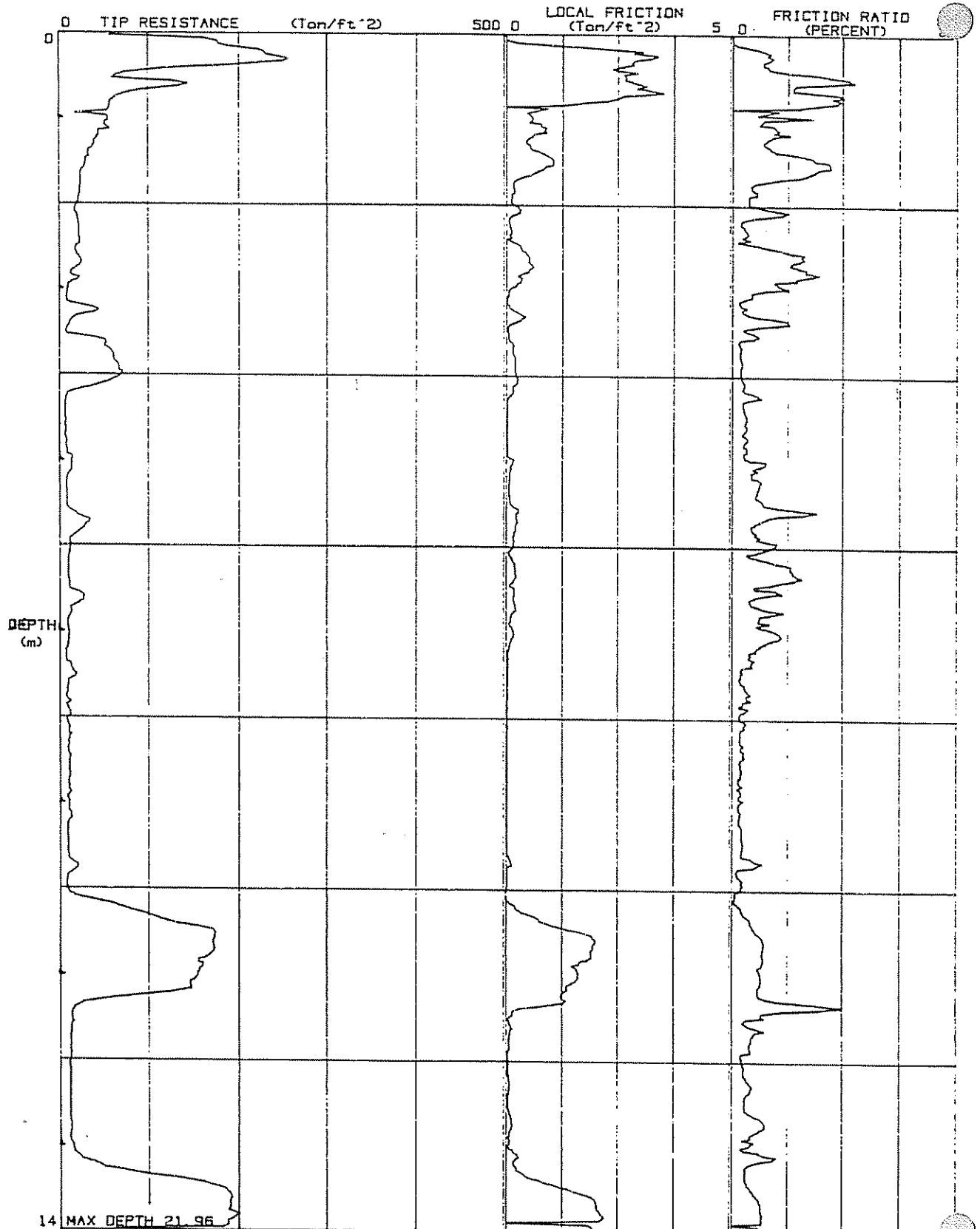
C.P.T. - 14

CLIENT: HOWARD HUGHES PROPERTIES
JOB NO. AE-88473
DATE: JANUARY 30, 1989
ELEVATION: 11.8 FT



C.P.T. - 17

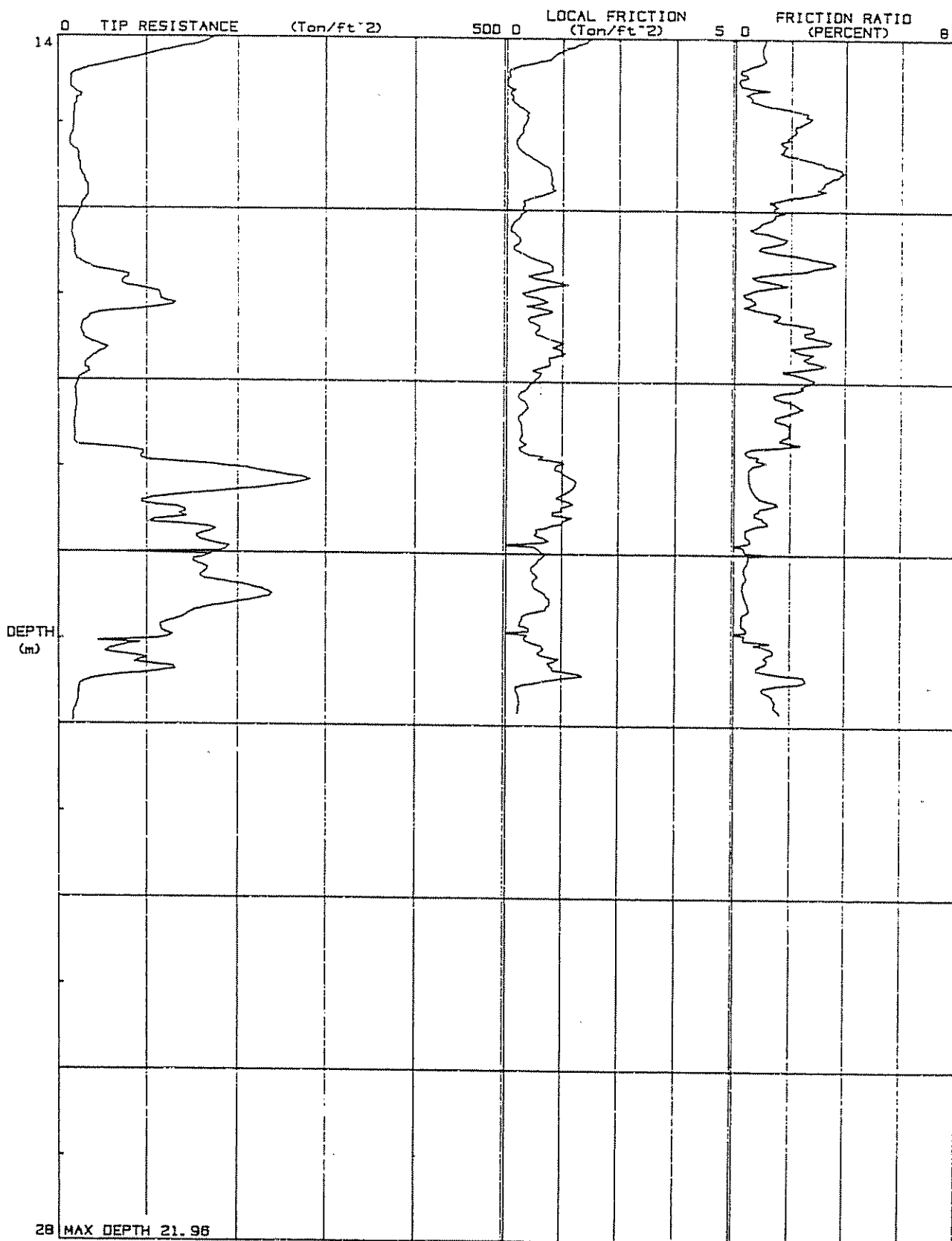
CLIENT: HOWARD HUGHES PROPERTIES
JOB NO: AE-88473
DATE: JANUARY 30, 1989
ELEVATION: 18.2 FT



(CONTINUED)

C.P.T. - 17

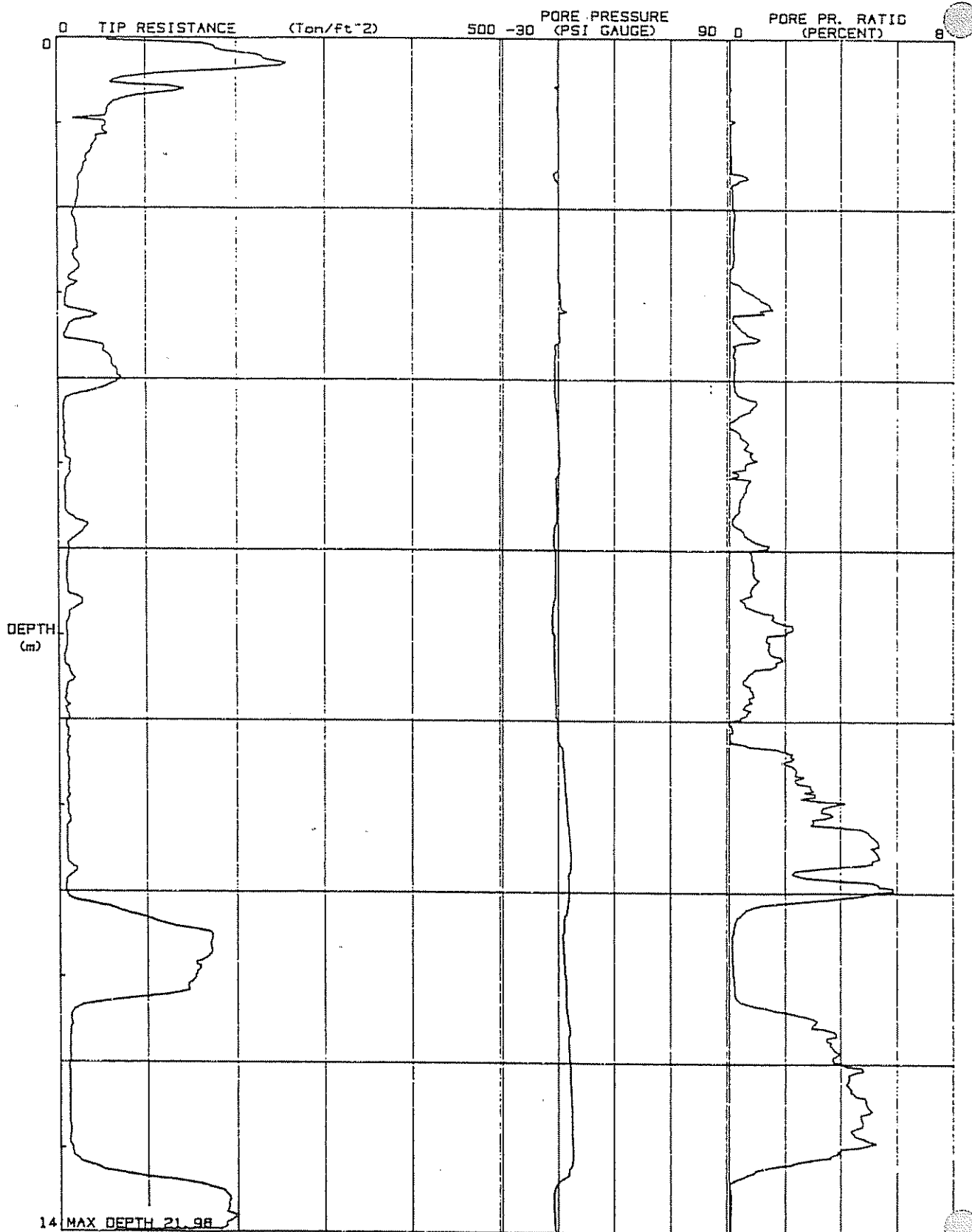
CLIENT: HOWARD HUGHES PROPERTIES
JOB NO. AE-88473
DATE: JANUARY 30, 1989
ELEVATION: 18.2 FT



(CONTINUED)

C.P.T. - 17

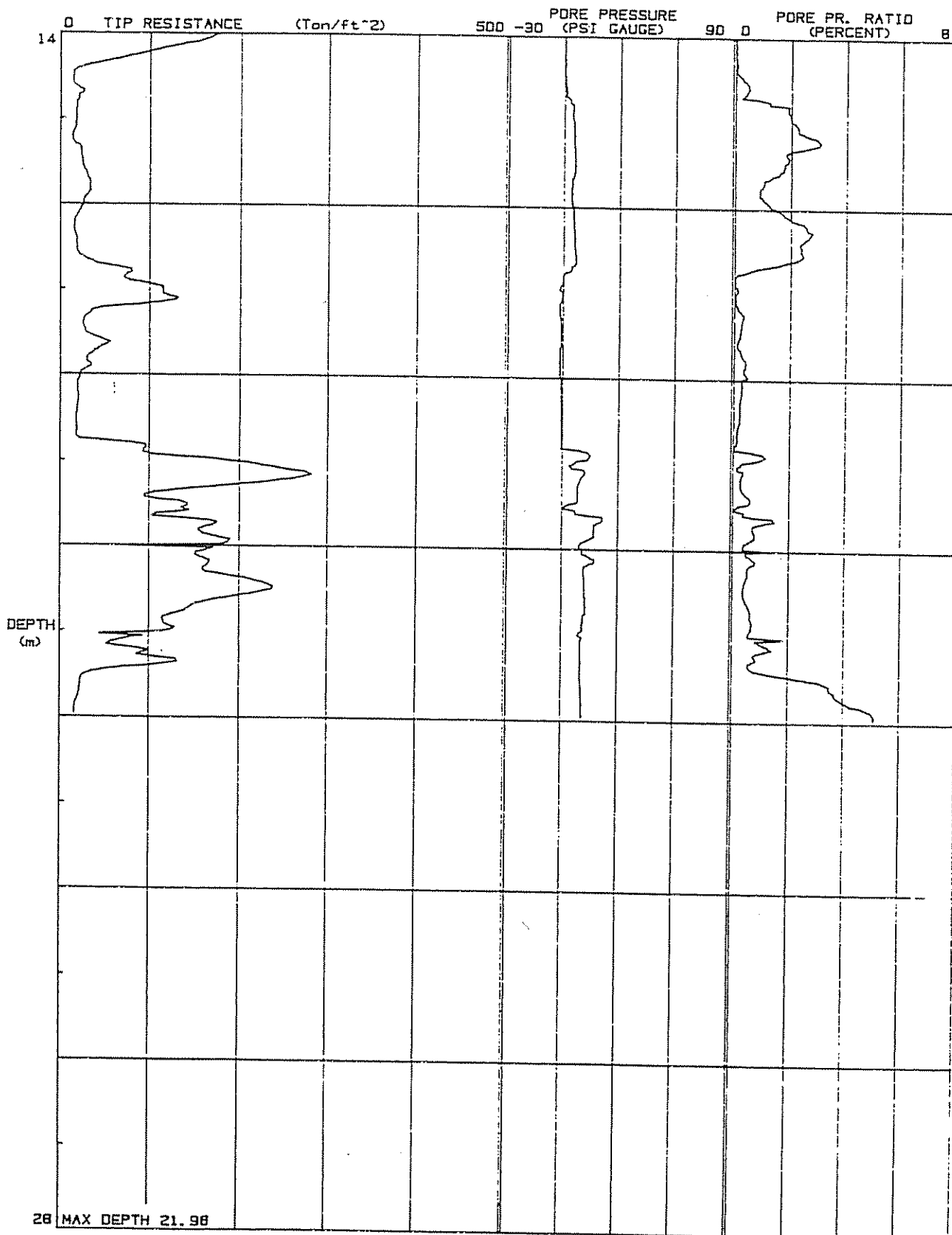
CLIENT: HOWARD HUGHES PROPERTIES
JOB NO. AE-88473
DATE: JANUARY 30, 1989
ELEVATION: 18.2 FT



(CONTINUED)

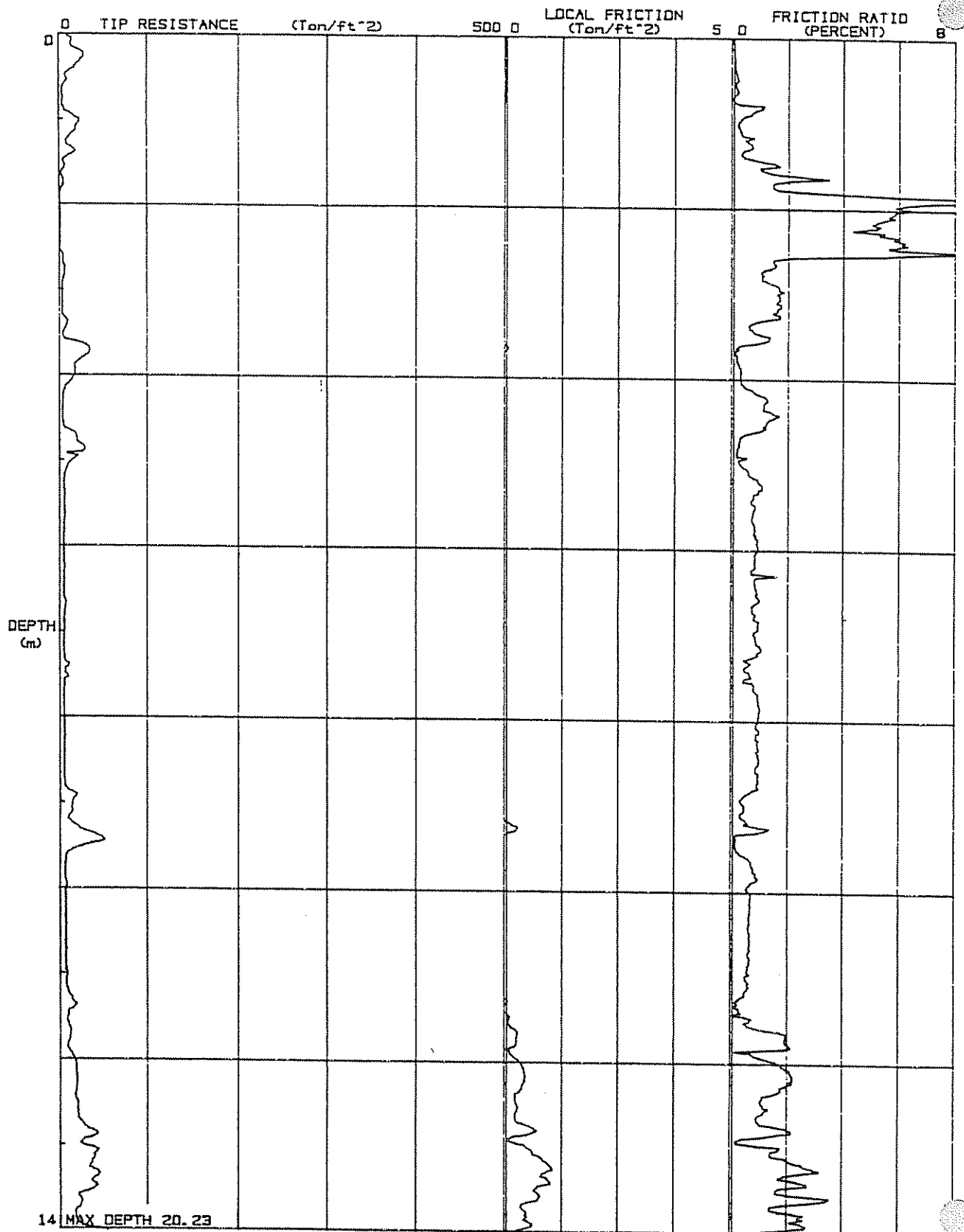
C.P.T. - 17

CLIENT: HOWARD HUGHES PROPERTIES
JOB NO. AE-88473
DATE: JANUARY 30, 1989
ELEVATION: 18.2 FT



C.P.T. - 18

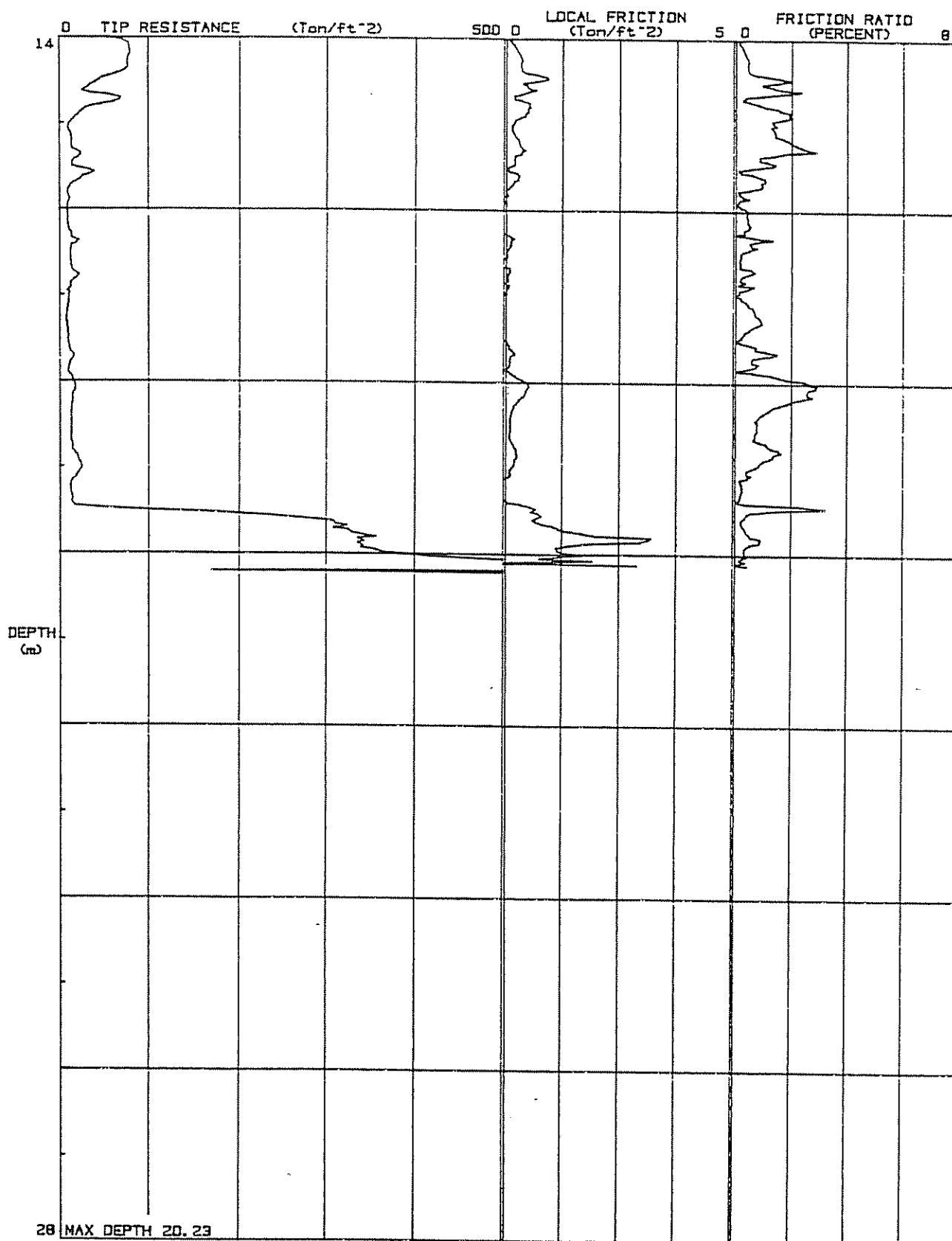
CLIENT: HOWARD HUGHES PROPERTIES
JOB NO. AE-88473
DATE: JANUARY 30, 1989
ELEVATION: 12.7 FT



(CONTINUED)

C.P.T. - 18

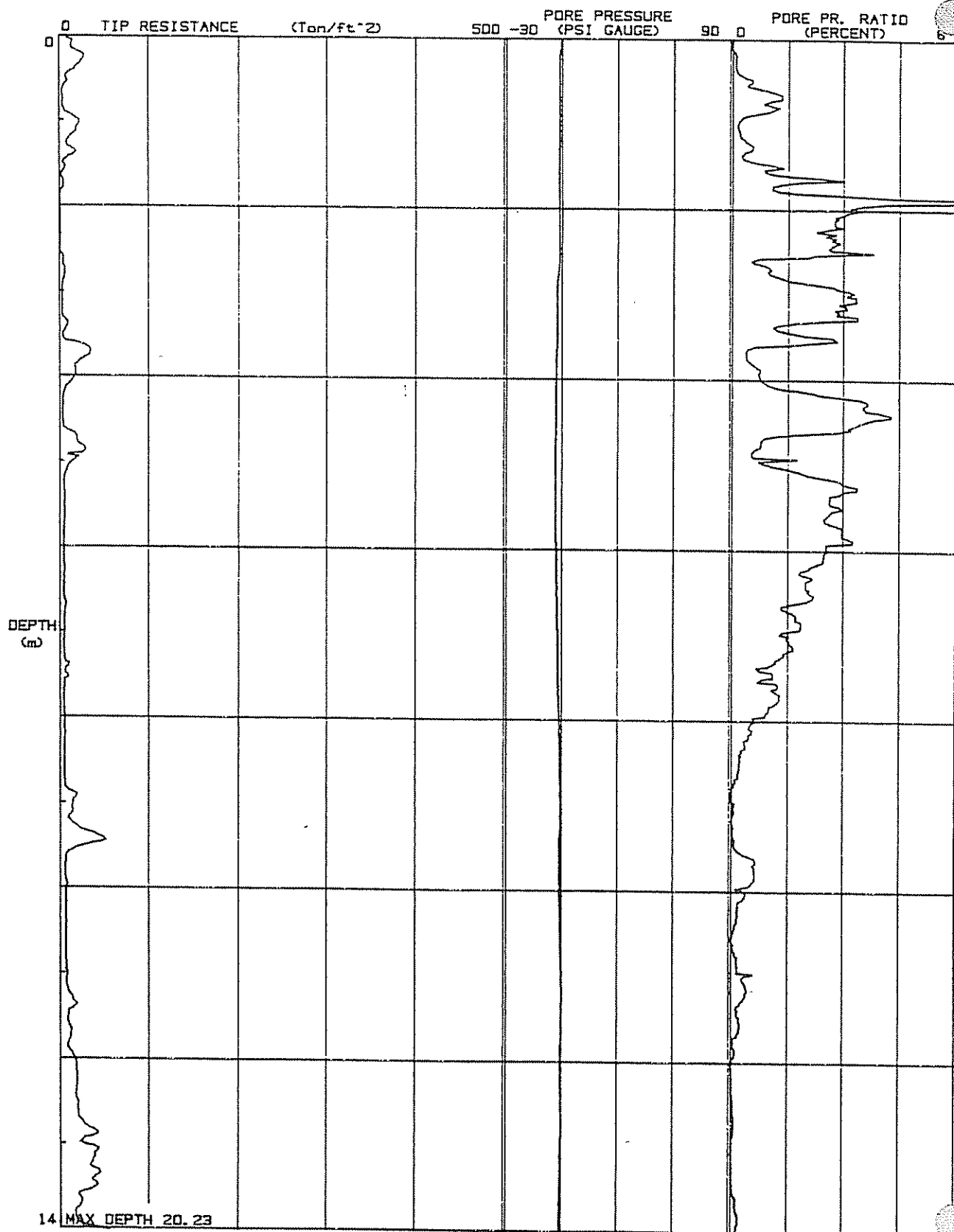
CLIENT: HOWARD HUGHES PROPERTIES
JOB NO. AE-88473
DATE: JANUARY 30, 1989
ELEVATION: 12.7 FT



(CONTINUED)

C.P.T. - 18

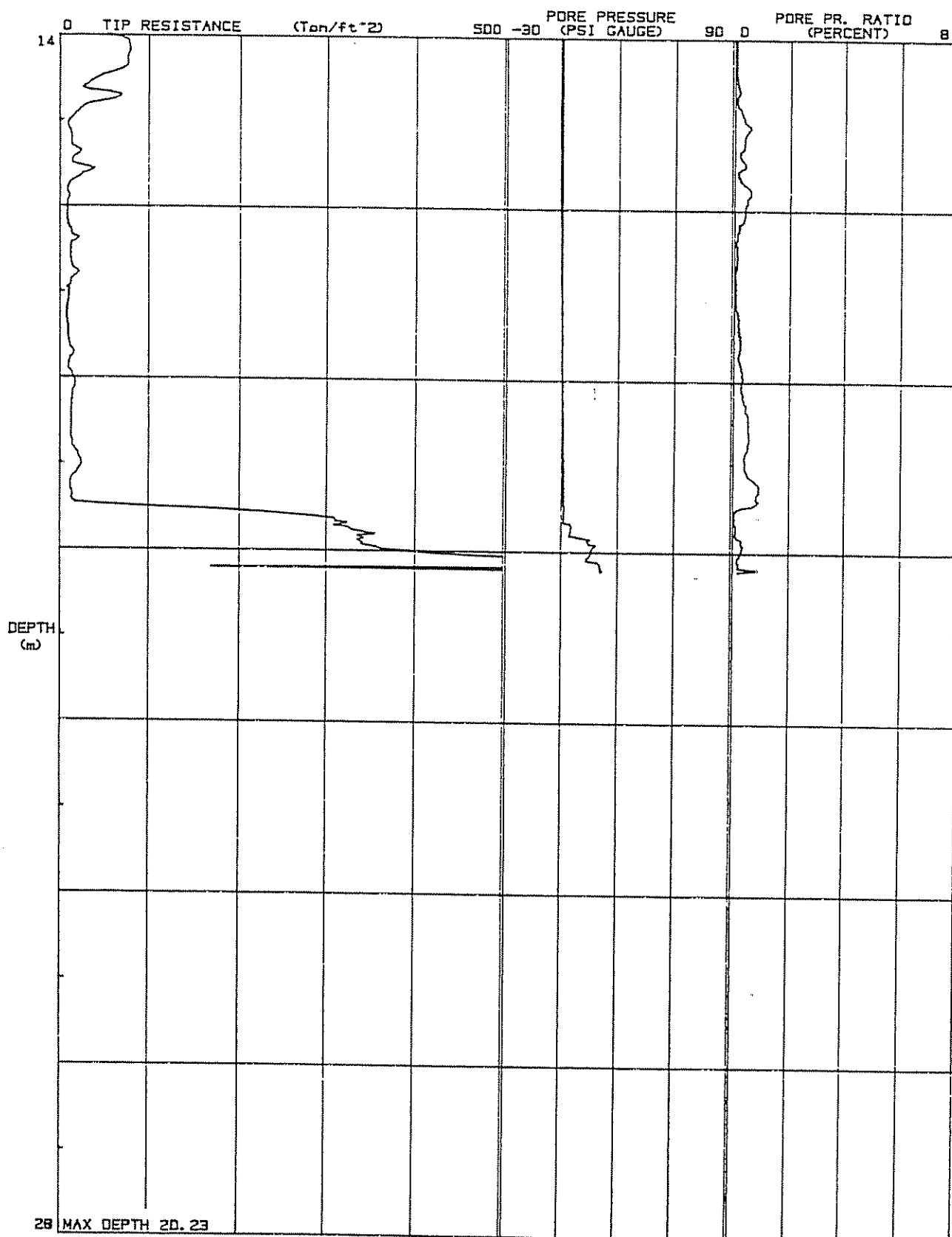
CLIENT: HOWARD HUGHES PROPERTIES
 JOB NO. AE-88473
 DATE: JANUARY 30, 1989
 ELEVATION: 12.7 FT



(CONTINUED)

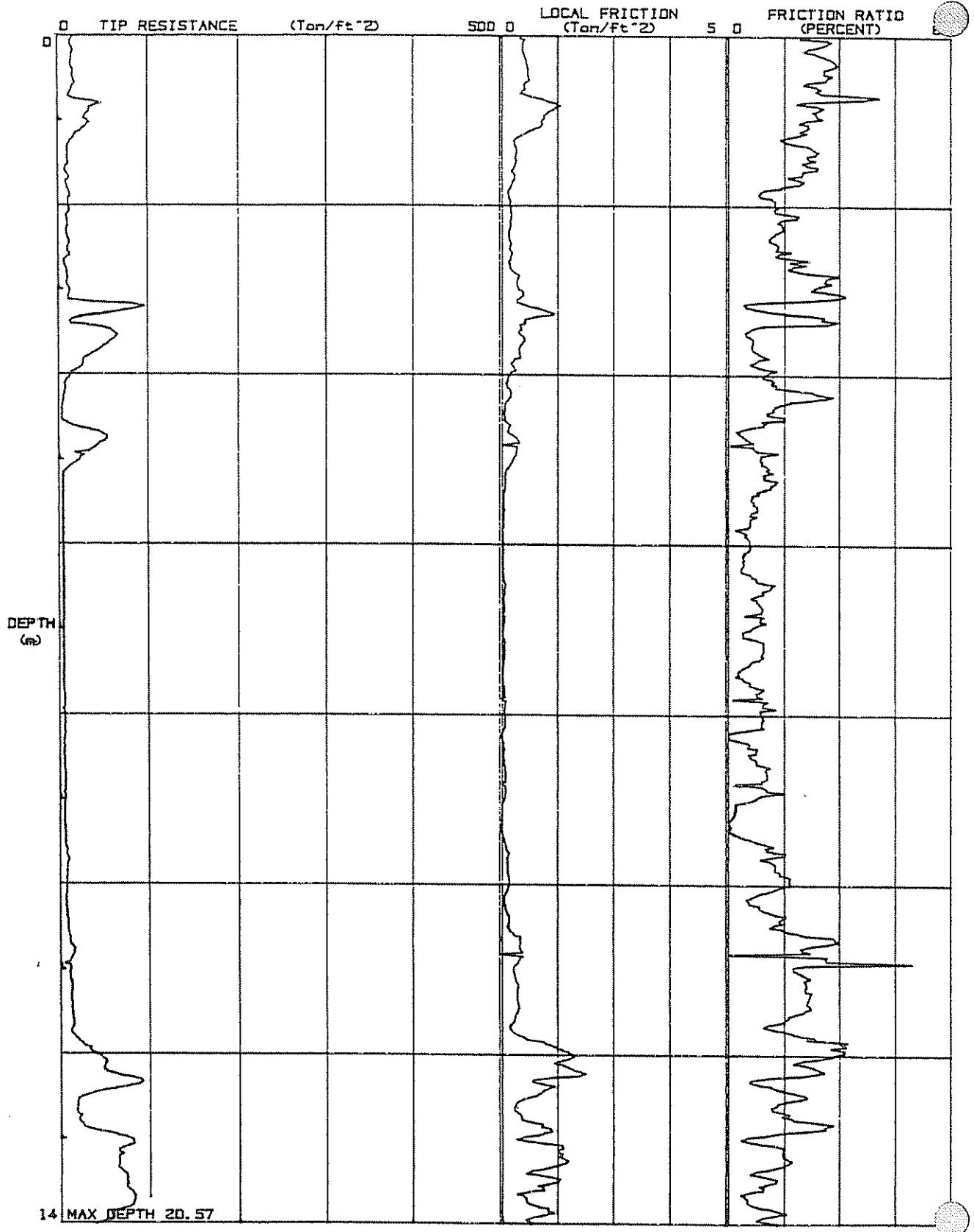
C.P.T. - 18

CLIENT: HOWARD HUGHES PROPERTIES
JOB NO. AE-88473
DATE: JANUARY 30, 1989
ELEVATION: 12.7 FT



C.P.T. - 20

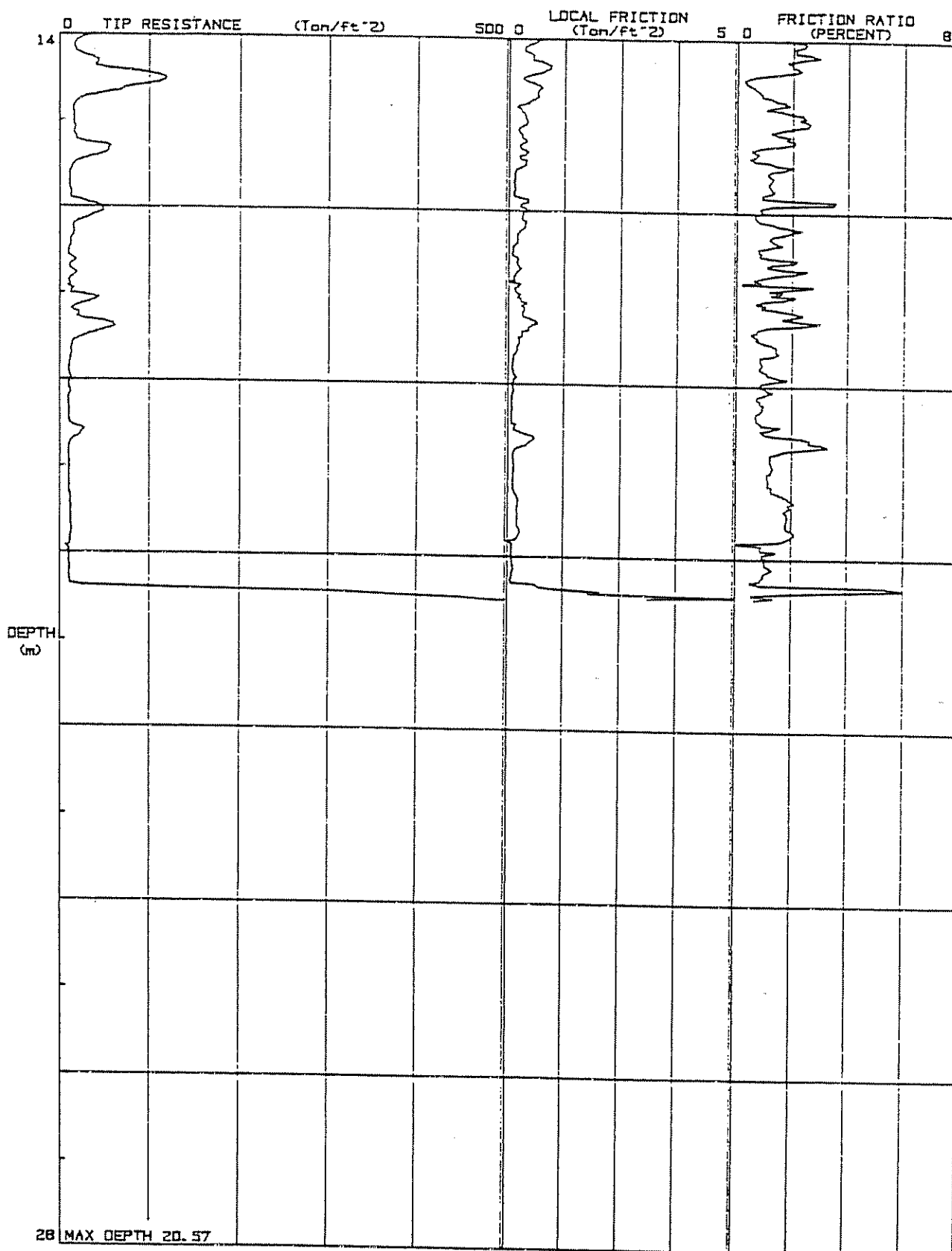
CLIENT: HOWARD HUGHES PROPERTIES
JOB NO: AE-88473
DATE: JANUARY 30, 1989
ELEVATION: 12.3 FT



(CONTINUED)

C.P.T. - 20

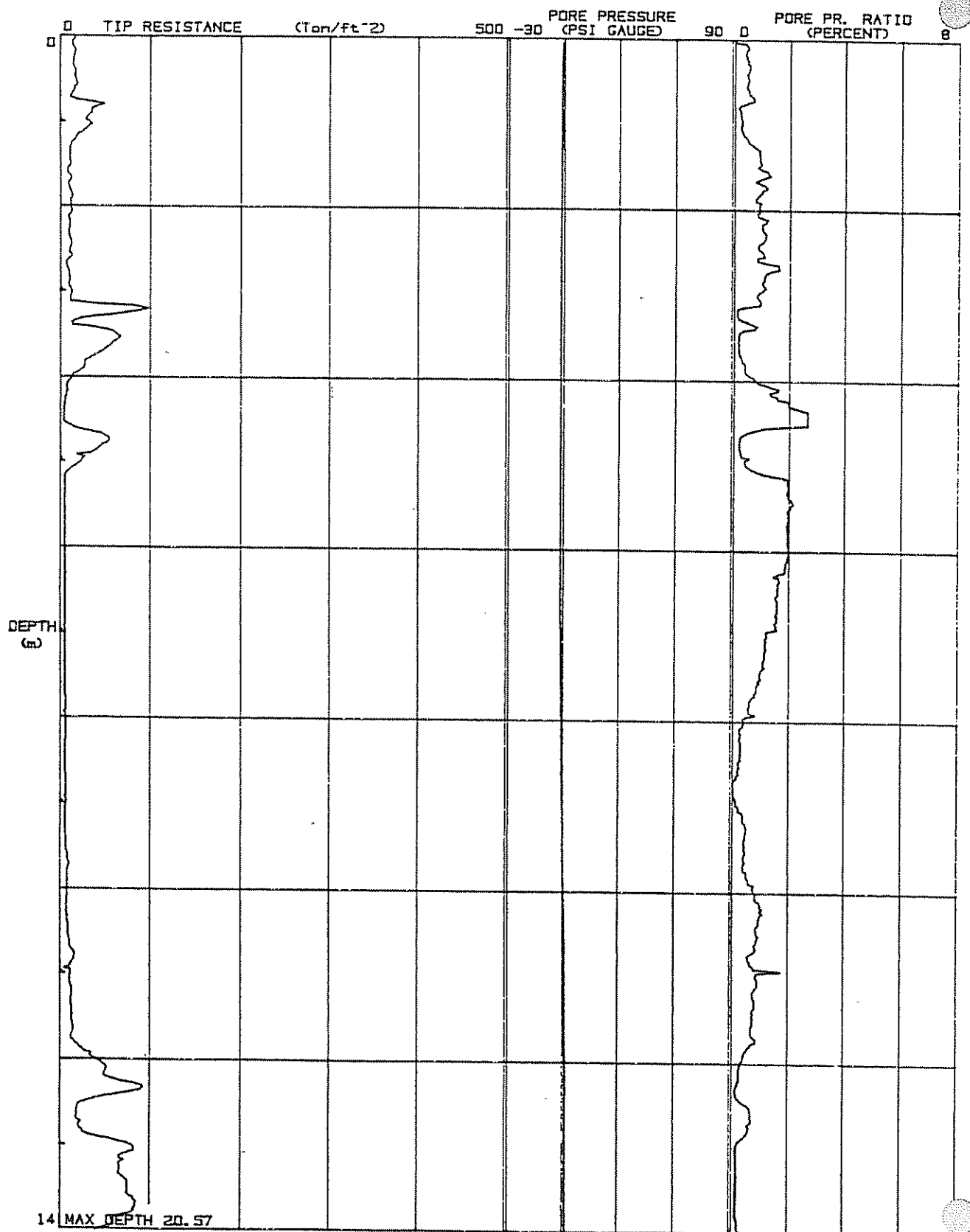
CLIENT: HOWARD HUGHES PROPERTIES
JOB NO. AE-88473
DATE: JANUARY 30, 1989
ELEVATION: 12.3 FT



(CONTINUED)

C.P.T. - 20

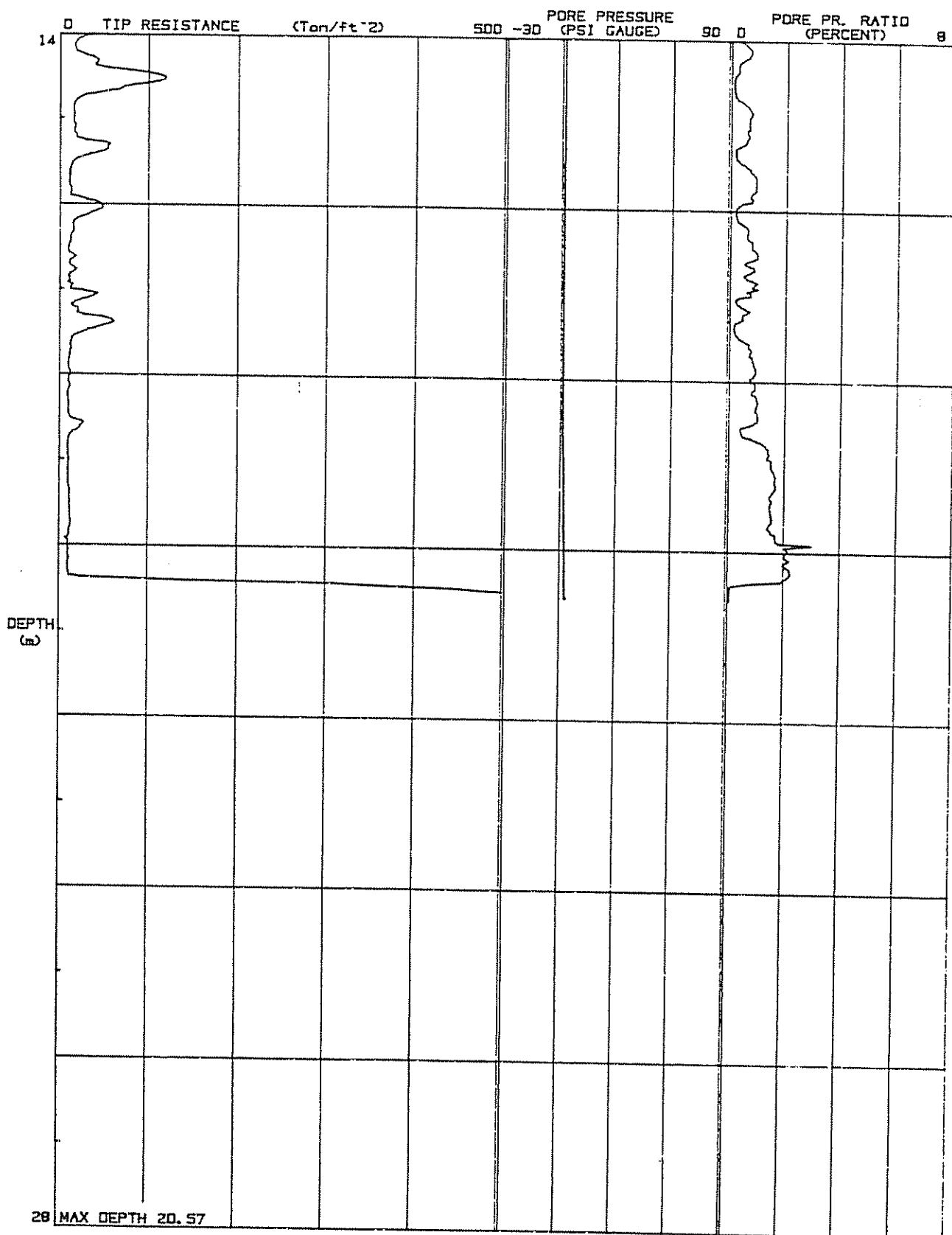
CLIENT: HOWARD HUGHES PROPERTIES
JOB NO. AE-88473
DATE: JANUARY 30, 1989
ELEVATION: 12.3 FT



(CONTINUED)

C.P.T. - 20

CLIENT: HOWARD HUGHES PROPERTIES
JOB NO. AE-88473
DATE: JANUARY 30, 1989
ELEVATION: 12.3 FT



INTERPRETED CONE PENETRATION TEST DATA

C. P.T. - 1

CLIENT: HOWARD HUGHES PROPERTIES
 JOB NO: AE-88473
 DATE: JANUARY 30, 1989
 ELEVATION: 14.9 FT*
 UNIT WEIGHT OF SOIL: 125 PCF
 DEPTH TO GROUND WATER: 11 FT

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF
0.1	6.1	0.0	0.49	SANDY SILT AND SILT	2	8	0.04	--	0.01	--
0.3	33.3	0.1	0.42	SILTY SAND	9	29	0.09	--	0.02	--
0.4	30.5	0.1	0.36	SILTY SAND	8	24	0.09	--	0.03	--
0.6	26.6	0.0	0.04	SILTY SAND	7	20	0.10	--	0.04	--
0.8	25.3	0.2	0.59	SILTY SAND	7	19	0.08	--	0.05	--
0.9	22.1	0.2	0.95	SILTY SAND	6	16	0.07	--	0.06	--
1.1	23.1	0.0	0.13	SILTY SAND	6	15	0.10	--	0.07	--
1.2	25.8	0.1	0.27	SILTY SAND	7	17	0.09	--	0.08	--
1.4	25.8	0.2	0.85	SILTY SAND	7	17	0.07	--	0.09	--
1.6	23.8	0.0	0.17	SILTY SAND	6	14	0.09	--	0.10	--
1.7	23.6	0.1	0.59	SILTY SAND	7	14	0.08	--	0.11	--
1.9	27.2	0.3	1.07	SILTY SAND	8	17	0.07	--	0.12	--
2.1	22.4	0.3	1.52	SANDY SILT AND SILT	7	15	0.05	--	0.13	--
2.2	19.0	0.4	2.15	SANDY SILT AND SILT	7	14	0.03	--	0.14	--
2.4	23.5	0.4	1.57	SANDY SILT AND SILT	7	15	0.05	--	0.15	--
2.6	46.2	0.3	0.71	SILTY SAND	12	24	0.10	--	0.16	--
2.7	68.0	0.3	0.43	SAND	11	21	1.28	--	0.17	--
2.9	68.0	0.4	0.56	SAND	11	22	1.06	--	0.18	--
3.1	48.3	0.2	0.50	SAND	9	18	0.71	--	0.19	--
3.2	31.3	0.1	0.38	SILTY SAND	8	16	0.09	--	0.20	--
3.4	31.5	0.1	0.32	SILTY SAND	8	16	0.09	--	0.21	--
3.5	34.0	0.2	0.62	SILTY SAND	9	17	0.09	--	0.22	--
3.7	39.5	0.8	1.95	SILTY SAND	12	21	0.06	--	0.23	--
3.9	277.1	1.8	0.65	SAND	39	68	1.63	--	0.24	--
4.0	555.6	2.4	0.43	SAND AND GRAVEL	63	99	3.18	48	100	0.25
4.2	616.9	3.3	0.54	SAND AND GRAVEL	72	99	2.89	48	100	0.26
4.3	657.1	4.6	0.70	SAND AND GRAVEL	80	99	2.37	48	100	0.27
4.5	637.5	9.2	1.44	SAND	96	99	1.38	48	100	0.28
4.7	638.5	4.6	0.73	SAND AND GRAVEL	78	99	2.22	48	100	0.29
4.8	654.9	3.9	0.60	SAND AND GRAVEL	77	99	2.74	48	100	0.30
5.0	555.9	2.8	0.51	SAND AND GRAVEL	65	99	2.84	48	100	0.31
5.2	524.2	2.7	0.51	SAND AND GRAVEL	62	99	2.69	48	100	0.32
5.3	520.1	3.3	0.63	SAND AND GRAVEL	64	99	2.12	48	100	0.33
5.5	418.5	2.8	0.66	SAND	54	86	1.88	48	100	0.34

* Elevations provided by Psomas and Associates.

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 1

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.
5.6	330.2	1.8	0.55	SAND	43 68	1.33	48	100	0.35	--
5.8	189.7	3.0	1.60	SAND	40 62	0.54	48	98	0.36	--
6.0	116.8	2.0	1.57	SILTY SAND	31 47	0.10	46	31	0.37	--
6.1	214.9	1.1	0.51	SAND	30 46	1.59	48	100	0.38	--
6.3	230.2	0.7	0.29	SAND	30 45	1.83	48	100	0.39	--
6.5	239.4	2.1	0.36	SAND	37 55	1.34	48	100	0.40	--
6.6	195.1	2.2	1.13	SAND	34 50	0.98	48	97	0.41	--
6.8	193.6	1.7	0.90	SAND	31 46	1.19	48	97	0.42	--
7.0	196.8	0.4	0.22	SAND	26 38	1.80	48	97	0.43	--
7.1	180.9	0.8	0.44	SAND	26 37	1.59	47	94	0.44	--
7.3	159.5	1.3	0.82	SAND	26 37	1.19	47	90	0.46	--
7.4	147.6	0.8	0.52	SAND	22 31	1.47	46	87	0.47	--
7.6	115.6	0.5	0.39	SAND	17 23	1.55	45	78	0.48	--
7.8	81.6	0.5	0.64	SAND	14 19	1.08	43	68	0.49	--
7.9	59.7	0.6	0.97	SAND	15 21	0.14	41	57	0.50	--
8.1	40.7	0.5	1.11	SILTY SAND	11 15	0.08	39	46	0.51	--
8.3	32.8	0.3	0.88	SILTY SAND	9 12	0.08	38	40	0.52	--
8.4	24.0	0.1	0.54	SILTY SAND	7 9	0.08	37	31	0.53	--
8.6	15.6	0.2	1.02	SANDY SILT AND SILT	5 7	0.05	34	20	0.54	--
8.8	26.6	0.1	0.53	SILTY SAND	7 10	0.08	37	33	0.55	--
8.9	21.4	0.3	1.36	SILTY SAND	7 9	0.05	36	26	0.56	--
9.1	24.2	0.2	0.83	SILTY SAND	7 9	0.07	36	29	0.57	--
9.3	14.3	0.1	0.98	SANDY SILT AND SILT	5 6	0.05	34	20	0.58	--
9.4	27.8	0.1	0.36	SILTY SAND	8 10	0.09	37	32	0.59	--
9.6	20.5	0.1	0.34	SILTY SAND	6 7	0.09	35	23	0.60	--
9.7	11.6	0.1	0.95	SANDY SILT AND SILT	4 5	0.04	32	20	0.61	--
9.9	6.8	0.1	1.46	SANDY SILT AND SILT	3 3	0.02	30	20	0.62	--
10.1	7.2	0.1	0.83	SANDY SILT AND SILT	3 3	0.03	30	20	0.63	--
10.2	8.8	0.0	0.45	SANDY SILT AND SILT	3 4	0.04	31	20	0.64	--
10.4	6.8	0.0	0.59	SANDY SILT AND SILT	2 3	0.04	29	20	0.65	--
10.6	7.5	0.0	0.53	SANDY SILT AND SILT	3 3	0.04	30	20	0.66	--
10.7	8.2	0.0	0.49	SANDY SILT AND SILT	3 3	0.04	30	20	0.67	--
10.9	5.4	0.0	0.74	SANDY SILT AND SILT	2 2	0.03	28	20	0.68	--
11.1	6.6	0.1	0.75	SANDY SILT AND SILT	2 3	0.03	29	20	0.69	--
11.2	5.8	0.0	0.69	SANDY SILT AND SILT	2 2	0.03	28	20	0.69	--
11.4	5.7	0.0	0.52	SANDY SILT AND SILT	2 2	0.04	28	20	0.70	--
11.5	4.1	0.0	0.74	SANDY SILT AND SILT	2 2	0.03	26	20	0.70	--
11.7	6.1	0.0	0.49	SANDY SILT AND SILT	2 2	0.04	28	20	0.71	--
11.9	6.8	0.0	0.44	SANDY SILT AND SILT	2 3	0.04	29	20	0.71	--
12.0	5.4	0.0	0.55	SANDY SILT AND SILT	2 2	0.04	28	20	0.72	--
12.2	4.3	0.0	0.70	SANDY SILT AND SILT	2 2	0.03	27	20	0.73	--
12.4	4.5	0.0	0.67	SANDY SILT AND SILT	2 2	0.03	27	20	0.73	--
12.5	4.6	0.0	0.43	SANDY SILT AND SILT	2 2	0.04	27	20	0.74	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 1

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
12.7	4.6	0.0	0.44	SANDY SILT AND SILT	2	2	0.04	27	20	0.74	--
12.9	4.4	0.0	0.45	SANDY SILT AND SILT	2	2	0.04	27	20	0.75	--
13.0	4.6	0.0	0.43	SANDY SILT AND SILT	2	2	0.04	27	20	0.75	--
13.2	4.3	0.0	0.47	SANDY SILT AND SILT	1	2	0.03	26	20	0.76	--
13.4	4.6	0.0	0.44	SANDY SILT AND SILT	2	2	0.04	27	20	0.76	--
13.5	4.8	0.0	0.42	SANDY SILT AND SILT	2	2	0.04	27	20	0.77	--
13.7	4.8	0.0	0.21	SANDY SILT AND SILT	2	2	0.04	27	20	0.77	--
13.8	4.8	0.0	0.42	SANDY SILT AND SILT	2	2	0.04	27	20	0.78	--
14.0	4.8	0.0	0.42	SANDY SILT AND SILT	2	2	0.04	27	20	0.78	--
14.2	4.8	0.0	0.42	SANDY SILT AND SILT	2	2	0.04	27	20	0.79	--
14.3	4.9	0.0	0.21	SANDY SILT AND SILT	2	2	0.04	27	20	0.79	--
14.5	6.9	0.1	0.87	SANDY SILT AND SILT	2	3	0.03	28	20	0.80	--
14.7	7.4	0.1	1.88	SANDY SILT AND SILT	3	4	0.01	29	20	0.80	--
14.8	6.8	0.1	2.06	CLAYEY SILT AND SILTY CLAY	3	3	----	----	0.81	0.37	
15.0	7.4	0.2	2.31	CLAYEY SILT AND SILTY CLAY	3	4	----	----	0.81	0.40	
15.2	8.2	0.2	2.70	CLAYEY SILT AND SILTY CLAY	4	4	----	----	0.82	0.45	
15.3	10.4	0.3	2.98	CLAYEY SILT AND SILTY CLAY	5	5	----	----	0.82	0.59	
15.5	17.8	0.3	1.74	SANDY SILT AND SILT	6	7	0.04	33	20	0.83	--
15.7	18.1	0.3	1.82	SANDY SILT AND SILT	6	7	0.04	33	20	0.83	--
15.8	25.8	0.2	0.77	SILTY SAND	7	8	0.08	35	20	0.84	--
16.0	29.2	0.2	0.55	SILTY SAND	8	9	0.09	35	24	0.84	--
16.2	29.9	0.1	0.37	SILTY SAND	8	9	0.09	35	24	0.85	--
16.3	30.6	0.2	0.49	SILTY SAND	8	9	0.09	35	25	0.85	--
16.5	25.4	0.3	1.10	SILTY SAND	7	8	0.07	34	20	0.86	--
16.7	15.1	0.2	1.26	SANDY SILT AND SILT	5	5	0.04	32	20	0.86	--
16.8	9.5	0.1	1.05	SANDY SILT AND SILT	3	4	0.03	30	20	0.87	--
17.0	8.2	0.1	1.22	SANDY SILT AND SILT	3	3	0.03	29	20	0.88	--
17.2	9.4	0.1	1.28	SANDY SILT AND SILT	3	4	0.03	29	20	0.88	--
17.3	9.5	0.2	1.89	SANDY SILT AND SILT	4	4	0.02	29	20	0.89	--
17.5	10.9	0.2	1.94	SANDY SILT AND SILT	4	5	0.02	30	20	0.89	--
17.7	20.4	0.1	0.59	SILTY SAND	6	6	0.08	33	20	0.90	--
17.8	24.2	0.1	0.45	SILTY SAND	7	7	0.09	34	20	0.90	--
18.0	24.0	0.1	0.38	SILTY SAND	7	7	0.09	34	20	0.91	--
18.1	22.2	0.1	0.27	SILTY SAND	6	6	0.09	33	20	0.91	--
18.3	20.8	0.1	0.58	SILTY SAND	6	6	0.08	33	20	0.92	--
18.5	14.6	0.1	0.82	SANDY SILT AND SILT	5	5	0.05	31	20	0.92	--
18.6	13.2	0.1	0.38	SILTY SAND	4	4	0.07	31	20	0.93	--
18.8	10.2	0.0	0.29	SANDY SILT AND SILT	3	3	0.05	30	20	0.93	--
19.0	7.4	0.0	0.27	SANDY SILT AND SILT	2	2	0.04	28	20	0.94	--
19.1	6.8	0.0	0.15	SANDY SILT AND SILT	2	2	0.05	28	20	0.94	--
19.3	6.1	0.0	0.16	SANDY SILT AND SILT	2	2	0.05	27	20	0.95	--
19.5	6.1	0.0	0.16	SANDY SILT AND SILT	2	2	0.05	27	20	0.95	--
19.6	6.1	0.0	0.16	SANDY SILT AND SILT	2	2	0.05	27	20	0.96	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 1

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
19.8	5.5	0.0	0.18	SANDY SILT AND SILT	2	2	0.05	26	20	0.96	--
19.9	6.1	0.0	0.16	SANDY SILT AND SILT	2	2	0.05	27	20	0.97	--
20.1	7.4	0.1	0.81	SANDY SILT AND SILT	3	3	0.03	28	20	0.97	--
20.3	18.4	0.0	0.05	SILTY SAND	5	5	0.10	32	20	0.98	--
20.4	39.4	0.0	0.08	SAND	5	5	1.70	36	28	0.98	--
20.6	46.9	0.1	0.17	SAND	7	7	1.54	37	33	0.99	--
20.8	44.3	0.1	0.32	SAND	8	8	1.04	36	32	0.99	--
20.9	42.3	0.2	0.36	SAND	8	8	0.81	36	30	1.00	--
21.1	44.9	0.1	0.20	SAND	7	7	1.41	36	32	1.00	--
21.3	44.9	0.1	0.29	SAND	7	7	1.14	36	32	1.01	--
21.4	42.2	0.1	0.24	SAND	7	7	1.20	36	30	1.01	--
21.6	44.2	0.1	0.16	SAND	6	6	1.52	36	31	1.02	--
21.8	44.2	0.1	0.14	SAND	6	6	1.59	36	31	1.02	--
21.9	44.2	0.1	0.16	SAND	6	6	1.52	36	31	1.03	--
22.1	47.7	0.2	0.34	SAND	8	8	1.11	37	33	1.03	--
22.2	51.7	0.2	0.46	SAND	9	9	0.92	37	35	1.04	--
22.4	54.0	0.3	0.48	SAND	10	9	0.95	37	36	1.04	--
22.6	61.2	0.2	0.33	SAND	9	9	1.39	38	40	1.05	--
22.7	70.7	0.3	0.38	SAND	11	10	1.38	38	45	1.05	--
22.9	70.7	0.3	0.44	SAND	11	11	1.29	38	45	1.06	--
23.1	62.1	0.3	0.44	SAND	10	10	1.20	38	40	1.07	--
23.2	51.0	0.2	0.43	SAND	9	9	0.97	37	34	1.07	--
23.4	38.1	0.2	0.53	SILTY SAND	10	10	0.10	35	25	1.08	--
23.6	23.8	0.1	0.50	SILTY SAND	7	6	0.08	33	20	1.08	--
23.7	12.9	0.1	1.01	SANDY SILT AND SILT	4	4	0.04	30	20	1.09	--
23.9	11.6	0.1	0.52	SANDY SILT AND SILT	4	3	0.05	29	20	1.09	--
24.0	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	28	20	1.10	--
24.2	8.3	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	28	20	1.10	--
24.4	9.5	0.0	0.11	SILTY SAND	3	3	0.05	28	20	1.11	--
24.5	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	28	20	1.11	--
24.7	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	28	20	1.12	--
24.9	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	28	20	1.12	--
25.0	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	28	20	1.13	--
25.2	8.8	0.0	0.11	SANDY SILT AND SILT	3	3	0.05	28	20	1.13	--
25.4	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	27	20	1.14	--
25.5	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	27	20	1.14	--
25.7	8.0	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	27	20	1.15	--
25.9	8.5	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	28	20	1.15	--
26.0	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	27	20	1.16	--
26.2	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	27	20	1.16	--
26.3	7.9	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	27	20	1.17	--
26.5	7.5	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	27	20	1.17	--
26.7	7.5	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	27	20	1.18	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 1

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
26.8	7.5	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	27	20	1.18	--
27.0	7.6	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	27	20	1.19	--
27.2	7.5	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	27	20	1.19	--
27.3	7.5	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	27	20	1.20	--
27.5	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	27	20	1.20	--
27.7	8.5	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	27	20	1.21	--
27.8	8.8	0.0	0.11	SANDY SILT AND SILT	3	3	0.05	28	20	1.21	--
28.0	8.6	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	27	20	1.22	--
28.1	8.4	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	27	20	1.22	--
28.3	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	27	20	1.23	--
28.5	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	27	20	1.23	--
28.6	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	27	20	1.24	--
28.8	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	27	20	1.24	--
29.0	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	27	20	1.25	--
29.1	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	27	20	1.26	--
29.3	8.8	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	27	20	1.26	--
29.5	8.8	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	27	20	1.27	--
29.6	9.5	0.0	0.11	SILTY SAND	3	3	0.05	28	20	1.27	--
29.8	9.5	0.0	0.32	SANDY SILT AND SILT	3	3	0.05	28	20	1.28	--
30.0	8.8	0.0	0.34	SANDY SILT AND SILT	3	3	0.04	27	20	1.28	--
30.1	9.4	0.0	0.11	SANDY SILT AND SILT	3	3	0.05	28	20	1.29	--
30.3	8.8	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	27	20	1.29	--
30.4	9.5	0.0	0.11	SILTY SAND	3	3	0.05	28	20	1.30	--
30.6	9.5	0.0	0.11	SILTY SAND	3	3	0.05	28	20	1.30	--
30.8	8.8	0.0	0.34	SANDY SILT AND SILT	3	3	0.04	27	20	1.31	--
30.9	9.9	0.0	0.20	SANDY SILT AND SILT	3	3	0.05	28	20	1.31	--
31.1	9.1	0.0	0.33	SANDY SILT AND SILT	3	3	0.04	27	20	1.32	--
31.3	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	27	20	1.32	--
31.4	7.7	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	26	20	1.33	--
31.6	7.5	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	26	20	1.33	--
31.8	7.5	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	26	20	1.34	--
31.9	7.5	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	26	20	1.34	--
32.1	7.5	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	26	20	1.35	--
32.3	7.6	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	26	20	1.35	--
32.4	8.0	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	27	20	1.36	--
32.6	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	27	20	1.36	--
32.7	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	27	20	1.37	--
32.9	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	27	20	1.37	--
33.1	8.8	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	27	20	1.38	--
33.2	9.1	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	27	20	1.38	--
33.4	9.3	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	27	20	1.39	--
33.6	8.8	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	27	20	1.39	--
33.7	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	26	20	1.40	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 1

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
33.9	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	26	20	1.40	--
34.1	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	26	20	1.41	--
34.2	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	26	20	1.41	--
34.4	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	26	20	1.42	--
34.5	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	26	20	1.42	--
34.7	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	26	20	1.43	--
34.9	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	26	20	1.43	--
35.0	9.5	0.0	0.11	SILTY SAND	3	2	0.05	27	20	1.44	--
35.2	8.8	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	27	20	1.45	--
35.4	8.8	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	27	20	1.45	--
35.5	9.5	0.0	0.11	SILTY SAND	3	2	0.05	27	20	1.46	--
35.7	9.0	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	27	20	1.46	--
35.9	8.8	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	27	20	1.47	--
36.0	8.8	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	27	20	1.47	--
36.2	8.8	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	27	20	1.48	--
36.4	8.8	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	27	20	1.48	--
36.5	8.9	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	27	20	1.49	--
36.7	9.5	0.0	0.11	SILTY SAND	3	2	0.05	27	20	1.49	--
36.8	9.5	0.0	0.11	SILTY SAND	3	2	0.05	27	20	1.50	--
37.0	9.5	0.0	0.11	SILTY SAND	3	2	0.05	27	20	1.50	--
37.2	9.2	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	27	20	1.51	--
37.3	8.9	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	27	20	1.51	--
37.5	8.8	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	26	20	1.52	--
37.7	8.8	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	26	20	1.52	--
37.8	9.0	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	27	20	1.53	--
38.0	9.5	0.0	0.11	SILTY SAND	3	2	0.05	27	20	1.53	--
38.2	9.5	0.0	0.11	SILTY SAND	3	2	0.05	27	20	1.54	--
38.3	9.5	0.0	0.11	SILTY SAND	3	2	0.05	27	20	1.54	--
38.5	9.5	0.0	0.11	SILTY SAND	3	2	0.05	27	20	1.55	--
38.6	10.3	0.0	0.39	SANDY SILT AND SILT	3	3	0.05	27	20	1.55	--
38.8	10.9	0.0	0.28	SILTY SAND	3	3	0.06	27	20	1.56	--
39.0	10.9	0.0	0.37	SANDY SILT AND SILT	3	3	0.05	27	20	1.56	--
39.1	11.0	0.0	0.36	SANDY SILT AND SILT	3	3	0.05	27	20	1.57	--
39.3	10.9	0.0	0.09	SILTY SAND	3	2	0.09	27	20	1.57	--
39.5	9.7	0.0	0.10	SILTY SAND	3	2	0.07	27	20	1.58	--
39.6	9.6	0.0	0.10	SILTY SAND	3	2	0.06	27	20	1.58	--
39.8	10.9	0.0	0.37	SANDY SILT AND SILT	3	3	0.05	27	20	1.59	--
40.0	13.5	0.2	1.41	SANDY SILT AND SILT	5	4	0.04	28	20	1.59	--
40.1	17.7	0.2	1.07	SILTY SAND	5	4	0.05	30	20	1.60	--
40.3	17.7	0.2	1.13	SANDY SILT AND SILT	5	4	0.05	30	20	1.60	--
40.5	17.0	0.4	2.05	SANDY SILT AND SILT	6	5	0.03	29	20	1.61	--
40.6	25.8	0.3	1.04	SILTY SAND	7	6	0.07	31	20	1.61	--
40.8	19.6	0.2	1.12	SILTY SAND	6	5	0.06	30	20	1.62	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 1

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
40.9	23.8	0.2	0.67	SILTY SAND	7	5	0.08	31	20	1.62	--
41.1	44.9	0.1	0.20	SAND	7	5	1.41	34	20	1.63	--
41.3	40.1	0.0	0.02	SAND	5	4	1.91	34	20	1.64	--
41.4	24.5	0.2	0.90	SILTY SAND	7	6	0.07	31	20	1.64	--
41.6	26.7	0.2	0.75	SILTY SAND	8	6	0.08	31	20	1.65	--
41.8	17.2	0.2	1.05	SILTY SAND	5	4	0.05	29	20	1.65	--
41.9	14.5	0.1	0.55	SILTY SAND	4	3	0.06	28	20	1.66	--
42.1	13.6	0.1	0.96	SANDY SILT AND SILT	4	3	0.04	28	20	1.66	--
42.3	14.2	0.1	0.91	SANDY SILT AND SILT	4	3	0.05	28	20	1.67	--
42.4	15.0	0.2	1.34	SANDY SILT AND SILT	5	4	0.04	29	20	1.67	--
42.6	13.9	0.3	2.01	SANDY SILT AND SILT	5	4	0.02	28	20	1.68	--
42.7	17.7	0.2	1.07	SILTY SAND	5	4	0.05	29	20	1.68	--
42.9	22.4	0.1	0.49	SILTY SAND	6	5	0.08	31	20	1.69	--
43.1	19.7	0.2	1.22	SILTY SAND	6	5	0.05	30	20	1.69	--
43.2	17.0	0.2	1.36	SANDY SILT AND SILT	5	4	0.04	29	20	1.70	--
43.4	16.0	0.2	1.25	SANDY SILT AND SILT	5	4	0.04	29	20	1.70	--
43.6	15.0	0.2	1.34	SANDY SILT AND SILT	5	4	0.04	28	20	1.71	--
43.7	15.7	0.2	1.47	SANDY SILT AND SILT	5	4	0.04	29	20	1.71	--
43.9	16.0	0.2	1.25	SANDY SILT AND SILT	5	4	0.04	29	20	1.72	--
44.1	16.3	0.2	1.10	SANDY SILT AND SILT	5	4	0.05	29	20	1.72	--
44.2	15.6	0.1	0.90	SILTY SAND	5	4	0.05	29	20	1.73	--
44.4	15.6	0.1	0.90	SILTY SAND	5	4	0.05	29	20	1.73	--
44.6	14.0	0.2	1.36	SANDY SILT AND SILT	5	4	0.04	28	20	1.74	--
44.7	13.9	0.2	1.58	SANDY SILT AND SILT	5	4	0.03	28	20	1.74	--
44.9	14.3	0.2	1.33	SANDY SILT AND SILT	5	4	0.04	28	20	1.75	--
45.0	13.6	0.2	1.40	SANDY SILT AND SILT	5	4	0.04	28	20	1.75	--
45.2	15.0	0.2	1.20	SANDY SILT AND SILT	5	4	0.04	28	20	1.76	--
45.4	14.3	0.1	0.98	SANDY SILT AND SILT	5	3	0.05	28	20	1.76	--
45.5	12.9	0.1	0.85	SANDY SILT AND SILT	4	3	0.05	28	20	1.77	--
45.7	11.6	0.1	0.87	SANDY SILT AND SILT	4	3	0.04	27	20	1.77	--
45.9	10.9	0.1	0.92	SANDY SILT AND SILT	4	3	0.04	27	20	1.78	--
46.0	10.9	0.1	0.74	SANDY SILT AND SILT	4	3	0.04	27	20	1.78	--
46.2	10.4	0.1	0.58	SANDY SILT AND SILT	3	2	0.04	26	20	1.79	--
46.4	9.8	0.0	0.20	SANDY SILT AND SILT	3	2	0.05	26	20	1.79	--
46.5	10.9	0.2	1.38	SANDY SILT AND SILT	4	3	0.03	27	20	1.80	--
46.7	16.9	0.4	2.13	SANDY SILT AND SILT	6	5	0.03	29	20	1.80	--
46.8	21.8	0.6	2.67	SANDY SILT AND SILT	9	6	0.02	30	20	1.81	--
47.0	24.5	0.5	2.12	SANDY SILT AND SILT	8	6	0.04	31	20	1.81	--
47.2	19.7	0.5	2.28	SANDY SILT AND SILT	7	5	0.03	30	20	1.82	--
47.3	13.2	0.3	2.57	SANDY SILT AND SILT	6	4	0.01	28	20	1.83	--
47.5	10.2	0.3	2.55	CLAYEY SILT AND SILTY CLAY	5	3	---	---	1.83	0.45	--
47.7	9.8	0.3	2.65	CLAYEY SILT AND SILTY CLAY	5	3	---	---	1.84	0.43	--
47.8	15.6	0.4	2.30	SANDY SILT AND SILT	6	5	0.02	28	20	1.84	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 1

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
48.0	19.0	0.4	2.31	SANDY SILT AND SILT	7	5	0.03	29	20	1.85	--
48.2	19.7	0.4	2.13	SANDY SILT AND SILT	7	5	0.03	29	20	1.85	--
48.3	17.7	0.4	2.15	SANDY SILT AND SILT	7	5	0.03	29	20	1.86	--
48.5	15.6	0.4	2.43	SANDY SILT AND SILT	6	5	0.02	28	20	1.86	--
48.7	16.9	0.5	2.96	SANDY SILT AND SILT	7	5	0.01	29	20	1.87	--
48.8	23.8	0.6	2.69	SANDY SILT AND SILT	9	7	0.02	30	20	1.87	--
49.0	23.1	0.7	3.07	SANDY SILT AND SILT	10	7	0.01	30	20	1.88	--
49.1	20.4	0.6	3.04	SANDY SILT AND SILT	9	6	0.01	30	20	1.88	--
49.3	17.9	0.6	3.23	CLAYEY SILT AND SILTY CLAY	8	6	---	---	---	1.89	0.93
49.5	19.0	0.6	3.16	CLAYEY SILT AND SILTY CLAY	9	6	---	---	---	1.89	0.99
49.6	18.9	0.6	3.29	CLAYEY SILT AND SILTY CLAY	9	6	---	---	---	1.90	0.99
49.8	15.6	0.6	3.71	CLAYEY SILT AND SILTY CLAY	8	6	---	---	---	1.90	0.78
50.0	13.6	0.5	3.38	CLAYEY SILT AND SILTY CLAY	7	5	---	---	---	1.91	0.65
50.1	12.7	0.3	2.60	SANDY SILT AND SILT	6	4	0.01	27	20	1.91	--
50.3	12.2	0.3	2.04	SANDY SILT AND SILT	5	3	0.02	27	20	1.92	--
50.5	11.7	0.2	1.45	SANDY SILT AND SILT	4	3	0.03	27	20	1.92	--
50.6	11.6	0.1	1.04	SANDY SILT AND SILT	4	3	0.04	27	20	1.93	--
50.8	12.2	0.1	0.74	SANDY SILT AND SILT	4	3	0.05	27	20	1.93	--
51.0	12.9	0.1	0.93	SANDY SILT AND SILT	4	3	0.04	27	20	1.94	--
51.1	14.3	0.3	2.17	SANDY SILT AND SILT	6	4	0.02	28	20	1.94	--
51.3	17.9	0.5	2.51	SANDY SILT AND SILT	7	5	0.02	29	20	1.95	--
51.4	38.1	0.2	0.42	SAND	10	7	0.12	32	20	1.95	--
51.6	49.8	0.2	0.44	SAND	9	6	0.91	34	20	1.96	--
51.8	41.5	0.3	0.60	SILTY SAND	11	8	0.10	33	20	1.96	--
51.9	29.2	0.3	1.03	SILTY SAND	8	6	0.07	31	20	1.97	--
52.1	18.9	0.4	2.33	SANDY SILT AND SILT	7	5	0.03	29	20	1.97	--
52.3	23.1	0.2	0.74	SILTY SAND	7	5	0.08	30	20	1.98	--
52.4	19.3	0.3	1.61	SANDY SILT AND SILT	6	4	0.04	29	20	1.98	--
52.6	17.9	0.6	3.12	CLAYEY SILT AND SILTY CLAY	8	6	---	---	---	1.99	0.92
52.8	43.7	0.2	0.43	SAND	9	6	0.65	33	20	1.99	--
52.9	63.5	0.2	0.31	SAND	9	7	1.44	35	24	2.00	--
53.1	57.8	0.2	0.26	SAND	8	6	1.48	34	21	2.00	--
53.2	47.8	0.3	0.52	SAND	10	7	0.62	33	20	2.01	--
53.4	33.9	0.1	0.38	SILTY SAND	9	6	0.10	32	20	2.01	--
53.6	23.9	0.5	2.05	SANDY SILT AND SILT	8	6	0.04	30	20	2.02	--
53.7	32.3	0.4	1.21	SILTY SAND	9	6	0.07	31	20	2.03	--
53.9	72.5	0.3	0.46	SAND	11	8	1.28	35	29	2.03	--
54.1	98.7	0.3	0.34	SAND	14	10	1.56	37	40	2.04	--
54.2	97.9	0.5	0.46	SAND	15	10	1.41	37	39	2.04	--
54.4	85.6	0.7	0.85	SAND	16	11	0.81	36	34	2.05	--
54.6	89.7	1.0	1.10	SAND	19	13	0.50	36	36	2.05	--
54.7	123.7	0.5	0.40	SAND	18	12	1.56	38	47	2.06	--
54.9	148.3	0.7	0.49	SAND	22	15	1.50	39	55	2.06	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 1

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
55.1	157.1	0.8	0.53	SAND	23	16	1.47	39	58	2.07	--
55.2	156.4	0.9	0.57	SAND	23	16	1.43	39	57	2.07	--
55.4	146.1	0.8	0.53	SAND	22	15	1.45	39	54	2.08	--
55.5	120.8	0.6	0.51	SAND	18	12	1.43	38	46	2.08	--
55.7	104.7	0.8	0.74	SAND	17	12	1.09	37	41	2.09	--
55.9	79.4	1.0	1.20	SAND	19	13	0.24	36	31	2.09	--
56.0	76.8	0.8	1.08	SAND	17	12	0.37	35	30	2.10	--
56.2	91.1	0.7	0.74	SAND	16	11	1.01	36	36	2.10	--
56.4	109.5	0.3	0.25	SAND	15	10	1.70	37	42	2.11	--
56.5	104.7	0.3	0.26	SAND	14	10	1.68	37	41	2.11	--
56.7	78.9	0.4	0.44	SAND	12	8	1.34	36	31	2.12	--
56.9	51.0	0.6	1.12	SILTY SAND	14	9	0.09	33	20	2.12	--
57.0	29.2	0.5	1.64	SILTY SAND	9	6	0.05	31	20	2.13	--
57.2	19.0	0.3	1.79	SANDY SILT AND SILT	6	4	0.04	29	20	2.13	--
57.3	17.7	0.1	0.79	SILTY SAND	5	3	0.06	28	20	2.14	--
57.5	16.3	0.1	0.74	SILTY SAND	5	3	0.06	28	20	2.14	--
57.7	15.0	0.1	0.80	SILTY SAND	5	3	0.05	27	20	2.15	--
57.8	14.3	0.1	0.56	SILTY SAND	4	3	0.06	27	20	2.15	--
58.0	11.6	0.0	0.09	SILTY SAND	3	2	0.09	26	20	2.16	--
58.2	9.7	0.0	0.10	SILTY SAND	3	2	0.07	25	20	2.16	--
58.3	10.9	0.0	0.09	SILTY SAND	3	2	0.09	26	20	2.17	--
58.5	15.8	0.2	1.14	SANDY SILT AND SILT	5	3	0.05	28	20	2.17	--
58.7	19.0	0.4	1.84	SANDY SILT AND SILT	7	4	0.04	28	20	2.18	--
58.8	19.7	0.5	2.28	SANDY SILT AND SILT	7	5	0.03	29	20	2.18	--
59.0	18.3	0.5	2.52	SANDY SILT AND SILT	7	5	0.02	28	20	2.19	--
59.2	17.0	0.4	2.47	SANDY SILT AND SILT	7	4	0.02	28	20	2.19	--
59.3	16.3	0.4	2.14	SANDY SILT AND SILT	6	4	0.03	28	20	2.20	--
59.5	17.7	0.4	2.15	SANDY SILT AND SILT	7	4	0.03	28	20	2.20	--
59.6	16.3	0.4	2.27	SANDY SILT AND SILT	6	4	0.02	28	20	2.21	--
59.8	15.0	0.3	2.21	SANDY SILT AND SILT	6	4	0.02	27	20	2.22	--
60.0	12.9	0.2	1.39	SANDY SILT AND SILT	5	3	0.03	26	20	2.22	--
60.1	10.9	0.1	0.92	SANDY SILT AND SILT	4	2	0.04	26	20	2.23	--
60.3	10.9	0.1	0.74	SANDY SILT AND SILT	4	2	0.04	26	20	2.23	--
60.5	11.6	0.1	0.61	SANDY SILT AND SILT	4	2	0.05	26	20	2.24	--
60.6	12.2	0.1	0.90	SANDY SILT AND SILT	4	3	0.04	26	20	2.24	--
60.8	15.0	0.2	1.47	SANDY SILT AND SILT	5	3	0.04	27	20	2.25	--
61.0	16.3	0.5	2.82	SANDY SILT AND SILT	7	5	0.01	28	20	2.25	--
61.1	17.0	0.5	3.18	CLAYEY SILT AND SILTY CLAY	8	5	---	---	---	2.26	0.82
61.3	16.3	0.4	2.70	SANDY SILT AND SILT	7	4	0.01	28	20	2.26	--
61.4	15.0	0.3	2.07	SANDY SILT AND SILT	6	4	0.03	27	20	2.27	--
61.6	13.9	0.2	1.73	SANDY SILT AND SILT	5	3	0.03	27	20	2.27	--
61.7	13.6	0.3	2.13	SANDY SILT AND SILT	5	3	0.02	27	20	2.28	--
61.9	14.7	0.3	2.24	SANDY SILT AND SILT	6	4	0.02	27	20	2.28	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 1

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
62.1	16.3	0.4	2.46	SANDY SILT AND SILT	7	4	0.02	27	20	2.29	--
62.2	16.5	0.5	2.78	SANDY SILT AND SILT	7	5	0.01	28	20	2.29	--
62.4	17.0	0.5	2.65	SANDY SILT AND SILT	7	4	0.02	28	20	2.30	--
62.6	16.5	0.4	2.57	SANDY SILT AND SILT	7	4	0.02	27	20	2.30	--
62.7	16.3	0.4	2.53	SANDY SILT AND SILT	7	4	0.02	27	20	2.31	--
62.9	16.3	0.4	2.58	SANDY SILT AND SILT	7	4	0.02	27	20	2.31	--
63.1	16.2	0.5	2.78	SANDY SILT AND SILT	7	4	0.02	27	20	2.31	--
63.2	16.4	0.5	2.93	CLAYEY SILT AND SILTY CLAY	7	5	---	---	---	2.32	0.78
63.4	17.7	0.5	2.83	SANDY SILT AND SILT	8	5	0.01	23	20	2.33	--
63.5	16.3	0.5	2.82	SANDY SILT AND SILT	7	4	0.01	27	20	2.33	--
63.7	14.9	0.4	2.35	SANDY SILT AND SILT	6	4	0.02	27	20	2.34	--
63.9	14.8	0.3	1.93	SANDY SILT AND SILT	5	3	0.03	27	20	2.34	--
64.0	16.4	0.2	1.22	SANDY SILT AND SILT	5	3	0.05	27	20	2.35	--
64.2	17.7	0.2	0.85	SILTY SAND	5	3	0.06	28	20	2.35	--
64.4	19.3	0.2	1.16	SILTY SAND	6	4	0.05	28	20	2.36	--
64.5	24.8	0.5	1.82	SANDY SILT AND SILT	8	5	0.04	29	20	2.36	--
64.7	65.5	0.3	0.50	SAND	11	7	1.12	34	21	2.37	--
64.9	120.4	0.4	0.31	SAND	17	10	1.65	37	43	2.37	--
65.0	129.2	0.5	0.36	SAND	18	11	1.60	37	46	2.38	--
65.2	129.6	0.6	0.47	SAND	19	12	1.48	37	46	2.38	--
65.4	130.8	0.7	0.57	SAND	20	12	1.38	37	46	2.39	--
65.5	133.0	0.8	0.52	SAND	20	13	1.33	38	46	2.39	--
65.7	133.8	0.9	0.64	SAND	21	13	1.31	38	47	2.40	--
65.8	140.0	0.9	0.64	SAND	22	13	1.32	38	48	2.40	--
66.0	164.5	0.9	0.53	SAND	24	15	1.48	39	55	2.41	--
66.2	195.8	0.8	0.43	SAND	27	17	1.61	39	63	2.41	--
66.3	207.6	0.9	0.45	SAND	29	18	1.62	40	65	2.42	--
66.5	202.6	1.1	0.52	SAND	29	18	1.54	40	64	2.42	--
66.7	208.3	0.9	0.44	SAND	29	18	1.63	40	65	2.43	--
66.8	216.3	0.9	0.42	SAND	30	18	1.68	40	67	2.43	--
67.0	223.0	1.2	0.53	SAND	31	19	1.60	40	68	2.44	--
67.2	240.0	0.9	0.39	SAND	32	20	1.77	40	71	2.45	--
67.3	351.2	1.7	0.47	SAND	45	27	1.94	42	87	2.45	--
67.5	555.1	3.8	0.68	SAND AND GRAVEL	69	42	2.11	44	100	2.46	--
67.7	563.7	3.3	0.59	SAND AND GRAVEL	68	41	2.51	44	100	2.46	--
67.8	433.9	3.4	0.79	SAND	58	35	1.78	43	96	2.47	--
68.0	360.4	1.0	0.27	SAND AND GRAVEL	41	25	3.06	42	88	2.47	--
68.1	331.8	1.1	0.32	SAND AND GRAVEL	40	24	2.40	42	85	2.48	--
68.3	329.1	2.4	0.73	SAND	45	27	1.67	42	84	2.48	--
68.5	365	9	1.09	SAND	55	33	1.41	42	88	2.49	--

INTERPRETED CONE PENETRATION TEST DATA

C. P.T. - 14

CLIENT: HOWARD HUGHES PROPERTIES
 JOB NO: AE-88473
 DATE: JANUARY 30, 1989
 ELEVATION: 11.8 FT
 UNIT WEIGHT OF SOIL: 125 PCF
 DEPTH TO GROUND WATER: 10 FT

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	P0 TSF	SU TSF
0.1	10.9	0.4	3.40	CLAYEY SILT AND SILTY CLAY	5 21	---	---	---	0.01	0.68
0.3	19.7	0.6	3.19	CLAYEY SILT AND SILTY CLAY	9 29	---	---	---	0.02	1.23
0.4	18.4	0.6	3.43	CLAYEY SILT AND SILTY CLAY	9 26	---	---	---	0.03	1.15
0.6	17.8	0.6	3.21	CLAYEY SILT AND SILTY CLAY	8 23	---	---	---	0.04	1.11
0.8	18.4	0.6	3.10	CLAYEY SILT AND SILTY CLAY	8 22	---	---	---	0.05	1.15
0.9	21.1	0.6	2.75	SANDY SILT AND SILT	8 22	0.02	---	---	0.06	--
1.1	24.9	0.5	1.93	SANDY SILT AND SILT	8 20	0.04	---	---	0.07	--
1.2	21.8	0.3	1.29	SILTY SAND	7 16	0.05	---	---	0.08	--
1.4	16.3	0.2	0.98	SANDY SILT AND SILT	5 12	0.05	---	---	0.09	--
1.6	19.0	0.2	0.95	SILTY SAND	6 13	0.06	---	---	0.10	--
1.7	27.9	0.2	0.79	SILTY SAND	8 17	0.08	---	---	0.11	--
1.9	31.3	0.2	0.74	SILTY SAND	9 19	0.08	---	---	0.12	--
2.1	29.9	0.3	0.90	SILTY SAND	8 18	0.08	---	---	0.13	--
2.2	29.7	0.5	1.55	SILTY SAND	9 18	0.06	---	---	0.14	--
2.4	30.4	0.7	2.34	SANDY SILT AND SILT	10 21	0.04	---	---	0.15	--
2.6	25.2	0.8	3.30	SANDY SILT AND SILT	11 22	0.01	---	---	0.16	--
2.7	25.0	0.7	2.69	SANDY SILT AND SILT	10 19	0.02	---	---	0.17	--
2.9	21.6	0.5	2.32	SANDY SILT AND SILT	8 15	0.03	---	---	0.18	--
3.1	15.5	0.4	2.52	SANDY SILT AND SILT	6 12	0.02	---	---	0.19	--
3.2	14.5	0.5	3.16	CLAYEY SILT AND SILTY CLAY	7 13	---	---	---	0.20	0.90
3.4	19.6	0.5	2.55	SANDY SILT AND SILT	8 14	0.02	---	---	0.21	--
3.5	15.6	0.6	3.64	CLAYEY SILT AND SILTY CLAY	8 14	---	---	---	0.22	0.96
3.7	12.9	0.5	3.73	CLAYEY SILT AND SILTY CLAY	7 12	---	---	---	0.23	0.79
3.9	12.9	0.3	2.63	SANDY SILT AND SILT	6 10	0.01	---	---	0.24	--
4.0	19.2	0.3	1.51	SANDY SILT AND SILT	6 11	0.04	39	39	0.25	--
4.2	25.2	0.3	1.11	SILTY SAND	7 13	0.07	40	45	0.26	--
4.3	19.0	0.3	1.37	SANDY SILT AND SILT	6 10	0.05	39	37	0.27	--
4.5	17.1	0.3	1.93	SANDY SILT AND SILT	6 10	0.03	38	34	0.28	--
4.7	15.0	0.5	3.01	CLAYEY SILT AND SILTY CLAY	7 11	---	---	---	0.29	0.92
4.8	14.3	0.5	3.50	CLAYEY SILT AND SILTY CLAY	7 12	---	---	---	0.30	0.87
5.0	17.0	0.6	3.53	CLAYEY SILT AND SILTY CLAY	8 13	---	---	---	0.31	1.04
5.2	19.7	0.6	2.94	SANDY SILT AND SILT	8 14	0.01	38	35	0.32	--
5.3	18.4	0.6	3.43	CLAYEY SILT AND SILTY CLAY	9 14	---	---	---	0.33	1.13
5.5	31.1	0.5	1.74	SILTY SAND	9 15	0.05	40	46	0.34	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 14

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.
5.6	44.2	0.5	1.18	SILTY SAND	12 19	0.08	42	54	0.35	--
5.8	37.1	0.4	0.94	SILTY SAND	10 16	0.08	41	50	0.36	--
6.0	28.7	0.3	1.05	SILTY SAND	8 13	0.07	39	42	0.37	--
6.1	24.4	0.2	0.90	SILTY SAND	7 11	0.07	38	38	0.38	--
6.3	19.7	0.2	0.76	SILTY SAND	6 9	0.07	37	32	0.39	--
6.5	15.0	0.1	0.87	SANDY SILT AND SILT	5 7	0.05	36	24	0.40	--
6.6	11.8	0.2	1.52	SANDY SILT AND SILT	4 6	0.03	34	20	0.41	--
6.8	9.5	0.2	1.58	SANDY SILT AND SILT	4 5	0.02	33	20	0.42	--
7.0	5.7	0.1	2.48	CLAYEY SILT AND SILTY CLAY	3 4	---	---	---	0.43	0.33
7.1	7.5	0.1	1.20	SANDY SILT AND SILT	3 4	0.03	32	20	0.44	--
7.3	4.8	0.1	1.89	CLAYEY SILT AND SILTY CLAY	2 3	---	---	---	0.46	0.27
7.4	5.4	0.1	2.39	CLAYEY SILT AND SILTY CLAY	3 4	---	---	---	0.47	0.31
7.6	5.4	0.1	2.21	CLAYEY SILT AND SILTY CLAY	3 4	---	---	---	0.48	0.31
7.8	5.4	0.2	2.76	CLAYEY SILT AND SILTY CLAY	3 4	---	---	---	0.49	0.31
7.9	6.8	0.1	1.92	SANDY SILT AND SILT	3 4	0.01	31	20	0.50	--
8.1	8.8	0.1	1.24	SANDY SILT AND SILT	3 4	0.03	32	20	0.51	--
8.3	10.9	0.1	0.46	SANDY SILT AND SILT	3 5	0.05	33	20	0.52	--
8.4	11.9	0.1	0.67	SANDY SILT AND SILT	4 5	0.05	33	20	0.53	--
8.6	10.2	0.1	0.88	SANDY SILT AND SILT	3 5	0.04	32	20	0.54	--
8.8	7.4	0.1	0.67	SANDY SILT AND SILT	3 3	0.04	31	20	0.55	--
8.9	5.0	0.1	1.41	SANDY SILT AND SILT	2 3	0.01	29	20	0.56	--
9.1	4.8	0.1	2.94	CLAYEY SILT AND SILTY CLAY	3 3	---	---	---	0.57	0.26
9.3	5.4	0.2	3.49	CLAY	5 7	---	---	---	0.58	0.30
9.4	6.1	0.2	3.59	CLAYEY SILT AND SILTY CLAY	3 4	---	---	---	0.59	0.35
9.6	7.5	0.2	2.27	CLAYEY SILT AND SILTY CLAY	3 4	---	---	---	0.60	0.43
9.7	7.5	0.3	4.01	CLAY	7 9	---	---	---	0.61	0.43
9.9	10.8	0.5	4.93	CLAY	11 14	---	---	---	0.62	0.63
10.1	17.0	0.7	4.35	CLAYEY SILT AND SILTY CLAY	9 12	---	---	---	0.63	1.02
10.2	19.4	0.9	4.38	CLAYEY SILT AND SILTY CLAY	11 13	---	---	---	0.63	1.17
10.4	18.4	0.8	4.30	CLAYEY SILT AND SILTY CLAY	10 12	---	---	---	0.64	1.11
10.6	17.0	0.7	4.29	CLAYEY SILT AND SILTY CLAY	9 11	---	---	---	0.64	1.02
10.7	15.0	0.7	4.48	CLAY	15 18	---	---	---	0.65	0.89
10.9	13.6	0.6	4.71	CLAY	14 17	---	---	---	0.65	0.81
11.1	12.9	0.6	4.57	CLAY	13 16	---	---	---	0.66	0.76
11.2	11.2	0.5	4.38	CLAY	11 14	---	---	---	0.66	0.65
11.4	9.4	0.4	3.96	CLAYEY SILT AND SILTY CLAY	5 6	---	---	---	0.67	0.54
11.5	7.5	0.2	2.94	CLAYEY SILT AND SILTY CLAY	4 5	---	---	---	0.67	0.42
11.7	6.1	0.1	2.12	CLAYEY SILT AND SILTY CLAY	3 3	---	---	---	0.68	0.34
11.9	5.7	0.1	1.95	CLAYEY SILT AND SILTY CLAY	3 3	---	---	---	0.68	0.31
12.0	6.1	0.3	4.08	CLAY	6 7	---	---	---	0.69	0.34
12.2	9.7	0.2	2.26	SANDY SILT AND SILT	4 5	0.01	31	20	0.69	--
12.4	12.2	0.2	1.47	SANDY SILT AND SILT	4 5	0.03	32	20	0.70	--
12.5	7.5	0.1	1.74	SANDY SILT AND SILT	3 4	0.02	29	20	0.70	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 14

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.
12.7	6.1	0.1	2.29	CLAYEY SILT AND SILTY CLAY	3 3	---	--	---	0.71	0.33
12.9	6.0	0.2	2.50	CLAYEY SILT AND SILTY CLAY	3 3	---	--	---	0.71	0.32
13.0	5.7	0.2	2.61	CLAYEY SILT AND SILTY CLAY	3 3	---	--	---	0.72	0.31
13.2	6.1	0.1	1.96	CLAYEY SILT AND SILTY CLAY	3 3	---	--	---	0.72	0.33
13.4	6.6	0.1	2.11	CLAYEY SILT AND SILTY CLAY	3 4	---	--	---	0.73	0.36
13.5	8.7	0.2	2.41	CLAYEY SILT AND SILTY CLAY	4 5	---	--	---	0.74	0.49
13.7	11.6	0.3	2.34	SANDY SILT AND SILT	5 6	0.01	31	20	0.74	--
13.8	19.7	0.2	1.01	SILTY SAND	6 7	0.06	34	20	0.75	--
14.0	29.2	0.1	0.44	SILTY SAND	8 9	0.09	36	27	0.75	--
14.2	33.3	0.2	0.45	SILTY SAND	9 10	0.09	36	31	0.76	--
14.3	35.4	0.2	0.45	SILTY SAND	9 11	0.10	37	32	0.76	--
14.5	32.7	0.2	0.49	SILTY SAND	9 10	0.09	36	30	0.77	--
14.7	30.9	0.2	0.58	SILTY SAND	8 10	0.09	36	28	0.77	--
14.8	30.6	0.2	0.72	SILTY SAND	8 10	0.08	36	28	0.78	--
15.0	32.6	0.3	0.77	SILTY SAND	9 10	0.08	36	29	0.78	--
15.2	35.2	0.2	0.65	SILTY SAND	10 11	0.09	36	31	0.79	--
15.3	34.0	0.2	0.65	SILTY SAND	9 10	0.09	36	30	0.79	--
15.5	29.2	0.3	1.06	SILTY SAND	8 9	0.07	35	25	0.80	--
15.7	27.9	0.2	0.75	SILTY SAND	8 9	0.08	35	24	0.80	--
15.8	30.5	0.1	0.36	SILTY SAND	8 9	0.09	36	26	0.81	--
16.0	24.5	0.2	0.78	SILTY SAND	7 8	0.08	35	20	0.81	--
16.2	12.4	0.2	1.37	SANDY SILT AND SILT	4 5	0.03	31	20	0.82	--
16.3	7.4	0.1	1.09	SANDY SILT AND SILT	3 3	0.03	29	20	0.82	--
16.5	6.1	0.0	0.16	SANDY SILT AND SILT	2 2	0.05	28	20	0.83	--
16.7	5.9	0.0	0.17	SANDY SILT AND SILT	2 2	0.05	27	20	0.83	--
16.8	5.9	0.0	0.17	SANDY SILT AND SILT	2 2	0.05	27	20	0.84	--
17.0	6.1	0.0	0.49	SANDY SILT AND SILT	2 2	0.04	28	20	0.84	--
17.2	6.1	0.0	0.16	SANDY SILT AND SILT	2 2	0.05	27	20	0.85	--
17.3	6.0	0.0	0.33	SANDY SILT AND SILT	2 2	0.04	27	20	0.85	--
17.5	5.6	0.0	0.36	SANDY SILT AND SILT	2 2	0.04	27	20	0.86	--
17.7	6.1	0.0	0.49	SANDY SILT AND SILT	2 2	0.04	27	20	0.86	--
17.8	6.1	0.0	0.65	SANDY SILT AND SILT	2 2	0.04	27	20	0.87	--
18.0	5.4	0.0	0.55	SANDY SILT AND SILT	2 2	0.04	27	20	0.87	--
18.1	5.4	0.0	0.18	SANDY SILT AND SILT	2 2	0.05	27	20	0.88	--
18.3	5.4	0.0	0.55	SANDY SILT AND SILT	2 2	0.04	27	20	0.89	--
18.5	5.9	0.0	0.51	SANDY SILT AND SILT	2 2	0.04	27	20	0.89	--
18.6	6.1	0.0	0.33	SANDY SILT AND SILT	2 2	0.04	27	20	0.90	--
18.8	5.4	0.0	0.18	SANDY SILT AND SILT	2 2	0.05	27	20	0.90	--
19.0	5.4	0.0	0.18	SANDY SILT AND SILT	2 2	0.05	27	20	0.91	--
19.1	5.4	0.0	0.18	SANDY SILT AND SILT	2 2	0.05	27	20	0.91	--
19.3	5.2	0.0	0.19	SANDY SILT AND SILT	2 2	0.05	26	20	0.92	--
19.5	5.3	0.0	0.19	SANDY SILT AND SILT	2 2	0.05	26	20	0.92	--
19.6	5.4	0.0	0.18	SANDY SILT AND SILT	2 2	0.05	27	20	0.93	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 14

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE				SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
19.8	5.4	0.0	0.18	SANDY	SILT	AND	SILT	2	2	0.05	26	20	0.93	--
19.9	5.4	0.0	0.18	SANDY	SILT	AND	SILT	2	2	0.05	26	20	0.94	--
20.1	5.4	0.0	0.55	SANDY	SILT	AND	SILT	2	2	0.04	26	20	0.94	--
20.3	6.1	0.1	1.47	SANDY	SILT	AND	SILT	3	3	0.02	27	20	0.95	--
20.4	7.1	0.1	1.13	SANDY	SILT	AND	SILT	3	3	0.03	28	20	0.95	--
20.6	6.2	0.1	1.14	SANDY	SILT	AND	SILT	2	2	0.03	27	20	0.96	--
20.8	6.1	0.1	0.82	SANDY	SILT	AND	SILT	2	2	0.03	27	20	0.96	--
20.9	6.1	0.0	0.65	SANDY	SILT	AND	SILT	2	2	0.04	27	20	0.97	--
21.1	6.1	0.0	0.49	SANDY	SILT	AND	SILT	2	2	0.04	27	20	0.97	--
21.3	5.9	0.0	0.51	SANDY	SILT	AND	SILT	2	2	0.04	27	20	0.98	--
21.4	6.0	0.0	0.66	SANDY	SILT	AND	SILT	2	2	0.04	27	20	0.98	--
21.6	5.8	0.0	0.69	SANDY	SILT	AND	SILT	2	2	0.03	27	20	0.99	--
21.8	5.4	0.0	0.55	SANDY	SILT	AND	SILT	2	2	0.04	26	20	0.99	--
21.9	5.4	0.0	0.37	SANDY	SILT	AND	SILT	2	2	0.04	26	20	1.00	--
22.1	5.4	0.0	0.55	SANDY	SILT	AND	SILT	2	2	0.04	26	20	1.00	--
22.2	5.4	0.1	0.92	SANDY	SILT	AND	SILT	2	2	0.03	26	20	1.01	--
22.4	6.0	0.1	1.17	SANDY	SILT	AND	SILT	2	2	0.02	27	20	1.01	--
22.6	6.1	0.1	1.47	SANDY	SILT	AND	SILT	3	2	0.02	27	20	1.02	--
22.7	6.1	0.1	1.63	SANDY	SILT	AND	SILT	3	3	0.01	27	20	1.02	--
22.9	6.0	0.1	1.16	SANDY	SILT	AND	SILT	2	2	0.02	26	20	1.03	--
23.1	5.5	0.0	0.72	SANDY	SILT	AND	SILT	2	2	0.03	26	20	1.03	--
23.2	5.4	0.0	0.37	SANDY	SILT	AND	SILT	2	2	0.04	26	20	1.04	--
23.4	5.4	0.0	0.55	SANDY	SILT	AND	SILT	2	2	0.04	26	20	1.04	--
23.6	5.4	0.0	0.55	SANDY	SILT	AND	SILT	2	2	0.04	26	20	1.05	--
23.7	5.4	0.0	0.37	SANDY	SILT	AND	SILT	2	2	0.04	26	20	1.05	--
23.9	5.4	0.0	0.37	SANDY	SILT	AND	SILT	2	2	0.04	26	20	1.06	--
24.0	5.4	0.0	0.55	SANDY	SILT	AND	SILT	2	2	0.04	26	20	1.06	--
24.2	5.6	0.0	0.53	SANDY	SILT	AND	SILT	2	2	0.04	26	20	1.07	--
24.4	6.0	0.1	0.83	SANDY	SILT	AND	SILT	2	2	0.03	26	20	1.07	--
24.5	6.1	0.1	1.31	SANDY	SILT	AND	SILT	2	2	0.02	26	20	1.08	--
24.7	6.8	0.1	1.47	SANDY	SILT	AND	SILT	3	3	0.02	27	20	1.09	--
24.9	6.4	0.1	1.25	SANDY	SILT	AND	SILT	2	2	0.02	26	20	1.09	--
25.0	6.1	0.1	1.31	SANDY	SILT	AND	SILT	2	2	0.02	26	20	1.10	--
25.2	6.8	0.1	1.04	SANDY	SILT	AND	SILT	2	2	0.03	27	20	1.10	--
25.4	6.2	0.1	1.14	SANDY	SILT	AND	SILT	2	2	0.03	26	20	1.11	--
25.5	6.1	0.1	0.82	SANDY	SILT	AND	SILT	2	2	0.03	26	20	1.11	--
25.7	6.0	0.0	0.67	SANDY	SILT	AND	SILT	2	2	0.04	26	20	1.12	--
25.9	6.1	0.0	0.49	SANDY	SILT	AND	SILT	2	2	0.04	26	20	1.12	--
26.0	6.1	0.0	0.65	SANDY	SILT	AND	SILT	2	2	0.04	26	20	1.13	--
26.2	6.1	0.1	0.98	SANDY	SILT	AND	SILT	2	2	0.03	26	20	1.13	--
26.3	6.2	0.1	1.29	SANDY	SILT	AND	SILT	2	2	0.02	26	20	1.14	--
26.5	6.3	0.1	1.44	SANDY	SILT	AND	SILT	3	2	0.02	26	20	1.14	--
26.7	6.8	0.1	1.62	SANDY	SILT	AND	SILT	3	3	0.02	27	20	1.15	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 14

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
26.8	7.5	0.1	1.87	SANDY SILT AND SILT	3	3	0.01	27	20	1.15	--
27.0	7.9	0.1	1.78	SANDY SILT AND SILT	3	3	0.02	27	20	1.16	--
27.2	7.9	0.1	1.40	SANDY SILT AND SILT	3	3	0.02	27	20	1.16	--
27.3	7.5	0.1	1.73	SANDY SILT AND SILT	3	3	0.02	27	20	1.17	--
27.5	7.1	0.1	1.27	SANDY SILT AND SILT	3	3	0.02	27	20	1.17	--
27.7	6.1	0.1	0.82	SANDY SILT AND SILT	2	2	0.03	26	20	1.18	--
27.8	6.1	0.0	0.16	SANDY SILT AND SILT	2	2	0.05	26	20	1.18	--
28.0	6.1	0.0	0.16	SANDY SILT AND SILT	2	2	0.05	26	20	1.19	--
28.1	6.8	0.0	0.15	SANDY SILT AND SILT	2	2	0.05	26	20	1.19	--
28.3	6.8	0.0	0.15	SANDY SILT AND SILT	2	2	0.05	26	20	1.20	--
28.5	7.5	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	27	20	1.20	--
28.6	7.5	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	27	20	1.21	--
28.8	7.5	0.0	0.27	SANDY SILT AND SILT	2	2	0.04	27	20	1.21	--
29.0	8.2	0.1	0.86	SANDY SILT AND SILT	3	3	0.03	27	20	1.22	--
29.1	10.0	0.1	1.30	SANDY SILT AND SILT	4	3	0.03	28	20	1.22	--
29.3	10.2	0.2	1.77	SANDY SILT AND SILT	4	4	0.02	28	20	1.23	--
29.5	9.5	0.1	1.47	SANDY SILT AND SILT	4	3	0.03	28	20	1.23	--
29.6	8.4	0.1	1.31	SANDY SILT AND SILT	3	3	0.03	27	20	1.24	--
29.8	7.5	0.1	1.07	SANDY SILT AND SILT	3	2	0.03	27	20	1.24	--
30.0	7.5	0.1	1.07	SANDY SILT AND SILT	3	2	0.03	27	20	1.25	--
30.1	7.5	0.1	0.94	SANDY SILT AND SILT	3	2	0.03	27	20	1.25	--
30.3	7.5	0.1	1.34	SANDY SILT AND SILT	3	3	0.02	27	20	1.26	--
30.4	6.8	0.1	1.18	SANDY SILT AND SILT	3	2	0.03	26	20	1.26	--
30.6	6.8	0.1	1.03	SANDY SILT AND SILT	2	2	0.03	26	20	1.27	--
30.8	6.8	0.1	1.18	SANDY SILT AND SILT	3	2	0.03	26	20	1.28	--
30.9	6.8	0.1	1.47	SANDY SILT AND SILT	3	2	0.02	26	20	1.28	--
31.1	7.5	0.1	1.34	SANDY SILT AND SILT	3	3	0.02	26	20	1.29	--
31.3	7.4	0.1	1.34	SANDY SILT AND SILT	3	3	0.02	26	20	1.29	--
31.4	7.2	0.1	0.84	SANDY SILT AND SILT	3	2	0.03	26	20	1.30	--
31.6	7.5	0.1	0.67	SANDY SILT AND SILT	3	2	0.04	26	20	1.30	--
31.8	7.4	0.1	0.95	SANDY SILT AND SILT	3	2	0.03	26	20	1.31	--
31.9	8.2	0.1	0.98	SANDY SILT AND SILT	3	3	0.03	27	20	1.31	--
32.1	9.2	0.2	1.63	SANDY SILT AND SILT	4	3	0.02	27	20	1.32	--
32.3	9.1	0.2	2.65	CLAYEY SILT AND SILTY CLAY	4	4	----	----	1.32	0.44	--
32.4	9.1	0.3	3.19	CLAYEY SILT AND SILTY CLAY	5	4	----	----	1.33	0.44	--
32.6	8.8	0.3	2.94	CLAYEY SILT AND SILTY CLAY	4	4	----	----	1.33	0.43	--
32.7	8.8	0.2	2.49	CLAYEY SILT AND SILTY CLAY	4	4	----	----	1.34	0.42	--
32.9	8.8	0.2	2.15	SANDY SILT AND SILT	4	3	0.01	27	20	1.34	--
33.1	9.2	0.2	1.63	SANDY SILT AND SILT	4	3	0.02	27	20	1.35	--
33.2	8.8	0.2	1.70	SANDY SILT AND SILT	4	3	0.02	27	20	1.35	--
33.4	9.5	0.2	1.79	SANDY SILT AND SILT	4	3	0.02	27	20	1.36	--
33.6	10.0	0.2	1.80	SANDY SILT AND SILT	4	3	0.02	28	20	1.36	--
33.7	8.1	0.1	1.72	SANDY SILT AND SILT	3	3	0.02	27	20	1.37	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 14

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
33.9	6.8	0.1	1.18	SANDY SILT AND SILT	3	2	0.03	26	20	1.37	--
34.1	6.8	0.0	0.59	SANDY SILT AND SILT	2	2	0.04	26	20	1.38	--
34.2	6.3	0.0	0.47	SANDY SILT AND SILT	2	2	0.04	25	20	1.38	--
34.4	6.8	0.0	0.59	SANDY SILT AND SILT	2	2	0.04	26	20	1.39	--
34.5	6.8	0.1	0.88	SANDY SILT AND SILT	2	2	0.03	26	20	1.39	--
34.7	6.8	0.1	1.03	SANDY SILT AND SILT	2	2	0.03	26	20	1.40	--
34.9	6.1	0.0	0.65	SANDY SILT AND SILT	2	2	0.04	25	20	1.40	--
35.0	6.1	0.0	0.33	SANDY SILT AND SILT	2	2	0.04	25	20	1.41	--
35.2	6.8	0.0	0.44	SANDY SILT AND SILT	2	2	0.04	26	20	1.41	--
35.4	7.5	0.0	0.40	SANDY SILT AND SILT	2	2	0.04	26	20	1.42	--
35.5	7.5	0.0	0.53	SANDY SILT AND SILT	2	2	0.04	26	20	1.42	--
35.7	7.7	0.0	0.39	SANDY SILT AND SILT	2	2	0.04	26	20	1.42	--
35.9	8.8	0.1	0.90	SANDY SILT AND SILT	2	2	0.04	26	20	1.43	--
36.0	8.8	0.1	1.36	SANDY SILT AND SILT	3	3	0.03	27	20	1.43	--
36.2	9.5	0.2	1.58	SANDY SILT AND SILT	3	3	0.03	27	20	1.44	--
36.4	9.7	0.2	1.65	SANDY SILT AND SILT	4	3	0.02	27	20	1.44	--
36.5	10.2	0.2	1.47	SANDY SILT AND SILT	4	3	0.02	27	20	1.45	--
36.7	9.5	0.2	1.68	SANDY SILT AND SILT	4	3	0.03	27	20	1.45	--
36.8	10.9	0.2	1.38	SANDY SILT AND SILT	4	3	0.02	27	20	1.46	--
37.0	18.0	0.1	0.61	SANDY SILT AND SILT	4	3	0.03	28	20	1.47	--
37.2	18.4	0.2	0.87	SILTY SAND	5	4	0.07	30	20	1.47	--
37.3	18.5	0.2	0.98	SILTY SAND	5	5	0.06	30	20	1.48	--
37.5	15.6	0.4	2.37	SILTY SAND	6	5	0.06	30	20	1.48	--
37.7	21.6	0.5	2.08	SANDY SILT AND SILT	6	5	0.02	29	20	1.49	--
37.8	40.3	0.3	0.72	SANDY SILT AND SILT	8	6	0.04	31	20	1.49	--
38.0	48.4	0.2	0.39	SILTY SAND	11	9	0.09	34	20	1.50	--
38.2	40.3	0.4	0.89	SAND	8	7	0.98	35	22	1.50	--
38.3	28.6	0.3	0.88	SILTY SAND	11	9	0.09	34	20	1.51	--
38.5	18.3	0.2	1.09	SILTY SAND	8	7	0.08	32	20	1.51	--
38.6	14.7	0.2	1.22	SILTY SAND	6	5	0.05	30	20	1.52	--
38.8	13.6	0.2	1.62	SANDY SILT AND SILT	5	4	0.04	29	20	1.52	--
39.0	21.8	0.2	0.74	SANDY SILT AND SILT	5	4	0.03	29	20	1.53	--
39.1	16.2	0.2	1.17	SILTY SAND	6	5	0.07	31	20	1.53	--
39.3	13.6	0.2	1.47	SANDY SILT AND SILT	5	4	0.05	29	20	1.54	--
39.5	13.0	0.2	1.47	SANDY SILT AND SILT	5	4	0.03	28	20	1.54	--
39.6	12.6	0.2	1.59	SANDY SILT AND SILT	5	4	0.03	28	20	1.55	--
39.8	12.9	0.2	1.86	SANDY SILT AND SILT	5	4	0.03	28	20	1.55	--
40.0	13.7	0.3	2.18	SANDY SILT AND SILT	5	4	0.03	28	20	1.56	--
40.1	15.2	0.3	1.91	SANDY SILT AND SILT	5	4	0.02	28	20	1.56	--
40.3	13.4	0.2	1.64	SANDY SILT AND SILT	6	4	0.03	29	20	1.57	--
40.5	11.3	0.2	1.95	SANDY SILT AND SILT	5	4	0.03	28	20	1.57	--
40.6	14.3	0.2	1.68	SANDY SILT AND SILT	4	4	0.02	27	20	1.58	--
40.8	15.6	0.4	2.81	SANDY SILT AND SILT	5	4	0.03	29	20	1.58	--
				SANDY SILT AND SILT	7	5	0.01	29	20	1.59	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 14

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	P0 TSF	SU TSF.	
40.9	15.0	0.4	2.47	SANDY SILT AND SILT	6	5	0.02	29	20	1.59	--
41.1	12.9	0.3	2.17	SANDY SILT AND SILT	5	4	0.02	28	20	1.60	--
41.3	13.6	0.3	2.13	SANDY SILT AND SILT	5	4	0.02	28	20	1.60	--
41.4	15.6	0.4	2.81	SANDY SILT AND SILT	7	5	0.01	29	20	1.61	--
41.6	15.9	0.5	2.83	SANDY SILT AND SILT	7	6	0.01	29	20	1.61	--
41.8	14.3	0.4	2.73	SANDY SILT AND SILT	6	5	0.01	28	20	1.62	--
41.9	14.3	0.3	2.31	SANDY SILT AND SILT	6	5	0.02	28	20	1.62	--
42.1	14.3	0.3	2.31	SANDY SILT AND SILT	6	5	0.02	28	20	1.63	--
42.3	12.2	0.3	2.53	SANDY SILT AND SILT	5	4	0.01	28	20	1.63	--
42.4	11.6	0.3	2.34	SANDY SILT AND SILT	5	4	0.01	27	20	1.64	--
42.6	12.3	0.3	2.36	SANDY SILT AND SILT	5	4	0.01	28	20	1.64	--
42.7	14.4	0.4	2.51	SANDY SILT AND SILT	6	5	0.02	28	20	1.65	--
42.9	14.9	0.4	2.68	SANDY SILT AND SILT	6	5	0.01	29	20	1.66	--
43.1	13.9	0.4	3.02	CLAYEY SILT AND SILTY CLAY	6	5	---	---	---	1.66	0.70
43.2	12.9	0.4	2.79	CLAYEY SILT AND SILTY CLAY	6	5	---	---	---	1.67	0.64
43.4	13.1	0.3	2.28	SANDY SILT AND SILT	5	4	0.02	28	20	1.67	--
43.6	12.9	0.4	2.71	CLAYEY SILT AND SILTY CLAY	6	4	---	---	---	1.68	0.64
43.7	15.6	0.3	2.17	SANDY SILT AND SILT	6	5	0.02	29	20	1.68	--
43.9	17.7	0.4	2.43	SANDY SILT AND SILT	7	5	0.02	29	20	1.69	--
44.1	17.5	0.5	2.74	SANDY SILT AND SILT	7	6	0.02	29	20	1.69	--
44.2	17.7	0.4	2.43	SANDY SILT AND SILT	7	5	0.02	29	20	1.70	--
44.4	17.6	0.4	2.22	SANDY SILT AND SILT	7	5	0.03	29	20	1.70	--
44.6	15.9	0.4	2.46	SANDY SILT AND SILT	6	5	0.02	29	20	1.71	--
44.7	15.6	0.4	2.75	SANDY SILT AND SILT	7	5	0.01	29	20	1.71	--
44.9	14.3	0.4	2.73	SANDY SILT AND SILT	6	5	0.01	28	20	1.72	--
45.0	14.6	0.3	1.99	SANDY SILT AND SILT	5	4	0.03	28	20	1.72	--
45.2	17.0	0.3	1.59	SANDY SILT AND SILT	6	4	0.04	29	20	1.73	--
45.4	15.6	0.2	1.41	SANDY SILT AND SILT	5	4	0.04	29	20	1.73	--
45.5	13.6	0.2	1.18	SANDY SILT AND SILT	5	3	0.04	28	20	1.74	--
45.7	12.3	0.2	1.46	SANDY SILT AND SILT	4	3	0.03	27	20	1.74	--
45.9	11.6	0.2	1.90	SANDY SILT AND SILT	5	3	0.02	27	20	1.75	--
46.0	11.6	0.2	1.64	SANDY SILT AND SILT	4	3	0.03	27	20	1.75	--
46.2	10.5	0.1	1.34	SANDY SILT AND SILT	4	3	0.03	27	20	1.76	--
46.4	10.2	0.1	0.98	SANDY SILT AND SILT	4	3	0.04	26	20	1.76	--
46.5	10.2	0.1	0.69	SANDY SILT AND SILT	3	3	0.04	26	20	1.77	--
46.7	10.6	0.1	1.03	SANDY SILT AND SILT	4	3	0.04	27	20	1.77	--
46.8	12.9	0.1	0.54	SILTY SAND	4	3	0.06	28	20	1.78	--
47.0	10.9	0.0	0.28	SILTY SAND	3	2	0.06	27	20	1.78	--
47.2	9.5	0.0	0.32	SANDY SILT AND SILT	3	2	0.05	26	20	1.79	--
47.3	9.5	0.0	0.21	SANDY SILT AND SILT	3	2	0.05	26	20	1.79	--
47.5	8.9	0.0	0.34	SANDY SILT AND SILT	3	2	0.04	26	20	1.80	--
47.7	9.5	0.0	0.11	SILTY SAND	3	2	0.05	26	20	1.80	--
47.8	8.8	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	26	20	1.81	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 14

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
48.0	7.6	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	25	20	1.81	--
48.2	8.8	0.1	0.79	SANDY SILT AND SILT	3	2	0.04	26	20	1.82	--
48.3	13.7	0.1	1.02	SANDY SILT AND SILT	4	3	0.04	28	20	1.82	--
48.5	17.7	0.2	1.24	SANDY SILT AND SILT	6	4	0.05	29	20	1.83	--
48.7	35.8	0.4	1.03	SILTY SAND	10	7	0.08	32	20	1.83	--
48.8	24.7	0.5	1.94	SANDY SILT AND SILT	8	6	0.04	31	20	1.84	--
49.0	19.0	0.4	1.84	SANDY SILT AND SILT	7	5	0.04	29	20	1.85	--
49.1	19.8	0.4	2.02	SANDY SILT AND SILT	7	5	0.03	29	20	1.85	--
49.3	19.7	0.4	2.23	SANDY SILT AND SILT	7	5	0.03	29	20	1.86	--
49.5	18.4	0.4	2.02	SANDY SILT AND SILT	7	5	0.03	29	20	1.86	--
49.6	16.8	0.3	1.60	SANDY SILT AND SILT	6	4	0.04	29	20	1.87	--
49.8	15.0	0.2	1.60	SANDY SILT AND SILT	5	4	0.03	28	20	1.87	--
50.0	15.6	0.1	0.58	SILTY SAND	5	3	0.07	28	20	1.88	--
50.1	14.3	0.1	0.42	SILTY SAND	4	3	0.07	28	20	1.88	--
50.3	12.9	0.0	0.23	SILTY SAND	4	3	0.08	27	20	1.89	--
50.5	12.9	0.0	0.23	SILTY SAND	4	3	0.08	27	20	1.89	--
50.6	13.0	0.0	0.31	SILTY SAND	4	3	0.08	27	20	1.90	--
50.8	13.6	0.1	0.74	SANDY SILT AND SILT	4	3	0.05	27	20	1.90	--
51.0	13.4	0.1	0.60	SILTY SAND	4	3	0.05	27	20	1.91	--
51.1	13.0	0.1	0.39	SILTY SAND	4	3	0.07	27	20	1.91	--
51.3	13.4	0.1	0.45	SILTY SAND	4	3	0.07	27	20	1.92	--
51.4	13.6	0.1	0.44	SILTY SAND	4	3	0.07	27	20	1.92	--
51.6	13.3	0.3	2.03	SANDY SILT AND SILT	5	4	0.02	27	20	1.93	--
51.8	27.2	0.5	1.65	SILTY SAND	8	6	0.05	31	20	1.93	--
51.9	46.9	0.2	0.43	SAND	9	6	0.84	33	20	1.94	--
52.1	29.2	0.0	0.03	SILTY SAND	8	5	0.10	31	20	1.94	--
52.3	32.7	0.5	1.53	SILTY SAND	10	7	0.06	32	20	1.95	--
52.4	19.4	0.5	2.68	SANDY SILT AND SILT	8	6	0.02	29	20	1.95	--
52.6	27.5	0.3	1.05	SILTY SAND	8	6	0.07	31	20	1.96	--
52.8	20.3	0.2	1.08	SILTY SAND	6	4	0.06	29	20	1.96	--
52.9	16.2	0.1	0.43	SILTY SAND	5	3	0.08	28	20	1.97	--
53.1	14.2	0.0	0.21	SILTY SAND	4	3	0.09	27	20	1.97	--
53.2	12.2	0.1	0.41	SILTY SAND	4	3	0.06	27	20	1.98	--
53.4	12.9	0.1	0.62	SANDY SILT AND SILT	4	3	0.05	27	20	1.98	--
53.6	12.3	0.1	0.98	SANDY SILT AND SILT	4	3	0.04	27	20	1.99	--
53.7	12.2	0.1	1.06	SANDY SILT AND SILT	4	3	0.04	27	20	1.99	--
53.9	12.2	0.2	1.23	SANDY SILT AND SILT	4	3	0.04	27	20	2.00	--
54.1	12.2	0.1	1.15	SANDY SILT AND SILT	4	3	0.04	27	20	2.00	--
54.2	11.6	0.1	0.95	SANDY SILT AND SILT	4	3	0.04	26	20	2.01	--
54.4	12.9	0.1	0.93	SANDY SILT AND SILT	4	3	0.04	27	20	2.01	--
54.6	12.9	0.2	1.32	SANDY SILT AND SILT	4	3	0.04	27	20	2.02	--
54.7	14.3	0.2	1.12	SANDY SILT AND SILT	5	3	0.04	27	20	2.02	--
54.9	13.4	0.1	1.05	SANDY SILT AND SILT	4	3	0.04	27	20	2.03	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 14

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
55.1	12.2	0.1	0.82	SANDY SILT AND SILT	4	3	0.04	27	20	2.04	--
55.2	12.5	0.1	0.80	SANDY SILT AND SILT	4	3	0.05	27	20	2.04	--
55.4	12.2	0.1	1.06	SANDY SILT AND SILT	4	3	0.04	27	20	2.05	--
55.5	12.9	0.2	1.39	SANDY SILT AND SILT	5	3	0.03	27	20	2.05	--
55.7	11.6	0.2	1.64	SANDY SILT AND SILT	4	3	0.03	26	20	2.06	--
55.9	10.9	0.2	1.38	SANDY SILT AND SILT	4	3	0.03	26	20	2.06	--
56.0	10.9	0.1	1.01	SANDY SILT AND SILT	4	3	0.04	26	20	2.07	--
56.2	11.6	0.1	0.95	SANDY SILT AND SILT	4	3	0.04	26	20	2.07	--
56.4	12.5	0.2	1.20	SANDY SILT AND SILT	4	3	0.04	27	20	2.08	--
56.5	14.3	0.3	1.89	SANDY SILT AND SILT	5	4	0.03	27	20	2.08	--
56.7	15.0	0.3	2.27	SANDY SILT AND SILT	6	4	0.02	27	20	2.09	--
56.9	14.3	0.4	2.45	SANDY SILT AND SILT	6	4	0.02	27	20	2.09	--
57.0	14.7	0.3	2.24	SANDY SILT AND SILT	6	4	0.02	27	20	2.10	--
57.2	13.0	0.3	2.38	SANDY SILT AND SILT	5	4	0.02	27	20	2.10	--
57.3	12.9	0.2	1.55	SANDY SILT AND SILT	5	3	0.03	27	20	2.11	--
57.5	13.0	0.2	1.16	SANDY SILT AND SILT	4	3	0.04	27	20	2.11	--
57.7	12.2	0.2	1.23	SANDY SILT AND SILT	4	3	0.04	26	20	2.12	--
57.8	11.6	0.1	1.04	SANDY SILT AND SILT	4	3	0.04	26	20	2.12	--
58.0	11.4	0.1	0.88	SANDY SILT AND SILT	4	3	0.04	26	20	2.13	--
58.2	11.6	0.1	0.95	SANDY SILT AND SILT	4	3	0.04	26	20	2.13	--
58.3	11.7	0.2	1.37	SANDY SILT AND SILT	4	3	0.03	26	20	2.14	--
58.5	13.6	0.2	1.25	SANDY SILT AND SILT	5	3	0.04	27	20	2.14	--
58.7	13.0	0.2	1.39	SANDY SILT AND SILT	5	3	0.03	27	20	2.15	--
58.8	12.8	0.2	1.57	SANDY SILT AND SILT	5	3	0.03	27	20	2.15	--
59.0	14.3	0.3	1.89	SANDY SILT AND SILT	5	4	0.03	27	20	2.16	--
59.2	17.0	0.4	2.41	SANDY SILT AND SILT	7	4	0.02	28	20	2.16	--
59.3	19.7	0.5	2.54	SANDY SILT AND SILT	8	5	0.02	29	20	2.17	--
59.5	20.1	0.5	2.29	SANDY SILT AND SILT	7	5	0.03	29	20	2.17	--
59.6	15.7	0.3	2.17	SANDY SILT AND SILT	6	4	0.02	27	20	2.18	--
59.8	16.2	0.3	2.09	SANDY SILT AND SILT	6	4	0.03	28	20	2.18	--
60.0	18.2	0.3	1.65	SANDY SILT AND SILT	6	4	0.04	28	20	2.19	--
60.1	17.0	0.2	1.41	SANDY SILT AND SILT	6	4	0.04	28	20	2.19	--
60.3	15.6	0.2	1.28	SANDY SILT AND SILT	5	3	0.04	27	20	2.20	--
60.5	16.2	0.2	1.18	SANDY SILT AND SILT	5	3	0.05	28	20	2.20	--
60.6	18.8	0.3	1.44	SANDY SILT AND SILT	6	4	0.04	28	20	2.21	--
60.8	19.7	0.2	0.76	SILTY SAND	6	4	0.07	29	20	2.21	--
61.0	18.4	0.2	1.31	SANDY SILT AND SILT	6	4	0.05	28	20	2.22	--
61.1	21.1	0.3	1.37	SILTY SAND	6	4	0.05	29	20	2.23	--
61.3	20.5	0.5	2.34	SANDY SILT AND SILT	8	5	0.03	29	20	2.23	--
61.4	34.7	0.5	1.30	SILTY SAND	10	6	0.07	31	20	2.24	--
61.6	48.5	0.4	0.87	SILTY SAND	13	8	0.10	33	20	2.24	--
61.7	61.9	0.9	1.50	SILTY SAND	17	11	0.08	34	20	2.24	--
61.9	43.2	0.9	1.97	SILTY SAND	13	8	0.06	32	20	2.25	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 14

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	P0 TSF	SU TSF.	
62.1	39.2	0.8	2.02	SILTY SAND	12	8	0.05	32	20	2.25	--
62.2	72.8	0.6	0.87	SAND	15	9	0.63	35	26	2.26	--
62.4	98.5	0.6	0.57	SAND	15	10	1.27	36	37	2.27	--
62.6	121.7	0.6	0.46	SAND	18	11	1.48	37	44	2.27	--
62.7	120.4	1.3	1.07	SAND	23	15	0.78	37	44	2.28	--
62.9	135.3	1.2	0.90	SAND	23	15	1.03	38	48	2.28	--
63.1	141.4	0.9	0.65	SAND	22	14	1.32	38	50	2.29	--
63.2	170.7	1.9	1.12	SAND	30	19	0.92	39	58	2.29	--
63.4	223.0	2.3	1.03	SAND	36	23	1.15	40	70	2.30	--
63.5	177.7	2.1	1.18	SAND	32	20	0.89	39	60	2.30	--
63.7	167.3	1.5	0.87	SAND	27	17	1.16	39	57	2.31	--
63.9	145.8	1.7	1.13	SAND	27	17	0.83	38	51	2.31	--
64.0	163.9	1.6	0.99	SAND	28	18	1.03	39	56	2.32	--
64.2	172.0	1.3	0.74	SAND	27	17	1.29	39	58	2.32	--
64.4	172.0	1.2	0.72	SAND	27	17	1.31	39	58	2.33	--
64.5	159.3	1.5	0.94	SAND	27	17	1.07	39	55	2.33	--
64.7	130.4	1.7	1.28	SAND	27	17	0.60	38	46	2.34	--
64.9	114.9	1.1	0.97	SAND	21	13	0.86	37	42	2.34	--
65.0	118.8	1.0	0.82	SAND	20	13	1.06	37	43	2.35	--
65.2	107.7	1.1	0.99	SAND	20	13	0.79	37	40	2.35	--
65.4	102.0	1.1	1.07	SAND	20	13	0.66	36	37	2.36	--
65.5	97.4	1.1	1.12	SAND	20	13	0.56	36	35	2.36	--
65.7	98.6	0.8	0.83	SAND	17	11	0.94	36	36	2.37	--
65.8	100.0	0.9	0.89	SAND	18	11	0.87	36	36	2.37	--
66.0	89.1	1.2	1.39	SAND	23	15	0.11	36	32	2.38	--
66.2	115.6	1.0	0.83	SAND	20	12	1.03	37	42	2.38	--
66.3	137.5	1.1	0.78	SAND	22	14	1.17	38	48	2.39	--
66.5	166.5	1.2	0.73	SAND	26	16	1.29	39	56	2.39	--
66.7	207.4	1.2	0.60	SAND	30	19	1.49	40	65	2.40	--
66.8	248.2	1.5	0.62	SAND	35	22	1.58	41	73	2.40	--
67.0	251.1	1.7	0.69	SAND	36	22	1.53	41	74	2.41	--
67.2	253.6	1.7	0.68	SAND	36	22	1.54	41	74	2.41	--
67.3	259.0	1.7	0.67	SAND	37	23	1.56	41	75	2.42	--
67.5	263.8	1.7	0.65	SAND	37	23	1.60	41	76	2.42	--
67.7	255.9	1.8	0.70	SAND	37	23	1.53	41	74	2.43	--
67.8	253.6	1.7	0.65	SAND	36	22	1.57	41	74	2.43	--
68.0	253.8	1.7	0.68	SAND	36	22	1.54	41	74	2.44	--
68.1	284.5	2.0	0.71	SAND	40	25	1.59	41	79	2.44	--
68.3	367.9	1.4	0.38	SAND AND GRAVEL	45	27	2.39	42	89	2.45	--
68.5	452.9	1.5	0.34	SAND AND GRAVEL	51	31	3.23	43	99	2.46	--
68.6	372.1	1.4	0.38	SAND AND GRAVEL	45	27	2.43	42	89	2.46	--
68.8	280.8	1.6	0.57	SAND	38	23	1.71	41	78	2.47	--
69.0	224.7	1.7	0.73	SAND	33	20	1.42	40	68	2.47	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 14

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.
69.1	192.4	1.6	0.82	SAND	30 18	1.26	39	61	2.48	--
69.3	164.6	1.6	0.99	SAND	28 17	1.04	38	54	2.48	--
69.5	159.7	1.9	1.16	SAND	29 18	0.86	38	53	2.49	--
69.6	177.4	2.0	1.14	SAND	31 19	0.93	39	58	2.49	--
69.8	210.3	1.4	0.65	SAND	31 19	1.45	40	65	2.50	--
69.9	253.0	1.7	0.66	SAND	36 23	1.56	40	73	2.50	--
70.1	303.3	2.1	0.68	SAND	42 27	1.66	41	81	2.51	--
70.3	335.2	2.6	0.78	SAND	47 30	1.64	42	85	2.51	--
70.4	341.5	3.2	0.94	SAND	50 32	1.50	42	85	2.52	--
70.6	339.3	3.5	1.03	SAND	51 32	1.41	42	85	2.52	--
70.8	335.0	3.6	1.07	SAND	51 32	1.37	42	85	2.53	--
70.9	338.6	3.5	1.04	SAND	51 32	1.40	42	85	2.53	--
71.1	346.1	3.5	1.02	SAND	51 33	1.44	42	86	2.54	--
71.3	338.6	3.5	1.02	SAND	50 32	1.42	42	85	2.54	--
71.4	321.6	3.2	0.99	SAND	48 30	1.42	42	83	2.55	--
71.6	299.7	2.6	0.87	SAND	44 28	1.48	41	80	2.55	--
71.8	276.8	2.4	0.85	SAND	41 26	1.45	41	77	2.56	--
71.9	257.7									
72.1	246.8	2.4	0.98	SAND	39 24	1.26	40	71	2.57	--

INTERPRETED CONE PENETRATION TEST DATA

C. P.T. - 17

CLIENT: HOWARD HUGHES PROPERTIES
 JOB NO: AE-88473
 DATE: JANUARY 30, 1989
 ELEVATION: 18.2 FT
 UNIT WEIGHT OF SOIL: 125 PCF
 DEPTH TO GROUND WATER: 14 FT

DEPTH FT.	QC TSF	FS TSF	FR %	SOIL BEHAVIOR TYPE	SPT N	D50 MM	PHI DEG	DR %	PO TSF	SU TSF
0.1	57.0	0.0	0.02	SAND	7	27	1.96	--	0.01	--
0.3	163.9	0.0	0.02	SAND	21	67	1.98	--	0.02	--
0.4	178.2	1.1	0.60	SAND	26	78	1.44	--	0.03	--
0.6	206.9	2.5	1.22	SAND	37	99	0.93	--	0.04	--
0.8	229.0	2.9	1.25	SAND	40	99	0.97	--	0.05	--
0.9	246.8	3.2	1.31	SAND	43	99	0.96	--	0.06	--
1.1	250.8	2.8	1.12	SAND	41	99	1.14	--	0.07	--
1.2	189.2	2.6	1.36	SAND	36	86	0.76	--	0.08	--
1.4	116.3	2.5	2.14	SILTY SAND	32	75	0.08	--	0.09	--
1.6	72.8	2.6	3.63	SANDY SILT AND SILT	28	64	0.02	--	0.10	--
1.7	60.4	2.7	4.41	CLAYEY SILT AND SILTY CLAY	29	63	---	--	0.11	3.77
1.9	127.7	2.9	2.28	SILTY SAND	36	77	0.08	--	0.12	--
2.1	133.8	2.9	2.20	SILTY SAND	37	78	0.08	--	0.13	--
2.2	89.4	3.5	3.93	SANDY SILT AND SILT	36	75	0.02	--	0.14	--
2.4	72.2	2.6	3.66	SANDY SILT AND SILT	28	57	0.02	--	0.15	--
2.6	60.6	2.3	3.75	SANDY SILT AND SILT	25	50	0.02	--	0.16	--
2.7	56.4	1.4	2.39	SILTY SAND	17	34	0.05	--	0.17	--
2.9	56.4	0.9	1.67	SILTY SAND	16	31	0.08	--	0.18	--
3.1	53.5	0.5	0.90	SILTY SAND	14	27	0.10	--	0.19	--
3.2	54.4	0.6	1.10	SILTY SAND	15	27	0.09	--	0.20	--
3.4	54.8	0.5	0.97	SILTY SAND	15	27	0.10	--	0.21	--
3.5	50.7	0.8	1.58	SILTY SAND	14	26	0.07	--	0.22	--
3.7	57.1	0.9	1.49	SILTY SAND	16	28	0.08	--	0.23	--
3.9	43.8	0.7	1.48	SILTY SAND	13	22	0.07	--	0.24	--
4.0	42.8	0.5	1.19	SILTY SAND	12	21	0.08	43	59	0.25
4.2	38.6	0.5	1.22	SILTY SAND	11	19	0.08	42	55	0.26
4.3	38.0	0.5	1.42	SILTY SAND	11	19	0.07	42	54	0.27
4.5	33.4	0.7	2.16	SANDY SILT AND SILT	11	18	0.05	41	51	0.28
4.7	32.0	0.9	2.91	SANDY SILT AND SILT	12	20	0.03	41	49	0.29
4.8	30.2	1.1	3.51	CLAYEY SILT AND SILTY CLAY	13	22	---	--	0.30	1.87
5.0	29.1	1.0	3.57	CLAYEY SILT AND SILTY CLAY	13	21	---	--	0.31	1.80
5.2	27.9	0.8	2.97	SANDY SILT AND SILT	11	18	0.02	40	44	0.32
5.3	24.3	0.7	2.71	SANDY SILT AND SILT	9	15	0.02	39	40	0.33
5.5	23.7	0.4	1.64	SANDY SILT AND SILT	7	12	0.05	39	39	0.34

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 17

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	P0 TSF	SU TSF.
5.6	24.5	0.2	0.77	SILTY SAND	7 11	0.08	39	40	0.35	--
5.8	23.8	0.2	0.71	SILTY SAND	7 10	0.08	38	38	0.36	--
6.0	23.0	0.2	0.87	SILTY SAND	7 10	0.07	38	37	0.37	--
6.1	23.0	0.1	0.57	SILTY SAND	6 10	0.08	38	36	0.38	--
6.3	21.5	0.1	0.61	SILTY SAND	6 9	0.08	37	34	0.39	--
6.5	21.0	0.1	0.62	SILTY SAND	6 9	0.08	37	33	0.40	--
6.6	18.4	0.3	1.58	SANDY SILT AND SILT	6 9	0.04	36	29	0.41	--
6.8	17.0	0.3	1.82	SANDY SILT AND SILT	6 9	0.03	36	27	0.42	--
7.0	19.8	0.1	0.71	SILTY SAND	6 8	0.07	37	30	0.43	--
7.1	20.6	0.1	0.29	SILTY SAND	6 8	0.09	37	31	0.44	--
7.3	21.6	0.1	0.42	SILTY SAND	6 8	0.09	37	31	0.46	--
7.4	22.4	0.1	0.53	SILTY SAND	6 9	0.08	37	32	0.47	--
7.6	21.9	0.1	0.50	SILTY SAND	6 9	0.08	37	31	0.48	--
7.8	21.9	0.1	0.64	SILTY SAND	6 9	0.08	37	30	0.49	--
7.9	23.7	0.1	0.46	SILTY SAND	7 9	0.08	37	32	0.50	--
8.1	23.8	0.3	1.34	SILTY SAND	7 10	0.06	37	32	0.51	--
8.3	17.8	0.4	2.13	SANDY SILT AND SILT	7 9	0.03	35	24	0.52	--
8.4	18.9	0.5	2.49	SANDY SILT AND SILT	7 10	0.02	35	25	0.53	--
8.6	22.0	0.5	2.46	SANDY SILT AND SILT	8 11	0.03	36	28	0.54	--
8.8	25.4	0.5	2.08	SANDY SILT AND SILT	9 11	0.04	37	31	0.55	--
8.9	23.3	0.6	2.67	SANDY SILT AND SILT	9 12	0.02	36	29	0.56	--
9.1	15.4	0.5	3.18	CLAYEY SILT AND SILTY CLAY	7 9	---	---	---	0.57	0.93
9.3	12.4	0.3	2.26	SANDY SILT AND SILT	5 7	0.02	33	20	0.58	--
9.4	22.6	0.3	1.51	SANDY SILT AND SILT	7 9	0.05	36	26	0.59	--
9.6	14.3	0.3	1.75	SANDY SILT AND SILT	5 7	0.03	33	20	0.60	--
9.7	9.7	0.1	1.14	SANDY SILT AND SILT	3 4	0.03	31	20	0.61	--
9.9	8.9	0.1	0.68	SANDY SILT AND SILT	3 4	0.04	31	20	0.62	--
10.1	8.6	0.0	0.46	SANDY SILT AND SILT	3 4	0.04	31	20	0.63	--
10.2	8.8	0.0	0.34	SANDY SILT AND SILT	3 4	0.04	31	20	0.64	--
10.4	13.1	0.1	0.53	SILTY SAND	4 5	0.06	33	20	0.65	--
10.6	37.4	0.2	0.40	SILTY SAND	10 12	0.10	38	38	0.66	--
10.7	39.1	0.4	0.92	SILTY SAND	11 13	0.09	38	39	0.67	--
10.9	19.0	0.4	2.00	SANDY SILT AND SILT	7 8	0.03	34	20	0.68	--
11.1	13.0	0.2	1.62	SANDY SILT AND SILT	5 6	0.03	32	20	0.69	--
11.2	10.9	0.0	0.37	SANDY SILT AND SILT	3 4	0.05	31	20	0.70	--
11.4	8.0	0.1	0.75	SANDY SILT AND SILT	3 3	0.04	30	20	0.71	--
11.5	9.4	0.1	0.74	SANDY SILT AND SILT	3 4	0.04	30	20	0.72	--
11.7	43.4	0.1	0.21	SAND	7 8	1.34	38	39	0.73	--
11.9	53.1	0.2	0.34	SAND	8 10	1.24	39	45	0.74	--
12.0	51.4	0.2	0.31	SAND	8 9	1.27	39	44	0.75	--
12.2	58.4	0.2	0.27	SAND	9 10	1.46	39	47	0.76	--
12.4	61.6	0.2	0.36	SAND	9 11	1.34	39	49	0.77	--
12.5	61.2	0.2	0.36	SAND	9 11	1.33	39	48	0.78	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 17

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.
12.7	66.0	0.2	0.30	SAND	10 11	1.48	40	50	0.79	--
12.9	67.7	0.2	0.32	SAND	10 11	1.45	40	51	0.80	--
13.0	71.6	0.2	0.34	SAND	11 12	1.46	40	52	0.81	--
13.2	66.3	0.3	0.42	SAND	10 11	1.27	39	50	0.82	--
13.4	56.0	0.3	0.45	SAND	9 10	1.07	38	44	0.83	--
13.5	43.3	0.2	0.37	SAND	8 9	0.82	37	35	0.84	--
13.7	24.5	0.2	0.74	SILTY SAND	7 7	0.08	34	20	0.86	--
13.8	9.6	0.1	1.05	SANDY SILT AND SILT	3 4	0.03	30	20	0.87	--
14.0	6.8	0.0	0.44	SANDY SILT AND SILT	2 2	0.04	28	20	0.88	--
14.2	6.3	0.0	0.47	SANDY SILT AND SILT	2 2	0.04	27	20	0.88	--
14.3	6.1	0.0	0.49	SANDY SILT AND SILT	2 2	0.04	27	20	0.89	--
14.5	6.1	0.0	0.49	SANDY SILT AND SILT	2 2	0.04	27	20	0.89	--
14.7	6.1	0.0	0.49	SANDY SILT AND SILT	2 2	0.04	27	20	0.90	--
14.8	7.5	0.0	0.40	SANDY SILT AND SILT	2 3	0.04	28	20	0.90	--
15.0	6.6	0.0	0.60	SANDY SILT AND SILT	2 2	0.04	28	20	0.91	--
15.2	7.5	0.0	0.40	SANDY SILT AND SILT	2 3	0.04	28	20	0.91	--
15.3	6.8	0.0	0.44	SANDY SILT AND SILT	2 2	0.04	28	20	0.92	--
15.5	6.8	0.0	0.59	SANDY SILT AND SILT	2 2	0.04	28	20	0.92	--
15.7	8.8	0.0	0.46	SANDY SILT AND SILT	3 3	0.04	29	20	0.93	--
15.8	7.5	0.0	0.53	SANDY SILT AND SILT	2 3	0.04	28	20	0.93	--
16.0	8.8	0.0	0.45	SANDY SILT AND SILT	3 3	0.04	29	20	0.94	--
16.2	8.5	0.0	0.47	SANDY SILT AND SILT	3 3	0.04	29	20	0.94	--
16.3	15.0	0.2	1.20	SANDY SILT AND SILT	5 5	0.04	31	20	0.95	--
16.5	14.3	0.2	1.05	SANDY SILT AND SILT	5 5	0.04	31	20	0.95	--
16.7	13.0	0.1	0.92	SANDY SILT AND SILT	4 4	0.04	31	20	0.96	--
16.8	12.9	0.1	0.70	SANDY SILT AND SILT	4 4	0.05	31	20	0.96	--
17.0	9.2	0.1	0.98	SANDY SILT AND SILT	3 3	0.03	29	20	0.97	--
17.2	8.2	0.1	0.98	SANDY SILT AND SILT	3 3	0.03	28	20	0.97	--
17.3	7.7	0.1	0.92	SANDY SILT AND SILT	3 3	0.03	28	20	0.98	--
17.5	8.2	0.1	0.98	SANDY SILT AND SILT	3 3	0.03	28	20	0.98	--
17.7	8.8	0.1	0.79	SANDY SILT AND SILT	3 3	0.04	29	20	0.99	--
17.8	8.8	0.1	1.02	SANDY SILT AND SILT	3 3	0.03	29	20	0.99	--
18.0	8.6	0.1	1.16	SANDY SILT AND SILT	3 3	0.03	28	20	1.00	--
18.1	8.2	0.2	2.45	CLAYEY SILT AND SILTY CLAY	4 4	---	---	---	1.00	0.44
18.3	11.9	0.3	2.53	SANDY SILT AND SILT	5 5	0.01	30	20	1.01	--
18.5	18.3	0.2	1.31	SANDY SILT AND SILT	6 6	0.05	32	20	1.01	--
18.6	26.8	0.3	0.97	SILTY SAND	8 8	0.07	34	20	1.02	--
18.8	33.4	0.2	0.69	SILTY SAND	9 9	0.09	35	22	1.03	--
19.0	27.9	0.2	0.79	SILTY SAND	8 8	0.08	34	20	1.03	--
19.1	23.8	0.2	0.80	SILTY SAND	7 7	0.07	33	20	1.04	--
19.3	17.7	0.2	0.90	SILTY SAND	5 5	0.06	32	20	1.04	--
19.5	11.6	0.2	1.64	SANDY SILT AND SILT	4 4	0.03	30	20	1.05	--
19.6	12.9	0.2	1.32	SANDY SILT AND SILT	4 4	0.04	30	20	1.05	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 17

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
19.8	11.6	0.1	0.87	SANDY SILT AND SILT	4	4	0.04	30	20	1.06	--
19.9	11.6	0.1	0.61	SANDY SILT AND SILT	4	4	0.05	30	20	1.06	--
20.1	10.2	0.1	1.27	SANDY SILT AND SILT	4	4	0.03	29	20	1.07	--
20.3	9.4	0.2	2.13	SANDY SILT AND SILT	4	4	0.01	28	20	1.07	--
20.4	9.6	0.2	2.18	SANDY SILT AND SILT	4	4	0.01	29	20	1.08	--
20.6	9.9	0.2	2.23	SANDY SILT AND SILT	4	4	0.01	29	20	1.08	--
20.8	9.3	0.2	2.58	CLAYEY SILT AND SILTY CLAY	4	4	---	---	---	1.09	0.50
20.9	10.3	0.2	1.65	SANDY SILT AND SILT	4	4	0.02	29	20	1.09	--
21.1	11.6	0.1	0.78	SANDY SILT AND SILT	4	4	0.04	29	20	1.10	--
21.3	11.6	0.2	1.55	SANDY SILT AND SILT	4	4	0.03	29	20	1.10	--
21.4	14.3	0.2	1.33	SANDY SILT AND SILT	5	5	0.04	30	20	1.11	--
21.6	27.2	0.2	0.62	SILTY SAND	8	7	0.08	34	20	1.11	--
21.8	27.8	0.2	0.72	SILTY SAND	8	7	0.08	34	20	1.12	--
21.9	21.2	0.2	1.04	SILTY SAND	6	6	0.06	32	20	1.12	--
22.1	12.8	0.2	1.87	SANDY SILT AND SILT	5	5	0.03	30	20	1.13	--
22.2	13.5	0.1	1.04	SANDY SILT AND SILT	4	4	0.04	30	20	1.13	--
22.4	10.5	0.1	0.95	SANDY SILT AND SILT	4	3	0.04	29	20	1.14	--
22.6	8.9	0.1	0.90	SANDY SILT AND SILT	3	3	0.03	28	20	1.14	--
22.7	9.4	0.1	1.38	SANDY SILT AND SILT	4	3	0.03	28	20	1.15	--
22.9	10.2	0.2	1.76	SANDY SILT AND SILT	4	4	0.02	29	20	1.15	--
23.1	10.4	0.2	1.74	SANDY SILT AND SILT	4	4	0.02	29	20	1.16	--
23.2	9.6	0.1	1.26	SANDY SILT AND SILT	3	3	0.03	28	20	1.16	--
23.4	9.5	0.1	0.95	SANDY SILT AND SILT	3	3	0.04	28	20	1.17	--
23.6	8.8	0.1	0.79	SANDY SILT AND SILT	3	3	0.04	28	20	1.17	--
23.7	6.8	0.1	0.73	SANDY SILT AND SILT	2	2	0.04	26	20	1.18	--
23.9	7.5	0.1	0.80	SANDY SILT AND SILT	3	2	0.03	27	20	1.18	--
24.0	7.6	0.1	0.66	SANDY SILT AND SILT	3	2	0.04	27	20	1.19	--
24.2	12.2	0.1	0.41	SILTY SAND	4	3	0.06	29	20	1.19	--
24.4	13.6	0.1	0.37	SILTY SAND	4	4	0.07	30	20	1.20	--
24.5	16.2	0.1	0.31	SILTY SAND	4	4	0.08	31	20	1.20	--
24.7	17.0	0.1	0.29	SILTY SAND	5	4	0.09	31	20	1.21	--
24.9	10.2	0.1	0.49	SANDY SILT AND SILT	3	3	0.05	28	20	1.22	--
25.0	9.5	0.1	0.53	SANDY SILT AND SILT	3	3	0.04	28	20	1.22	--
25.2	8.2	0.1	0.61	SANDY SILT AND SILT	3	3	0.04	27	20	1.23	--
25.4	8.2	0.1	0.61	SANDY SILT AND SILT	3	2	0.04	27	20	1.23	--
25.5	8.2	0.0	0.37	SANDY SILT AND SILT	3	2	0.04	27	20	1.24	--
25.7	12.9	0.0	0.23	SILTY SAND	4	3	0.08	29	20	1.24	--
25.9	9.4	0.0	0.43	SANDY SILT AND SILT	3	3	0.04	28	20	1.25	--
26.0	12.1	0.0	0.25	SILTY SAND	3	3	0.08	29	20	1.25	--
26.2	12.9	0.0	0.31	SILTY SAND	4	3	0.07	29	20	1.26	--
26.3	8.1	0.0	0.49	SANDY SILT AND SILT	3	2	0.04	27	20	1.26	--
26.5	9.1	0.0	0.44	SANDY SILT AND SILT	3	3	0.04	28	20	1.27	--
26.7	10.9	0.0	0.28	SILTY SAND	3	3	0.06	28	20	1.27	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 17

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
26.8	10.2	0.0	0.29	SANDY SILT AND SILT	3	3	0.05	28	20	1.28	--
27.0	11.1	0.0	0.27	SILTY SAND	3	3	0.06	28	20	1.28	--
27.2	9.4	0.0	0.42	SANDY SILT AND SILT	3	3	0.04	28	20	1.29	--
27.3	10.2	0.0	0.39	SANDY SILT AND SILT	3	3	0.05	28	20	1.29	--
27.5	9.8	0.0	0.41	SANDY SILT AND SILT	3	3	0.05	28	20	1.30	--
27.7	10.2	0.0	0.39	SANDY SILT AND SILT	3	3	0.05	28	20	1.30	--
27.8	11.7	0.0	0.26	SILTY SAND	3	3	0.07	29	20	1.31	--
28.0	10.7	0.0	0.28	SILTY SAND	3	3	0.05	28	20	1.31	--
28.1	11.4	0.0	0.35	SILTY SAND	3	3	0.06	28	20	1.32	--
28.3	10.5	0.0	0.29	SANDY SILT AND SILT	3	3	0.05	28	20	1.32	--
28.5	12.0	0.0	0.25	SILTY SAND	3	3	0.07	29	20	1.33	--
28.6	10.1	0.0	0.30	SANDY SILT AND SILT	3	3	0.05	28	20	1.33	--
28.8	10.2	0.0	0.20	SILTY SAND	3	3	0.06	28	20	1.34	--
29.0	10.3	0.0	0.29	SANDY SILT AND SILT	3	3	0.05	28	20	1.34	--
29.1	12.7	0.0	0.24	SILTY SAND	4	3	0.08	29	20	1.35	--
29.3	8.4	0.0	0.36	SANDY SILT AND SILT	3	2	0.04	27	20	1.35	--
29.5	10.8	0.0	0.18	SILTY SAND	3	3	0.07	28	20	1.36	--
29.6	10.9	0.0	0.27	SILTY SAND	3	3	0.06	28	20	1.36	--
29.8	10.2	0.0	0.30	SANDY SILT AND SILT	3	3	0.05	28	20	1.37	--
30.0	12.8	0.0	0.23	SILTY SAND	4	3	0.08	29	20	1.37	--
30.1	13.6	0.0	0.22	SILTY SAND	4	3	0.08	29	20	1.38	--
30.3	8.9	0.0	0.34	SANDY SILT AND SILT	3	2	0.04	27	20	1.38	--
30.4	8.8	0.0	0.34	SANDY SILT AND SILT	3	2	0.04	27	20	1.39	--
30.6	8.8	0.0	0.34	SANDY SILT AND SILT	3	2	0.04	27	20	1.39	--
30.8	8.8	0.0	0.34	SANDY SILT AND SILT	3	2	0.04	27	20	1.40	--
30.9	8.9	0.0	0.45	SANDY SILT AND SILT	3	2	0.04	27	20	1.41	--
31.1	9.5	0.0	0.42	SANDY SILT AND SILT	3	3	0.04	27	20	1.41	--
31.3	9.5	0.0	0.32	SANDY SILT AND SILT	3	3	0.05	27	20	1.42	--
31.4	9.5	0.0	0.32	SANDY SILT AND SILT	3	3	0.05	27	20	1.42	--
31.6	10.2	0.1	0.88	SANDY SILT AND SILT	3	3	0.04	27	20	1.43	--
31.8	14.3	0.1	0.91	SANDY SILT AND SILT	4	4	0.05	29	20	1.43	--
31.9	20.4	0.1	0.25	SILTY SAND	6	5	0.09	31	20	1.44	--
32.1	17.7	0.0	0.17	SILTY SAND	5	4	0.09	30	20	1.44	--
32.3	10.2	0.0	0.29	SANDY SILT AND SILT	3	3	0.05	27	20	1.45	--
32.4	8.2	0.0	0.37	SANDY SILT AND SILT	3	2	0.04	26	20	1.45	--
32.6	7.5	0.0	0.40	SANDY SILT AND SILT	2	2	0.04	26	20	1.46	--
32.7	8.4	0.0	0.24	SANDY SILT AND SILT	3	2	0.05	26	20	1.46	--
32.9	9.6	0.0	0.10	SILTY SAND	3	2	0.06	27	20	1.47	--
33.1	16.9	0.0	0.06	SILTY SAND	4	4	0.10	30	20	1.47	--
33.2	34.7	0.1	0.17	SAND	6	5	0.84	33	20	1.48	--
33.4	58.8	0.2	0.34	SAND	9	7	1.34	36	30	1.48	--
33.6	72.8	0.3	0.38	SAND	11	9	1.39	37	37	1.49	--
33.7	91.0	0.5	0.54	SAND	14	12	1.28	38	44	1.49	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 17

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.
33.9	106.8	0.7	0.68	SAND	17 14	1.17	39	50	1.50	--
34.1	121.0	0.9	0.73	SAND	20 16	1.17	39	55	1.50	--
34.2	145.6	1.2	0.82	SAND	24 19	1.16	40	63	1.51	--
34.4	170.6	1.6	0.94	SAND	28 23	1.10	41	70	1.51	--
34.5	173.2	1.9	1.11	SAND	31 25	0.94	41	70	1.52	--
34.7	171.9	2.0	1.18	SAND	31 25	0.88	41	70	1.52	--
34.9	171.4	2.0	1.15	SAND	31 25	0.90	41	69	1.53	--
35.0	172.0	2.0	1.14	SAND	31 25	0.91	41	70	1.53	--
35.2	171.4	1.9	1.11	SAND	30 25	0.94	41	69	1.54	--
35.4	166.6	1.9	1.15	SAND	30 24	0.89	41	68	1.54	--
35.5	155.1	1.7	1.08	SAND	28 22	0.92	40	65	1.55	--
35.7	159.5	1.5	0.93	SAND	27 22	1.08	41	66	1.55	--
35.9	155.7	1.6	1.03	SAND	27 22	0.97	40	65	1.56	--
36.0	153.9	1.6	1.06	SAND	27 22	0.94	40	64	1.56	--
36.2	152.3	1.6	1.05	SAND	27 22	0.94	40	64	1.57	--
36.4	146.0	1.5	1.01	SAND	26 21	0.95	40	62	1.57	--
36.5	146.9	1.4	0.92	SAND	25 20	1.06	40	62	1.58	--
36.7	138.7	1.3	0.94	SAND	24 19	1.01	40	60	1.58	--
36.8	104.7	1.2	1.18	SAND	22 18	0.54	38	48	1.59	--
37.0	57.8	1.3	2.32	SILTY SAND	17 14	0.05	35	27	1.60	--
37.2	26.3	1.1	3.99	CLAYEY SILT AND SILTY CLAY	13 10	---	---	---	1.60	1.50
37.3	17.0	0.3	1.76	SANDY SILT AND SILT	6 5	0.04	29	20	1.61	--
37.5	13.6	0.1	1.03	SANDY SILT AND SILT	4 4	0.04	28	20	1.61	--
37.7	12.9	0.1	0.54	SILTY SAND	4 3	0.06	28	20	1.62	--
37.8	13.7	0.1	0.51	SILTY SAND	4 3	0.06	28	20	1.62	--
38.0	12.9	0.1	0.85	SANDY SILT AND SILT	4 3	0.05	28	20	1.63	--
38.2	12.2	0.1	0.82	SANDY SILT AND SILT	4 3	0.04	28	20	1.63	--
38.3	12.2	0.1	0.74	SANDY SILT AND SILT	4 3	0.05	28	20	1.64	--
38.5	11.4	0.1	0.70	SANDY SILT AND SILT	4 3	0.04	27	20	1.64	--
38.6	11.6	0.1	0.61	SANDY SILT AND SILT	4 3	0.05	27	20	1.65	--
38.8	11.6	0.1	0.69	SANDY SILT AND SILT	4 3	0.05	27	20	1.65	--
39.0	11.6	0.0	0.35	SILTY SAND	3 3	0.06	27	20	1.66	--
39.1	12.2	0.1	0.41	SILTY SAND	4 3	0.06	28	20	1.66	--
39.3	11.6	0.1	0.43	SILTY SAND	4 3	0.05	27	20	1.67	--
39.5	10.2	0.1	0.49	SANDY SILT AND SILT	3 3	0.05	27	20	1.67	--
39.6	10.9	0.1	0.46	SANDY SILT AND SILT	3 3	0.05	27	20	1.68	--
39.8	11.6	0.1	0.43	SANDY SILT AND SILT	4 3	0.05	27	20	1.68	--
40.0	11.6	0.1	0.61	SANDY SILT AND SILT	4 3	0.05	27	20	1.69	--
40.1	11.6	0.1	0.69	SANDY SILT AND SILT	4 3	0.05	27	20	1.69	--
40.3	11.6	0.1	0.69	SANDY SILT AND SILT	4 3	0.05	27	20	1.70	--
40.5	11.6	0.1	0.60	SANDY SILT AND SILT	4 3	0.05	27	20	1.70	--
40.6	11.0	0.1	0.46	SANDY SILT AND SILT	3 3	0.05	27	20	1.71	--
40.8	11.0	0.1	0.45	SANDY SILT AND SILT	3 3	0.05	27	20	1.71	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 17

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	FO TSF	SU TSF.	
40.9	11.0	0.1	0.45	SANDY SILT AND SILT	3	3	0.05	27	20	1.72	--
41.1	10.9	0.1	0.46	SANDY SILT AND SILT	3	3	0.05	27	20	1.72	--
41.3	11.6	0.1	0.69	SANDY SILT AND SILT	4	3	0.05	27	20	1.73	--
41.4	11.6	0.1	1.04	SANDY SILT AND SILT	4	3	0.04	27	20	1.73	--
41.6	11.7	0.1	1.20	SANDY SILT AND SILT	4	3	0.04	27	20	1.74	--
41.8	12.3	0.2	1.22	SANDY SILT AND SILT	4	3	0.04	27	20	1.74	--
41.9	12.9	0.1	0.93	SANDY SILT AND SILT	4	3	0.04	28	20	1.75	--
42.1	12.2	0.1	0.65	SANDY SILT AND SILT	4	3	0.05	27	20	1.75	--
42.3	11.5	0.1	0.44	SANDY SILT AND SILT	4	3	0.05	27	20	1.76	--
42.4	11.6	0.1	0.69	SANDY SILT AND SILT	4	3	0.05	27	20	1.76	--
42.6	14.5	0.1	0.35	SILTY SAND	4	3	0.08	28	20	1.77	--
42.7	15.0	0.2	1.00	SANDY SILT AND SILT	5	4	0.05	28	20	1.77	--
42.9	16.3	0.3	1.59	SANDY SILT AND SILT	6	4	0.04	29	20	1.78	--
43.1	22.7	0.2	1.06	SILTY SAND	7	5	0.06	30	20	1.79	--
43.2	32.0	0.2	0.56	SILTY SAND	9	7	0.09	32	20	1.79	--
43.4	46.2	0.2	0.52	SAND	10	7	0.55	34	20	1.80	--
43.6	65.3	0.3	0.47	SAND	11	8	1.17	35	28	1.80	--
43.7	98.9	0.5	0.49	SAND	15	11	1.38	37	43	1.81	--
43.9	132.8	0.8	0.56	SAND	20	15	1.39	39	54	1.81	--
44.1	161.2	1.1	0.65	SAND	24	18	1.36	40	62	1.82	--
44.2	178.0	1.4	0.78	SAND	28	21	1.27	40	66	1.82	--
44.4	188.4	1.7	0.90	SAND	30	22	1.18	41	69	1.83	--
44.6	191.8	1.9	1.01	SAND	32	24	1.09	41	70	1.83	--
44.7	191.0	2.0	1.06	SAND	33	24	1.03	41	69	1.84	--
44.9	191.1	2.1	1.07	SAND	33	24	1.02	41	69	1.84	--
45.0	189.3	2.1	1.10	SAND	33	24	0.99	41	69	1.85	--
45.2	195.0	2.1	1.10	SAND	34	25	1.01	41	70	1.85	--
45.4	197.2	2.2	1.11	SAND	34	25	1.01	41	70	1.86	--
45.5	188.1	0.0	0.02	SAND	24	17	1.98	40	68	1.86	--
45.7	187.8	1.9	1.03	SAND	32	23	1.06	40	68	1.87	--
45.9	60.5	2.0	3.27	SANDY SILT AND SILT	22	16	0.03	35	24	1.87	--
46.0	171.5	1.8	1.06	SAND	30	22	0.99	40	64	1.88	--
46.2	161.4	1.6	1.00	SAND	28	20	1.01	40	62	1.88	--
46.4	140.3	1.4	0.97	SAND	24	18	0.98	39	55	1.89	--
46.5	117.6	1.2	0.98	SAND	21	15	0.87	38	48	1.89	--
46.7	98.6	1.0	1.04	SAND	20	14	0.67	37	41	1.90	--
46.8	76.8	0.8	1.02	SAND	17	12	0.46	36	32	1.90	--
47.0	56.4	0.3	0.57	SAND	10	7	0.83	34	21	1.91	--
47.2	33.6	0.1	0.21	SAND	8	6	0.32	32	20	1.91	--
47.3	19.0	0.1	0.42	SILTY SAND	5	4	0.08	29	20	1.92	--
47.5	14.6	0.0	0.21	SILTY SAND	4	3	0.09	28	20	1.92	--
47.7	14.3	0.0	0.21	SILTY SAND	4	3	0.09	28	20	1.93	--
47.8	14.3	0.1	0.63	SILTY SAND	4	3	0.06	28	20	1.93	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 17

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	P0 TSF	SU TSF.	
48.0	18.5	0.1	0.60	SILTY SAND	5	4	0.07	29	20	1.94	--
48.2	26.5	0.1	0.53	SILTY SAND	7	5	0.08	31	20	1.94	--
48.3	21.2	0.1	0.57	SILTY SAND	6	4	0.08	30	20	1.95	--
48.5	21.1	0.3	1.19	SILTY SAND	6	4	0.06	29	20	1.95	--
48.7	19.0	0.4	1.94	SANDY SILT AND SILT	7	5	0.03	29	20	1.96	--
48.8	19.0	0.5	2.63	SANDY SILT AND SILT	8	5	0.02	29	20	1.96	--
49.0	17.7	0.5	2.77	SANDY SILT AND SILT	7	5	0.02	29	20	1.97	--
49.1	17.7	0.5	2.55	SANDY SILT AND SILT	7	5	0.02	29	20	1.98	--
49.3	17.7	0.4	2.21	SANDY SILT AND SILT	7	5	0.03	29	20	1.98	--
49.5	17.4	0.4	2.18	SANDY SILT AND SILT	7	5	0.03	28	20	1.99	--
49.6	16.3	0.3	2.02	SANDY SILT AND SILT	6	4	0.03	28	20	1.99	--
49.8	13.6	0.3	1.91	SANDY SILT AND SILT	5	4	0.03	27	20	2.00	--
50.0	13.1	0.2	1.68	SANDY SILT AND SILT	5	3	0.03	27	20	2.00	--
50.1	15.6	0.3	1.74	SANDY SILT AND SILT	6	4	0.03	28	20	2.01	--
50.3	21.6	0.4	1.62	SANDY SILT AND SILT	7	5	0.05	29	20	2.01	--
50.5	23.1	0.5	2.25	SANDY SILT AND SILT	8	6	0.03	30	20	2.02	--
50.6	23.1	0.7	2.90	SANDY SILT AND SILT	9	7	0.02	30	20	2.02	--
50.8	25.2	0.8	3.22	SANDY SILT AND SILT	11	8	0.01	30	20	2.03	--
51.0	25.2	1.0	3.86	CLAYEY SILT AND SILTY CLAY	12	8	---	---	---	2.03	1.37
51.1	26.4	1.0	3.83	CLAYEY SILT AND SILTY CLAY	13	9	---	---	---	2.04	1.45
51.3	29.2	1.0	3.52	CLAYEY SILT AND SILTY CLAY	13	9	---	---	---	2.04	1.63
51.4	30.8	1.0	3.25	SANDY SILT AND SILT	13	9	0.02	31	20	2.05	--
51.6	33.3	1.1	3.15	SANDY SILT AND SILT	13	9	0.02	31	20	2.05	--
51.8	33.3	1.1	3.27	SANDY SILT AND SILT	14	9	0.02	31	20	2.06	--
51.9	34.2	0.8	2.40	SANDY SILT AND SILT	11	8	0.04	32	20	2.06	--
52.1	32.0	0.5	1.66	SILTY SAND	10	7	0.06	31	20	2.07	--
52.3	27.9	0.4	1.43	SILTY SAND	8	6	0.06	31	20	2.07	--
52.4	25.8	0.4	1.39	SILTY SAND	8	5	0.06	30	20	2.08	--
52.6	24.4	0.4	1.76	SANDY SILT AND SILT	8	5	0.05	30	20	2.08	--
52.8	22.3	0.3	1.44	SILTY SAND	7	5	0.05	29	20	2.09	--
52.9	19.0	0.2	1.05	SILTY SAND	6	4	0.06	29	20	2.09	--
53.1	16.3	0.1	0.86	SILTY SAND	5	3	0.06	28	20	2.10	--
53.2	16.0	0.1	0.56	SILTY SAND	5	3	0.07	28	20	2.10	--
53.4	15.1	0.2	1.19	SANDY SILT AND SILT	5	3	0.04	27	20	2.11	--
53.6	16.3	0.3	1.84	SANDY SILT AND SILT	6	4	0.03	28	20	2.11	--
53.7	17.7	0.3	1.75	SANDY SILT AND SILT	6	4	0.04	28	20	2.12	--
53.9	19.0	0.2	0.95	SILTY SAND	6	4	0.06	29	20	2.12	--
54.1	19.0	0.2	1.10	SILTY SAND	6	4	0.05	29	20	2.13	--
54.2	18.9	0.4	2.22	SANDY SILT AND SILT	7	5	0.03	29	20	2.13	--
54.4	20.4	0.6	3.04	SANDY SILT AND SILT	9	6	0.01	29	20	2.14	--
54.6	25.8	0.9	3.64	CLAYEY SILT AND SILTY CLAY	12	8	---	---	---	2.14	1.40
54.7	36.0	1.0	2.86	SANDY SILT AND SILT	13	9	0.03	32	20	2.15	--
54.9	53.0	1.0	1.79	SILTY SAND	15	10	0.07	34	20	2.15	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 17

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
55.1	80.1	0.5	0.64	SAND	13	9	1.07	36	31	2.16	--
55.2	75.0	0.9	1.25	SILTY SAND	20	13	0.10	35	28	2.17	--
55.4	72.1	1.4	1.90	SILTY SAND	20	14	0.08	35	27	2.17	--
55.5	92.5	0.7	0.78	SAND	16	11	0.97	36	36	2.18	--
55.7	115.1	0.4	0.31	SAND	16	11	1.63	37	43	2.18	--
55.9	116.3	0.7	0.58	SAND	18	12	1.32	37	44	2.19	--
56.0	121.0	0.9	0.76	SAND	20	13	1.14	38	45	2.19	--
56.2	129.9	0.5	0.35	SAND	18	12	1.61	38	48	2.20	--
56.4	90.4	1.0	1.14	SAND	20	13	0.46	36	34	2.20	--
56.5	45.6	0.8	1.65	SILTY SAND	13	9	0.07	33	20	2.21	--
56.7	34.7	0.5	1.41	SILTY SAND	10	7	0.07	31	20	2.21	--
56.9	27.9	0.7	2.33	SANDY SILT AND SILT	10	6	0.04	30	20	2.22	--
57.0	25.8	0.8	2.90	SANDY SILT AND SILT	10	7	0.02	30	20	2.22	--
57.2	25.8	0.7	2.52	SANDY SILT AND SILT	9	6	0.03	30	20	2.23	--
57.3	28.7	0.8	2.86	SANDY SILT AND SILT	11	7	0.02	30	20	2.23	--
57.5	31.3	1.1	3.52	CLAYEY SILT AND SILTY CLAY	14	9	---	---	---	2.24	1.73
57.7	43.7	1.2	2.68	SANDY SILT AND SILT	15	10	0.04	32	20	2.24	--
57.8	55.8	1.2	2.15	SILTY SAND	17	11	0.06	34	20	2.25	--
58.0	46.5	1.3	2.82	SANDY SILT AND SILT	16	10	0.04	33	20	2.25	--
58.2	38.8	1.0	2.50	SANDY SILT AND SILT	13	8	0.04	32	20	2.26	--
58.3	32.6	1.0	3.01	SANDY SILT AND SILT	13	8	0.02	31	20	2.26	--
58.5	29.9	0.9	2.94	SANDY SILT AND SILT	12	7	0.02	30	20	2.27	--
58.7	34.0	0.6	1.76	SILTY SAND	10	7	0.06	31	20	2.27	--
58.8	31.3	0.8	2.46	SANDY SILT AND SILT	11	7	0.04	31	20	2.28	--
59.0	23.0	0.7	2.92	SANDY SILT AND SILT	9	6	0.02	29	20	2.28	--
59.2	21.8	0.6	2.57	SANDY SILT AND SILT	8	5	0.02	29	20	2.29	--
59.3	19.7	0.5	2.48	SANDY SILT AND SILT	8	5	0.02	28	20	2.29	--
59.5	19.8	0.3	1.67	SANDY SILT AND SILT	6	4	0.04	28	20	2.30	--
59.6	19.7	0.3	1.42	SANDY SILT AND SILT	6	4	0.05	28	20	2.30	--
59.8	20.4	0.4	2.01	SANDY SILT AND SILT	7	5	0.04	29	20	2.31	--
60.0	21.0	0.5	2.33	SANDY SILT AND SILT	8	5	0.03	29	20	2.31	--
60.1	19.8	0.5	2.28	SANDY SILT AND SILT	7	5	0.03	28	20	2.32	--
60.3	19.0	0.4	1.84	SANDY SILT AND SILT	7	4	0.04	28	20	2.32	--
60.5	19.0	0.3	1.47	SANDY SILT AND SILT	6	4	0.04	28	20	2.33	--
60.6	17.7	0.3	1.87	SANDY SILT AND SILT	6	4	0.03	28	20	2.33	--
60.8	17.7	0.4	1.98	SANDY SILT AND SILT	6	4	0.03	28	20	2.34	--
61.0	18.5	0.4	1.94	SANDY SILT AND SILT	7	4	0.03	28	20	2.34	--
61.1	19.0	0.4	1.89	SANDY SILT AND SILT	7	4	0.04	28	20	2.35	--
61.3	18.2	0.3	1.65	SANDY SILT AND SILT	6	4	0.04	28	20	2.36	--
61.4	19.0	0.5	2.36	SANDY SILT AND SILT	7	5	0.03	28	20	2.36	--
61.6	46.9	0.3	0.62	SAND	11	7	0.31	32	20	2.37	--
61.7	96.2	0.4	0.42	SAND	14	9	1.46	36	35	2.37	--
61.9	93.8	0.8	0.88	SAND	17	11	0.83	36	34	2.37	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 17

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.
62.1	105.0	1.2	1.13	SAND	21 13	0.61	36	38	2.38	--
62.2	165.8	1.1	0.68	SAND	25 16	1.34	39	56	2.38	--
62.4	213.0	1.1	0.51	SAND	30 19	1.59	40	67	2.39	--
62.6	242.1	1.3	0.53	SAND	34 21	1.65	40	72	2.40	--
62.7	275.4	1.5	0.53	SAND	37 23	1.73	41	78	2.40	--
62.9	274.7	1.6	0.57	SAND	38 23	1.69	41	78	2.41	--
63.1	244.1	1.5	0.62	SAND	35 21	1.57	40	73	2.41	--
63.2	200.7	1.4	0.70	SAND	30 19	1.38	40	64	2.42	--
63.4	147.6	1.2	0.83	SAND	24 15	1.15	38	50	2.42	--
63.5	105.4	1.2	1.16	SAND	22 13	0.58	36	38	2.43	--
63.7	94.5	1.5	1.58	SILTY SAND	25 15	0.10	36	34	2.43	--
63.9	134.6	1.2	0.91	SAND	23 14	1.02	38	46	2.44	--
64.0	142.8	1.0	0.73	SAND	23 14	1.24	38	49	2.44	--
64.2	144.0	1.5	1.02	SAND	25 16	0.94	38	49	2.45	--
64.4	108.5	1.3	1.18	SAND	22 14	0.58	36	39	2.45	--
64.5	135.8	1.0	0.77	SAND	22 13	1.18	38	47	2.46	--
64.7	175.4	0.7	0.39	SAND	24 15	1.63	39	57	2.46	--
64.9	159.7	0.7	0.41	SAND	22 14	1.60	38	53	2.47	--
65.0	155.6	0.9	0.55	SAND	23 14	1.45	38	52	2.47	--
65.2	167.2	0.8	0.50	SAND	24 15	1.51	38	55	2.48	--
65.4	190.6	0.7	0.34	SAND	26 16	1.69	39	61	2.48	--
65.5	181.0	0.8	0.44	SAND	26 15	1.59	39	59	2.49	--
65.7	171.6	0.9	0.51	SAND	25 15	1.52	39	56	2.49	--
65.8	152.5	0.7	0.49	SAND	22 13	1.51	38	51	2.50	--
66.0	155.1	0.7	0.43	SAND	22 14	1.57	38	51	2.50	--
66.2	167.3	0.6	0.35	SAND	23 15	1.67	38	55	2.51	--
66.3	163.9	0.6	0.38	SAND	23 15	1.63	38	54	2.51	--
66.5	159.1	0.7	0.45	SAND	23 14	1.55	38	52	2.52	--
66.7	177.2	0.6	0.34	SAND	24 16	1.68	39	57	2.52	--
66.8	208.1	0.6	0.30	SAND	28 18	1.76	39	64	2.53	--
67.0	228.5	0.7	0.32	SAND	30 19	1.80	40	68	2.53	--
67.2	239.4	0.9	0.37	SAND	32 20	1.79	40	70	2.54	--
67.3	227.1	1.0	0.43	SAND	31 20	1.70	40	68	2.54	--
67.5	200.6	1.0	0.49	SAND	28 18	1.57	39	62	2.55	--
67.7	172.0	0.9	0.53	SAND	25 16	1.50	38	55	2.55	--
67.8	152.1	0.7	0.48	SAND	22 14	1.52	38	50	2.56	--
68.0	143.5	0.4	0.26	SAND	19 12	1.72	38	48	2.56	--
68.1	133.4	0.4	0.26	SAND	18 11	1.72	37	45	2.57	--
68.3	115.5	0.3	0.27	SAND	16 10	1.69	36	40	2.57	--
68.5	116.3	0.5	0.46	SAND	17 11	1.47	37	40	2.58	--
68.6	120.4	0.0	0.01	SAND	15 9	1.99	37	41	2.59	--
68.8	125.6	0.4	0.33	SAND	17 11	1.63	37	43	2.59	--
69.0	44.3	0.6	1.26	SILTY SAND	12 8	0.08	32	20	2.60	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 17

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	P0 TSF	SU TSF.
69.1	77.5	0.8	1.04	SAND	17 11	0.43	34	25	2.60	--
69.3	56.9	0.8	1.42	SILTY SAND	16 10	0.08	33	20	2.61	--
69.5	63.6	0.8	1.20	SILTY SAND	17 11	0.10	33	20	2.61	--
69.6	100.0	1.2	1.18	SAND	21 13	0.50	36	34	2.62	--
69.8	86.8	1.0	1.09	SAND	19 12	0.49	35	29	2.62	--
69.9	130.4	1.1	0.81	SAND	22 13	1.12	37	44	2.63	--
70.1	117.0	1.3	1.14	SAND	23 14	0.69	36	40	2.63	--
70.3	68.0	1.7	2.52	SILTY SAND	21 13	0.05	34	20	2.64	--
70.4	35.7	0.9	2.61	SANDY SILT AND SILT	12 8	0.04	31	20	2.64	--
70.6	24.5	0.4	1.51	SILTY SAND	7 5	0.05	29	20	2.65	--
70.8	22.4	0.2	0.98	SILTY SAND	7 4	0.07	28	20	2.65	--
70.9	22.4	0.3	1.25	SILTY SAND	7 4	0.06	28	20	2.66	--
71.1	21.7	0.3	1.38	SILTY SAND	7 4	0.05	28	20	2.66	--
71.3	21.1	0.3	1.52	SANDY SILT AND SILT	7 4	0.05	28	20	2.67	--
71.4	19.8	0.3	1.42	SANDY SILT AND SILT	6 4	0.05	28	20	2.67	--
71.6	17.7	0.3	1.64	SANDY SILT AND SILT	6 4	0.04	27	20	2.68	--

INTERPRETED CONE PENETRATION TEST DATA

C. P.T. - 18

CLIENT: HOWARD HUGHES PROPERTIES
 JOB NO: AE-88473
 DATE: JANUARY 30, 1989
 ELEVATION: 12.7 FT
 UNIT WEIGHT OF SOIL: 125 PCF
 DEPTH TO GROUND WATER: 12.0 FT

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF
0.1	9.5	0.0	0.11	SILTY SAND	3	11	0.05	--	0.01	--
0.3	15.0	0.0	0.07	SILTY SAND	4	13	0.10	--	0.02	--
0.4	13.4	0.0	0.07	SILTY SAND	4	11	0.09	--	0.03	--
0.6	20.4	0.0	0.10	SILTY SAND	5	15	0.10	--	0.04	--
0.8	27.9	0.0	0.07	SILTY SAND	7	20	0.10	--	0.05	--
0.9	26.5	0.0	0.08	SILTY SAND	7	18	0.10	--	0.06	--
1.1	23.1	0.0	0.09	SILTY SAND	6	15	0.10	--	0.07	--
1.2	17.8	0.0	0.11	SILTY SAND	5	11	0.09	--	0.08	--
1.4	13.1	0.0	0.15	SILTY SAND	4	8	0.09	--	0.09	--
1.6	7.1	0.0	0.14	SANDY SILT AND SILT	2	5	0.05	--	0.10	--
1.7	9.5	0.0	0.11	SILTY SAND	3	6	0.05	--	0.11	--
1.9	6.6	0.0	0.15	SANDY SILT AND SILT	2	4	0.05	--	0.12	--
2.1	3.9	0.0	0.26	SANDY SILT AND SILT	1	3	0.04	--	0.13	--
2.2	3.4	0.0	0.29	SANDY SILT AND SILT	1	2	0.04	--	0.14	--
2.4	4.1	0.0	0.24	SANDY SILT AND SILT	1	3	0.04	--	0.15	--
2.6	4.1	0.0	0.25	SANDY SILT AND SILT	1	3	0.04	--	0.16	--
2.7	4.1	0.0	0.25	SANDY SILT AND SILT	1	3	0.04	--	0.17	--
2.9	6.1	0.0	0.16	SANDY SILT AND SILT	2	4	0.05	--	0.18	--
3.1	11.5	0.0	0.09	SILTY SAND	3	6	0.09	--	0.19	--
3.2	19.7	0.0	0.05	SILTY SAND	5	10	0.10	--	0.20	--
3.4	23.2	0.0	0.04	SILTY SAND	5	11	0.10	--	0.21	--
3.5	19.7	0.0	0.05	SILTY SAND	5	9	0.10	--	0.22	--
3.7	19.0	0.0	0.05	SILTY SAND	5	9	0.10	--	0.23	--
3.9	14.3	0.0	0.07	SILTY SAND	4	7	0.10	--	0.24	--
4.0	10.2	0.0	0.10	SILTY SAND	3	5	0.08	36	24	0.25
4.2	8.2	0.0	0.12	SANDY SILT AND SILT	3	4	0.05	35	20	0.26
4.3	13.9	0.0	0.07	SILTY SAND	4	6	0.09	37	30	0.27
4.5	19.7	0.0	0.05	SILTY SAND	5	9	0.10	39	38	0.28
4.7	11.6	0.0	0.09	SILTY SAND	3	5	0.09	36	24	0.29
4.8	5.1	0.0	0.19	SANDY SILT AND SILT	2	3	0.04	32	20	0.30
5.0	7.0	0.0	0.14	SANDY SILT AND SILT	2	4	0.05	33	20	0.31
5.2	5.4	0.0	0.18	SANDY SILT AND SILT	2	3	0.05	32	20	0.32
5.3	2.7	0.0	0.37	SANDY SILT AND SILT	1	2	0.02	28	20	0.33
5.5	3.4	0.0	0.30	SANDY SILT AND SILT	1	2	0.04	29	20	0.34

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 18

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.
5.6	5.4	0.0	0.18	SANDY SILT AND SILT	2	3	0.05	31	20	0.35 --
5.8	4.8	0.0	0.21	SANDY SILT AND SILT	2	2	0.04	30	20	0.36 --
6.0	2.0	0.0	0.49	CLAYEY SILT AND SILTY CLAY	1	1	---	---	---	0.37 0.10
6.1	1.0	0.0	1.00	CLAY	1	2	---	---	---	0.38 0.04
6.3	1.1	0.0	0.90	CLAYEY SILT AND SILTY CLAY	1	1	---	---	---	0.39 0.04
6.5	1.4	0.0	0.74	CLAYEY SILT AND SILTY CLAY	1	1	---	---	---	0.40 0.06
6.6	1.4	0.0	0.74	CLAYEY SILT AND SILTY CLAY	1	1	---	---	---	0.41 0.06
6.8	1.4	0.0	0.71	CLAYEY SILT AND SILTY CLAY	1	1	---	---	---	0.42 0.06
7.0	1.5	0.0	0.69	CLAYEY SILT AND SILTY CLAY	1	1	---	---	---	0.43 0.06
7.1	1.5	0.0	0.69	CLAYEY SILT AND SILTY CLAY	1	1	---	---	---	0.44 0.06
7.3	1.4	0.0	0.71	CLAYEY SILT AND SILTY CLAY	1	1	---	---	---	0.45 0.06
7.4	1.4	0.0	0.74	CLAYEY SILT AND SILTY CLAY	1	1	---	---	---	0.47 0.06
7.6	1.3	0.0	0.78	CLAYEY SILT AND SILTY CLAY	1	1	---	---	---	0.48 0.05
7.8	1.2	0.0	0.84	CLAYEY SILT AND SILTY CLAY	1	1	---	---	---	0.49 0.04
7.9	1.2	0.0	0.87	CLAYEY SILT AND SILTY CLAY	1	1	---	---	---	0.50 0.04
8.1	1.1	0.0	0.90	CLAYEY SILT AND SILTY CLAY	1	1	---	---	---	0.51 0.04
8.3	1.2	0.0	0.84	CLAYEY SILT AND SILTY CLAY	1	1	---	---	---	0.52 0.04
8.4	3.7	0.0	0.27	SANDY SILT AND SILT	1	2	0.04	27	20	0.53 --
8.6	5.4	0.0	0.18	SANDY SILT AND SILT	2	2	0.05	29	20	0.54 --
8.8	5.1	0.0	0.20	SANDY SILT AND SILT	2	2	0.04	29	20	0.55 --
8.9	6.5	0.0	0.15	SANDY SILT AND SILT	2	3	0.05	30	20	0.56 --
9.1	6.6	0.0	0.15	SANDY SILT AND SILT	2	3	0.05	30	20	0.57 --
9.3	6.1	0.0	0.16	SANDY SILT AND SILT	2	3	0.05	29	20	0.58 --
9.4	4.8	0.0	0.21	SANDY SILT AND SILT	2	2	0.04	28	20	0.59 --
9.6	3.9	0.0	0.26	SANDY SILT AND SILT	1	2	0.04	27	20	0.60 --
9.7	3.5	0.0	0.29	SANDY SILT AND SILT	1	2	0.04	26	20	0.61 --
9.9	3.4	0.0	0.29	SANDY SILT AND SILT	1	1	0.04	26	20	0.62 --
10.1	3.4	0.0	0.29	SANDY SILT AND SILT	1	1	0.04	26	20	0.63 --
10.2	3.7	0.0	0.27	SANDY SILT AND SILT	1	2	0.04	26	20	0.64 --
10.4	4.0	0.0	0.25	SANDY SILT AND SILT	1	2	0.04	27	20	0.65 --
10.6	4.0	0.0	0.25	SANDY SILT AND SILT	1	2	0.04	27	20	0.66 --
10.7	3.4	0.0	0.29	SANDY SILT AND SILT	1	1	0.04	26	20	0.67 --
10.9	6.6	0.0	0.15	SANDY SILT AND SILT	2	3	0.05	29	20	0.68 --
11.1	10.2	0.0	0.10	SILTY SAND	3	3	0.08	31	20	0.69 --
11.2	8.2	0.0	0.12	SANDY SILT AND SILT	3	3	0.05	30	20	0.70 --
11.4	6.0	0.0	0.17	SANDY SILT AND SILT	2	2	0.05	28	20	0.71 --
11.5	4.1	0.0	0.25	SANDY SILT AND SILT	1	2	0.04	26	20	0.72 --
11.7	7.2	0.0	0.14	SANDY SILT AND SILT	2	3	0.05	29	20	0.73 --
11.9	27.2	0.0	0.15	SILTY SAND	7	8	0.10	35	25	0.74 --
12.0	33.9	0.0	0.09	SAND	5	6	1.32	37	31	0.75 --
12.2	34.7	0.0	0.03	SAND	5	5	1.81	37	32	0.76 --
12.4	29.2	0.0	0.03	SILTY SAND	8	9	0.10	36	27	0.76 --
12.5	23.1	0.0	0.04	SILTY SAND	6	7	0.10	35	20	0.77 --

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 18

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
12.7	17.9	0.0	0.06	SILTY SAND	5	5	0.10	33	20	0.77	--
12.9	18.5	0.0	0.05	SILTY SAND	5	6	0.10	33	20	0.78	--
13.0	18.4	0.0	0.05	SILTY SAND	5	6	0.10	33	20	0.78	--
13.2	15.5	0.0	0.06	SILTY SAND	4	5	0.10	32	20	0.79	--
13.4	12.1	0.0	0.08	SILTY SAND	3	4	0.09	31	20	0.79	--
13.5	7.5	0.0	0.13	SANDY SILT AND SILT	2	3	0.05	29	20	0.80	--
13.7	5.9	0.0	0.17	SANDY SILT AND SILT	2	2	0.05	28	20	0.80	--
13.8	4.1	0.0	0.24	SANDY SILT AND SILT	1	2	0.04	26	20	0.81	--
14.0	4.0	0.0	0.25	SANDY SILT AND SILT	1	1	0.04	26	20	0.81	--
14.2	4.1	0.0	0.25	SANDY SILT AND SILT	1	1	0.04	26	20	0.82	--
14.3	3.7	0.0	0.27	SANDY SILT AND SILT	1	1	0.04	25	20	0.82	--
14.5	3.4	0.0	0.29	SANDY SILT AND SILT	1	1	0.04	25	20	0.83	--
14.7	4.0	0.0	0.25	SANDY SILT AND SILT	1	1	0.04	26	20	0.83	--
14.8	4.6	0.0	0.22	SANDY SILT AND SILT	1	2	0.04	26	20	0.84	--
15.0	4.8	0.0	0.21	SANDY SILT AND SILT	2	2	0.04	26	20	0.84	--
15.2	7.7	0.0	0.13	SANDY SILT AND SILT	2	3	0.05	29	20	0.85	--
15.3	17.7	0.0	0.06	SILTY SAND	5	5	0.10	33	20	0.85	--
15.5	20.4	0.0	0.05	SILTY SAND	5	6	0.10	33	20	0.86	--
15.7	21.1	0.0	0.05	SILTY SAND	6	6	0.10	33	20	0.87	--
15.8	27.9	0.0	0.04	SILTY SAND	7	8	0.10	35	21	0.87	--
16.0	29.8	0.0	0.03	SAND	6	6	0.76	35	23	0.88	--
16.2	9.1	0.0	0.11	SANDY SILT AND SILT	3	3	0.05	29	20	0.88	--
16.3	18.4	0.0	0.05	SILTY SAND	5	5	0.10	33	20	0.89	--
16.5	12.2	0.0	0.08	SILTY SAND	3	4	0.09	31	20	0.89	--
16.7	8.8	0.0	0.11	SANDY SILT AND SILT	3	3	0.05	29	20	0.90	--
16.8	7.3	0.0	0.14	SANDY SILT AND SILT	2	2	0.05	28	20	0.90	--
17.0	5.6	0.0	0.18	SANDY SILT AND SILT	2	2	0.05	27	20	0.91	--
17.2	5.1	0.0	0.19	SANDY SILT AND SILT	2	2	0.04	26	20	0.91	--
17.3	4.8	0.0	0.21	SANDY SILT AND SILT	2	2	0.04	26	20	0.92	--
17.5	5.6	0.0	0.18	SANDY SILT AND SILT	2	2	0.05	27	20	0.92	--
17.7	6.1	0.0	0.16	SANDY SILT AND SILT	2	2	0.05	27	20	0.93	--
17.8	6.1	0.0	0.16	SANDY SILT AND SILT	2	2	0.05	27	20	0.93	--
18.0	5.6	0.0	0.18	SANDY SILT AND SILT	2	2	0.05	27	20	0.94	--
18.1	6.0	0.0	0.17	SANDY SILT AND SILT	2	2	0.05	27	20	0.94	--
18.3	6.3	0.0	0.16	SANDY SILT AND SILT	2	2	0.05	27	20	0.95	--
18.5	6.5	0.0	0.15	SANDY SILT AND SILT	2	2	0.05	27	20	0.95	--
18.6	6.1	0.0	0.16	SANDY SILT AND SILT	2	2	0.05	27	20	0.96	--
18.8	5.4	0.0	0.18	SANDY SILT AND SILT	2	2	0.05	26	20	0.96	--
19.0	5.4	0.0	0.18	SANDY SILT AND SILT	2	2	0.05	26	20	0.97	--
19.1	5.4	0.0	0.19	SANDY SILT AND SILT	2	2	0.05	26	20	0.97	--
19.3	4.8	0.0	0.21	SANDY SILT AND SILT	2	2	0.04	26	20	0.98	--
19.5	4.8	0.0	0.21	SANDY SILT AND SILT	2	2	0.04	26	20	0.98	--
19.6	4.8	0.0	0.21	SANDY SILT AND SILT	2	2	0.04	26	20	0.99	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 18

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
19.8	4.8	0.0	0.21	SANDY SILT AND SILT	2	2	0.04	26	20	0.99	--
19.9	4.8	0.0	0.21	SANDY SILT AND SILT	2	2	0.04	25	20	1.00	--
20.1	4.8	0.0	0.21	SANDY SILT AND SILT	2	2	0.04	25	20	1.00	--
20.3	5.4	0.0	0.18	SANDY SILT AND SILT	2	2	0.05	26	20	1.01	--
20.4	6.1	0.0	0.16	SANDY SILT AND SILT	2	2	0.05	27	20	1.01	--
20.6	6.0	0.0	0.17	SANDY SILT AND SILT	2	2	0.05	27	20	1.02	--
20.8	5.2	0.0	0.19	SANDY SILT AND SILT	2	2	0.05	26	20	1.02	--
20.9	5.4	0.0	0.18	SANDY SILT AND SILT	2	2	0.05	26	20	1.03	--
21.1	5.4	0.0	0.18	SANDY SILT AND SILT	2	2	0.05	26	20	1.03	--
21.3	5.2	0.0	0.19	SANDY SILT AND SILT	2	2	0.05	26	20	1.04	--
21.4	4.8	0.0	0.21	SANDY SILT AND SILT	2	2	0.04	25	20	1.04	--
21.6	5.0	0.0	0.20	SANDY SILT AND SILT	2	2	0.04	25	20	1.05	--
21.8	6.8	0.0	0.15	SANDY SILT AND SILT	2	2	0.05	27	20	1.06	--
21.9	7.6	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	27	20	1.06	--
22.1	6.6	0.0	0.15	SANDY SILT AND SILT	2	2	0.05	27	20	1.07	--
22.2	6.1	0.0	0.16	SANDY SILT AND SILT	2	2	0.05	26	20	1.07	--
22.4	5.4	0.0	0.18	SANDY SILT AND SILT	2	2	0.05	26	20	1.08	--
22.6	5.4	0.0	0.18	SANDY SILT AND SILT	2	2	0.05	26	20	1.08	--
22.7	5.4	0.0	0.18	SANDY SILT AND SILT	2	2	0.05	26	20	1.09	--
22.9	6.1	0.0	0.16	SANDY SILT AND SILT	2	2	0.05	26	20	1.09	--
23.1	5.4	0.0	0.18	SANDY SILT AND SILT	2	2	0.05	26	20	1.10	--
23.2	5.1	0.0	0.19	SANDY SILT AND SILT	2	2	0.04	25	20	1.10	--
23.4	4.8	0.0	0.21	SANDY SILT AND SILT	2	1	0.04	25	20	1.11	--
23.6	5.0	0.0	0.20	SANDY SILT AND SILT	2	2	0.04	25	20	1.11	--
23.7	5.4	0.0	0.18	SANDY SILT AND SILT	2	2	0.05	26	20	1.12	--
23.9	6.1	0.0	0.16	SANDY SILT AND SILT	2	2	0.05	26	20	1.12	--
24.0	6.1	0.0	0.16	SANDY SILT AND SILT	2	2	0.05	26	20	1.13	--
24.2	11.1	0.0	0.09	SILTY SAND	3	3	0.09	29	20	1.13	--
24.4	7.1	0.0	0.14	SANDY SILT AND SILT	2	2	0.05	27	20	1.14	--
24.5	6.6	0.0	0.15	SANDY SILT AND SILT	2	2	0.05	26	20	1.14	--
24.7	10.2	0.0	0.10	SILTY SAND	3	3	0.08	29	20	1.15	--
24.9	5.4	0.0	0.18	SANDY SILT AND SILT	2	2	0.05	25	20	1.15	--
25.0	6.1	0.0	0.16	SANDY SILT AND SILT	2	2	0.05	26	20	1.16	--
25.2	4.9	0.0	0.21	SANDY SILT AND SILT	2	1	0.04	25	20	1.16	--
25.4	4.8	0.0	0.21	SANDY SILT AND SILT	2	1	0.04	25	20	1.17	--
25.5	4.8	0.0	0.21	SANDY SILT AND SILT	2	1	0.04	25	20	1.17	--
25.7	5.2	0.0	0.19	SANDY SILT AND SILT	2	2	0.05	25	20	1.18	--
25.9	5.2	0.0	0.19	SANDY SILT AND SILT	2	2	0.05	25	20	1.18	--
26.0	5.4	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	25	20	1.19	--
26.2	5.7	0.0	0.18	SANDY SILT AND SILT	2	2	0.05	25	20	1.19	--
26.3	6.0	0.0	0.17	SANDY SILT AND SILT	2	2	0.05	26	20	1.20	--
26.5	6.1	0.0	0.16	SANDY SILT AND SILT	2	2	0.05	26	20	1.20	--
26.7	5.4	0.0	0.18	SANDY SILT AND SILT	2	2	0.05	25	20	1.21	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 18

DEPTH FT.	QC TSF	FS TSF	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF	
26.8	6.1	0.0	0.16	SANDY SILT AND SILT	2	2	0.05	26	20	1.21	--
27.0	6.1	0.0	0.16	SANDY SILT AND SILT	2	2	0.05	26	20	1.22	--
27.2	5.6	0.0	0.18	SANDY SILT AND SILT	2	2	0.05	25	20	1.22	--
27.3	5.1	0.0	0.20	SANDY SILT AND SILT	2	1	0.04	25	20	1.23	--
27.5	5.4	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	25	20	1.23	--
27.7	5.4	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	25	20	1.24	--
27.8	5.3	0.0	0.19	SANDY SILT AND SILT	2	2	0.05	25	20	1.25	--
28.0	5.4	0.0	0.18	SANDY SILT AND SILT	2	2	0.05	25	20	1.25	--
28.1	5.4	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	25	20	1.26	--
28.3	5.4	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	25	20	1.25	--
28.5	5.6	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	25	20	1.27	--
28.6	5.8	0.0	0.17	SANDY SILT AND SILT	2	2	0.05	25	20	1.27	--
28.8	6.1	0.0	0.16	SANDY SILT AND SILT	2	2	0.05	26	20	1.28	--
29.0	7.5	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	26	20	1.28	--
29.1	15.0	0.0	0.07	SILTY SAND	4	4	0.10	30	20	1.29	--
29.3	19.3	0.0	0.05	SILTY SAND	5	5	0.10	31	20	1.29	--
29.5	15.0	0.0	0.07	SILTY SAND	4	4	0.10	30	20	1.30	--
29.6	14.3	0.0	0.07	SILTY SAND	4	3	0.10	30	20	1.30	--
29.8	15.0	0.0	0.07	SILTY SAND	4	4	0.10	30	20	1.31	--
30.0	13.6	0.0	0.07	SILTY SAND	4	3	0.09	29	20	1.31	--
30.1	9.3	0.0	0.11	SANDY SILT AND SILT	3	3	0.05	27	20	1.32	--
30.3	15.0	0.1	0.87	SANDY SILT AND SILT	5	4	0.05	30	20	1.32	--
30.4	21.0	0.3	1.24	SILTY SAND	6	6	0.05	31	20	1.33	--
30.6	28.5	0.0	0.14	SILTY SAND	8	7	0.10	33	20	1.33	--
30.8	42.2	0.0	0.02	SAND	5	5	1.92	35	22	1.34	--
30.9	51.7	0.0	0.02	SAND	7	6	1.95	36	28	1.34	--
31.1	36.1	0.0	0.03	SAND	5	4	1.85	34	20	1.35	--
31.3	19.7	0.0	0.05	SILTY SAND	5	5	0.10	31	20	1.35	--
31.4	10.2	0.0	0.10	SILTY SAND	3	2	0.08	28	20	1.36	--
31.6	6.8	0.0	0.15	SANDY SILT AND SILT	2	2	0.05	26	20	1.36	--
31.8	6.7	0.0	0.15	SANDY SILT AND SILT	2	2	0.05	26	20	1.37	--
31.9	6.1	0.0	0.16	SANDY SILT AND SILT	2	2	0.05	25	20	1.37	--
32.1	6.1	0.0	0.16	SANDY SILT AND SILT	2	2	0.05	25	20	1.38	--
32.3	6.1	0.0	0.16	SANDY SILT AND SILT	2	2	0.05	25	20	1.38	--
32.4	6.8	0.0	0.15	SANDY SILT AND SILT	2	2	0.05	26	20	1.39	--
32.6	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	26	20	1.39	--
32.7	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	26	20	1.40	--
32.9	6.8	0.0	0.15	SANDY SILT AND SILT	2	2	0.05	26	20	1.40	--
33.1	6.9	0.0	0.14	SANDY SILT AND SILT	2	2	0.05	26	20	1.41	--
33.2	6.9	0.0	0.14	SANDY SILT AND SILT	2	2	0.05	26	20	1.41	--
33.4	6.8	0.0	0.15	SANDY SILT AND SILT	2	2	0.05	25	20	1.42	--
33.6	6.9	0.0	0.14	SANDY SILT AND SILT	2	2	0.05	26	20	1.42	--
33.7	7.2	0.0	0.14	SANDY SILT AND SILT	2	2	0.05	26	20	1.43	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 18

DEPTH FT.	QC TSF	FS TSF	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF	
33.9	7.0	0.0	0.14	SANDY SILT AND SILT	2	2	0.05	26	20	1.44	--
34.1	7.4	0.0	0.14	SANDY SILT AND SILT	2	2	0.05	26	20	1.44	--
34.2	7.5	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	26	20	1.45	--
34.4	6.8	0.0	0.15	SANDY SILT AND SILT	2	2	0.05	25	20	1.45	--
34.5	6.8	0.0	0.15	SANDY SILT AND SILT	2	2	0.05	25	20	1.46	--
34.7	6.8	0.0	0.15	SANDY SILT AND SILT	2	2	0.05	25	20	1.46	--
34.9	6.7	0.0	0.15	SANDY SILT AND SILT	2	2	0.05	25	20	1.47	--
35.0	6.9	0.0	0.14	SANDY SILT AND SILT	2	2	0.05	25	20	1.47	--
35.2	7.4	0.0	0.14	SANDY SILT AND SILT	2	2	0.05	26	20	1.48	--
35.4	6.8	0.0	0.15	SANDY SILT AND SILT	2	2	0.05	25	20	1.48	--
35.5	6.9	0.0	0.15	SANDY SILT AND SILT	2	2	0.05	25	20	1.49	--
35.7	7.5	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	26	20	1.49	--
35.9	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	26	20	1.50	--
36.0	8.8	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	27	20	1.50	--
36.2	8.4	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	26	20	1.51	--
36.4	7.5	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	26	20	1.51	--
36.5	8.1	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	26	20	1.52	--
36.7	9.4	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	27	20	1.52	--
36.8	10.9	0.0	0.09	SILTY SAND	3	2	0.09	27	20	1.53	--
37.0	15.0	0.0	0.13	SILTY SAND	4	3	0.09	29	20	1.53	--
37.2	16.3	0.0	0.06	SILTY SAND	4	4	0.10	29	20	1.54	--
37.3	18.4	0.0	0.05	SILTY SAND	5	4	0.10	30	20	1.54	--
37.5	10.7	0.0	0.09	SILTY SAND	3	2	0.08	27	20	1.55	--
37.7	10.2	0.1	0.69	SANDY SILT AND SILT	3	3	0.04	27	20	1.55	--
37.8	8.8	0.0	0.45	SANDY SILT AND SILT	3	2	0.04	26	20	1.56	--
38.0	10.2	0.1	0.78	SANDY SILT AND SILT	3	3	0.04	27	20	1.56	--
38.2	12.2	0.2	1.72	SANDY SILT AND SILT	5	4	0.03	28	20	1.57	--
38.3	12.9	0.3	1.93	SANDY SILT AND SILT	5	4	0.02	28	20	1.57	--
38.5	12.0	0.2	1.92	SANDY SILT AND SILT	5	4	0.02	28	20	1.58	--
38.6	10.9	0.2	2.11	SANDY SILT AND SILT	4	4	0.02	27	20	1.58	--
38.8	8.8	0.2	1.92	SANDY SILT AND SILT	4	3	0.02	26	20	1.59	--
39.0	10.2	0.1	0.69	SANDY SILT AND SILT	3	3	0.04	27	20	1.59	--
39.1	14.2	0.1	0.98	SANDY SILT AND SILT	5	4	0.05	29	20	1.60	--
39.3	17.0	0.3	1.59	SANDY SILT AND SILT	6	5	0.04	29	20	1.60	--
39.5	18.8	0.3	1.81	SANDY SILT AND SILT	6	5	0.04	30	20	1.61	--
39.6	19.2	0.4	2.09	SANDY SILT AND SILT	7	5	0.03	30	20	1.61	--
39.8	19.0	0.4	2.21	SANDY SILT AND SILT	7	5	0.03	30	20	1.62	--
40.0	19.7	0.4	2.23	SANDY SILT AND SILT	7	6	0.03	30	20	1.63	--
40.1	19.7	0.4	2.18	SANDY SILT AND SILT	7	6	0.03	30	20	1.63	--
40.3	19.7	0.4	1.77	SANDY SILT AND SILT	7	5	0.04	30	20	1.64	--
40.5	19.3	0.3	1.71	SANDY SILT AND SILT	6	5	0.04	30	20	1.64	--
40.6	18.4	0.2	1.09	SILTY SAND	6	4	0.05	30	20	1.65	--
40.8	18.1	0.2	1.05	SILTY SAND	5	4	0.05	30	20	1.65	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 18

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
40.9	20.2	0.2	1.14	SILTY SAND	6	5	0.06	30	20	1.66	--
41.1	21.1	0.3	1.38	SILTY SAND	6	5	0.05	30	20	1.66	--
41.3	20.4	0.2	1.13	SILTY SAND	5	5	0.06	30	20	1.67	--
41.4	21.3	0.2	0.89	SILTY SAND	6	5	0.07	30	20	1.67	--
41.6	21.8	0.2	1.01	SILTY SAND	6	5	0.06	30	20	1.68	--
41.8	25.5	0.4	1.49	SILTY SAND	8	6	0.05	31	20	1.68	--
41.9	29.2	0.6	2.15	SANDY SILT AND SILT	10	7	0.04	32	20	1.69	--
42.1	38.1	0.6	1.60	SILTY SAND	11	9	0.06	33	20	1.69	--
42.3	42.9	0.2	0.35	SAND	8	6	0.87	34	20	1.70	--
42.4	27.9	0.1	0.22	SILTY SAND	7	6	0.09	32	20	1.70	--
42.6	23.1	0.4	1.78	SANDY SILT AND SILT	7	6	0.04	31	20	1.71	--
42.7	38.8	0.6	1.42	SILTY SAND	11	9	0.07	33	20	1.71	--
42.9	42.0	0.6	1.43	SILTY SAND	12	9	0.07	34	20	1.72	--
43.1	39.7	0.8	2.04	SILTY SAND	12	9	0.05	33	20	1.72	--
43.2	36.7	0.9	2.53	SANDY SILT AND SILT	12	9	0.04	33	20	1.73	--
43.4	35.1	1.0	2.93	SANDY SILT AND SILT	13	10	0.03	33	20	1.73	--
43.6	37.7	1.0	2.55	SANDY SILT AND SILT	13	10	0.04	33	20	1.74	--
43.7	45.6	0.8	1.71	SILTY SAND	13	10	0.07	34	20	1.74	--
43.9	40.1	1.0	2.37	SANDY SILT AND SILT	13	10	0.04	33	20	1.75	--
44.1	36.7	1.0	2.75	SANDY SILT AND SILT	13	10	0.03	33	20	1.75	--
44.2	42.8	0.7	1.61	SILTY SAND	12	9	0.07	33	20	1.76	--
44.4	33.3	0.7	2.10	SANDY SILT AND SILT	11	8	0.05	32	20	1.76	--
44.6	21.8	0.8	3.54	CLAYEY SILT AND SILTY CLAY	10	8	---	---	---	1.77	1.19
44.7	23.7	0.7	2.91	SANDY SILT AND SILT	10	7	0.02	31	20	1.77	--
44.9	25.2	0.4	1.39	SILTY SAND	8	6	0.06	31	20	1.78	--
45.0	22.2	0.4	1.71	SANDY SILT AND SILT	7	5	0.04	30	20	1.78	--
45.2	17.7	0.5	2.60	SANDY SILT AND SILT	7	5	0.02	29	20	1.79	--
45.4	18.3	0.5	2.57	SANDY SILT AND SILT	7	5	0.02	29	20	1.79	--
45.5	19.7	0.5	2.48	SANDY SILT AND SILT	8	6	0.02	30	20	1.80	--
45.7	22.4	0.6	2.63	SANDY SILT AND SILT	9	6	0.02	30	20	1.80	--
45.9	36.0	0.3	0.75	SILTY SAND	10	7	0.09	32	20	1.81	--
46.0	68.8	0.1	0.12	SAND	9	7	1.81	36	30	1.82	--
46.2	78.5	0.2	0.23	SAND	11	8	1.66	36	34	1.82	--
46.4	79.6	0.2	0.30	SAND	11	8	1.56	36	35	1.83	--
46.5	80.2	0.3	0.36	SAND	12	9	1.47	36	35	1.83	--
46.7	78.3	0.4	0.45	SAND	12	9	1.34	36	34	1.84	--
46.8	77.1	0.4	0.45	SAND	12	9	1.32	36	33	1.84	--
47.0	77.5	0.3	0.44	SAND	12	9	1.34	36	33	1.85	--
47.2	75.5	0.4	0.48	SAND	12	9	1.27	36	32	1.85	--
47.3	64.7	0.7	1.10	SILTY SAND	17	12	0.10	35	27	1.86	--
47.5	48.3	1.0	1.99	SILTY SAND	14	10	0.06	34	20	1.86	--
47.7	40.3	0.4	0.99	SILTY SAND	11	8	0.09	33	20	1.87	--
47.8	32.1	0.5	1.68	SILTY SAND	10	7	0.06	32	20	1.87	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 18

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
48.0	26.5	0.5	1.92	SANDY SILT AND SILT	9	6	0.04	31	20	1.88	--
48.2	56.5	0.2	0.34	SAND	9	6	1.31	35	22	1.88	--
48.3	69.4	0.4	0.50	SAND	11	8	1.17	36	29	1.89	--
48.5	47.5	0.5	1.12	SILTY SAND	13	9	0.09	34	20	1.89	--
48.7	29.9	0.5	1.60	SILTY SAND	9	6	0.06	31	20	1.90	--
48.8	23.6	0.5	1.91	SANDY SILT AND SILT	8	6	0.04	30	20	1.90	--
49.0	18.4	0.3	1.80	SANDY SILT AND SILT	6	5	0.04	29	20	1.91	--
49.1	14.3	0.2	1.40	SANDY SILT AND SILT	5	3	0.04	28	20	1.91	--
49.3	10.3	0.1	1.25	SANDY SILT AND SILT	4	3	0.03	26	20	1.92	--
49.5	10.2	0.1	1.37	SANDY SILT AND SILT	4	3	0.03	26	20	1.92	--
49.6	13.0	0.2	1.47	SANDY SILT AND SILT	5	3	0.03	27	20	1.93	--
49.8	14.3	0.3	1.89	SANDY SILT AND SILT	5	4	0.03	28	20	1.93	--
50.0	13.9	0.3	2.15	SANDY SILT AND SILT	5	4	0.02	27	20	1.94	--
50.1	14.3	0.4	2.66	SANDY SILT AND SILT	6	4	0.01	28	20	1.94	--
50.3	19.4	0.4	1.80	SANDY SILT AND SILT	7	5	0.04	29	20	1.95	--
50.5	24.3	0.2	0.86	SILTY SAND	7	5	0.07	30	20	1.95	--
50.6	17.3	0.2	1.10	SANDY SILT AND SILT	5	4	0.05	29	20	1.96	--
50.8	14.3	0.2	1.33	SANDY SILT AND SILT	5	3	0.04	28	20	1.96	--
51.0	21.1	0.0	0.14	SILTY SAND	6	4	0.09	29	20	1.97	--
51.1	39.4	0.2	0.61	SILTY SAND	10	7	0.10	33	20	1.97	--
51.3	26.5	0.3	1.06	SILTY SAND	8	5	0.07	31	20	1.98	--
51.4	20.9	0.2	0.91	SILTY SAND	6	4	0.07	29	20	1.98	--
51.6	12.9	0.1	0.93	SANDY SILT AND SILT	4	3	0.04	27	20	1.99	--
51.8	9.5	0.0	0.11	SILTY SAND	3	2	0.05	25	20	1.99	--
51.9	9.1	0.0	0.33	SANDY SILT AND SILT	3	2	0.04	25	20	2.00	--
52.1	11.6	0.0	0.09	SILTY SAND	3	2	0.09	26	20	2.01	--
52.3	10.2	0.0	0.10	SILTY SAND	3	2	0.08	26	20	2.01	--
52.4	8.8	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	25	20	2.02	--
52.6	8.3	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	25	20	2.02	--
52.8	8.8	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	25	20	2.03	--
52.9	8.8	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	25	20	2.03	--
53.1	9.6	0.0	0.10	SILTY SAND	3	2	0.06	25	20	2.04	--
53.2	10.2	0.0	0.10	SILTY SAND	3	2	0.08	26	20	2.04	--
53.4	13.5	0.0	0.07	SILTY SAND	4	2	0.09	27	20	2.05	--
53.6	14.3	0.2	1.33	SANDY SILT AND SILT	5	3	0.04	27	20	2.05	--
53.7	22.2	0.1	0.50	SILTY SAND	6	4	0.08	29	20	2.06	--
53.9	13.8	0.1	0.73	SANDY SILT AND SILT	4	3	0.05	27	20	2.06	--
54.1	12.9	0.0	0.31	SILTY SAND	4	3	0.07	27	20	2.07	--
54.2	12.2	0.0	0.25	SILTY SAND	3	2	0.08	27	20	2.07	--
54.4	12.9	0.0	0.08	SILTY SAND	3	2	0.09	27	20	2.08	--
54.6	13.5	0.0	0.07	SILTY SAND	4	2	0.09	27	20	2.08	--
54.7	14.3	0.1	0.56	SILTY SAND	4	3	0.06	27	20	2.09	--
54.9	16.2	0.1	0.55	SILTY SAND	5	3	0.07	28	20	2.09	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 18

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PC TSF	SU TSF.	
55.1	22.4	0.0	0.13	SILTY SAND	6	4	0.09	29	20	2.10	--
55.2	16.9	0.1	0.35	SILTY SAND	5	3	0.08	28	20	2.10	--
55.4	12.9	0.1	0.62	SANDY SILT AND SILT	4	3	0.05	27	20	2.11	--
55.5	12.2	0.0	0.16	SILTY SAND	3	2	0.08	26	20	2.11	--
55.7	11.6	0.0	0.09	SILTY SAND	3	2	0.09	26	20	2.12	--
55.9	10.8	0.0	0.09	SILTY SAND	3	2	0.09	26	20	2.12	--
56.0	10.2	0.0	0.10	SILTY SAND	3	2	0.08	26	20	2.13	--
56.2	9.2	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	25	20	2.13	--
56.4	8.2	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	24	20	2.14	--
56.5	8.1	0.0	0.12	SANDY SILT AND SILT	3	2	0.05	24	20	2.14	--
56.7	7.5	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	24	20	2.15	--
56.9	8.7	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	25	20	2.15	--
57.0	9.4	0.0	0.11	SANDY SILT AND SILT	3	2	0.05	25	20	2.16	--
57.2	10.3	0.0	0.10	SILTY SAND	3	2	0.08	25	20	2.16	--
57.3	10.3	0.0	0.10	SILTY SAND	3	2	0.08	25	20	2.17	--
57.5	10.2	0.0	0.20	SILTY SAND	3	2	0.06	25	20	2.17	--
57.7	11.1	0.1	0.72	SANDY SILT AND SILT	4	2	0.04	26	20	2.18	--
57.8	12.0	0.1	1.08	SANDY SILT AND SILT	4	3	0.04	26	20	2.18	--
58.0	14.5	0.2	1.38	SANDY SILT AND SILT	5	3	0.04	27	20	2.19	--
58.2	16.3	0.1	0.61	SILTY SAND	5	3	0.07	28	20	2.20	--
58.3	12.3	0.1	0.81	SANDY SILT AND SILT	4	3	0.04	26	20	2.20	--
58.5	10.8	0.1	0.55	SANDY SILT AND SILT	3	2	0.05	26	20	2.21	--
58.7	10.2	0.1	0.59	SANDY SILT AND SILT	3	2	0.04	25	20	2.21	--
58.8	12.6	0.2	1.58	SANDY SILT AND SILT	5	3	0.03	26	20	2.22	--
59.0	15.3	0.4	2.49	SANDY SILT AND SILT	6	4	0.02	27	20	2.22	--
59.2	17.6	0.5	2.95	SANDY SILT AND SILT	8	5	0.01	28	20	2.23	--
59.3	18.4	0.5	2.67	SANDY SILT AND SILT	7	5	0.02	28	20	2.23	--
59.5	16.9	0.4	2.60	SANDY SILT AND SILT	7	5	0.02	28	20	2.24	--
59.6	15.0	0.4	2.47	SANDY SILT AND SILT	6	4	0.02	27	20	2.24	--
59.8	14.5	0.3	1.36	SANDY SILT AND SILT	5	3	0.03	27	20	2.25	--
60.0	13.6	0.2	1.40	SANDY SILT AND SILT	5	3	0.04	27	20	2.25	--
60.1	13.6	0.2	1.18	SANDY SILT AND SILT	5	3	0.04	27	20	2.26	--
60.3	13.0	0.1	0.92	SANDY SILT AND SILT	4	3	0.04	26	20	2.26	--
60.5	13.6	0.1	0.74	SANDY SILT AND SILT	4	3	0.05	27	20	2.27	--
60.6	13.5	0.1	0.74	SANDY SILT AND SILT	4	3	0.05	27	20	2.27	--
60.8	13.5	0.1	0.74	SANDY SILT AND SILT	4	3	0.05	27	20	2.28	--
61.0	13.6	0.1	0.74	SANDY SILT AND SILT	4	3	0.05	27	20	2.28	--
61.1	13.1	0.1	0.69	SANDY SILT AND SILT	4	3	0.05	26	20	2.29	--
61.3	12.9	0.1	0.85	SANDY SILT AND SILT	4	3	0.05	26	20	2.29	--
61.4	14.4	0.2	1.13	SANDY SILT AND SILT	5	3	0.04	27	20	2.30	--
61.6	15.0	0.2	1.53	SANDY SILT AND SILT	5	3	0.04	27	20	2.30	--
61.7	17.0	0.2	1.41	SANDY SILT AND SILT	6	4	0.04	28	20	2.31	--
61.9	20.8	0.3	1.25	SILTY SAND	6	4	0.05	29	20	2.31	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 18

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
62.1	22.4	0.2	0.39	SILTY SAND	6	4	0.07	29	20	2.32	--
62.2	24.5	0.2	0.74	SILTY SAND	7	4	0.08	29	20	2.32	--
62.4	24.5	0.1	0.41	SILTY SAND	7	4	0.09	29	20	2.33	--
62.6	21.1	0.1	0.57	SILTY SAND	6	4	0.08	29	20	2.33	--
62.7	18.2	0.0	0.06	SILTY SAND	5	3	0.10	28	20	2.34	--
62.9	13.8	0.0	0.07	SILTY SAND	4	2	0.09	27	20	2.34	--
63.1	12.2	0.0	0.08	SILTY SAND	3	2	0.09	26	20	2.35	--
63.2	12.9	0.0	0.08	SILTY SAND	3	2	0.09	26	20	2.35	--
63.4	14.2	0.0	0.07	SILTY SAND	4	2	0.10	27	20	2.36	--
63.5	15.0	0.0	0.07	SILTY SAND	4	2	0.10	27	20	2.36	--
63.7	15.0	0.4	2.34	SANDY SILT AND SILT	5	4	0.02	27	20	2.37	--
63.9	43.8	0.7	1.55	SILTY SAND	13	8	0.07	32	20	2.37	--
64.0	172.0	0.7	0.42	SAND	24	15	1.60	39	58	2.38	--
64.2	265.1	0.7	0.26	SAND	34	21	1.95	41	77	2.38	--
64.4	309.2	0.7	0.22	SAND AND GRAVEL	36	22	2.87	42	83	2.39	--
64.5	324.3	1.1	0.33	SAND AND GRAVEL	40	25	2.14	42	85	2.39	--
64.7	325.7	1.3	0.40	SAND	41	26	1.96	42	85	2.40	--
64.9	345.6	2.0	0.58	SAND	45	28	1.93	42	87	2.40	--
65.0	336.6	3.2	0.94	SAND	49	30	1.49	42	86	2.41	--
65.2	336.5	1.9	0.55	SAND	44	27	1.84	42	86	2.41	--
65.4	340.4	1.1	0.33	SAND AND GRAVEL	41	25	2.38	42	86	2.42	--
65.5	365.2	1.3	0.35	SAND AND GRAVEL	44	27	2.56	42	89	2.43	--
65.7	419.7	1.2	0.29	SAND AND GRAVEL	47	29	3.35	43	95	2.43	--
65.9	550.3	2.0	0.35	SAND AND GRAVEL	61	38	3.47	44	100	2.44	--
66.0	646.0	2.9	0.45	SAND AND GRAVEL	73	45	3.27	45	100	2.44	--

INTERPRETED CONE PENETRATION TEST DATA

C. P.T. - 20

CLIENT: HOWARD HUGHES PROPERTIES
 JOB NO: AE-88473
 DATE: JANUARY 30, 1989
 ELEVATION: 12.3 FT
 UNIT WEIGHT OF SOIL: 125 PCF
 DEPTH TO GROUND WATER: 10.1 FT

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF
0.1	16.3	0.4	2.57	SANDY SILT AND SILT	7 25	0.02	--	---	0.01	--
0.3	13.6	0.5	3.68	CLAYEY SILT AND SILTY CLAY	7 22	---	--	---	0.02	0.85
0.4	15.0	0.5	3.21	CLAYEY SILT AND SILTY CLAY	7 21	---	--	---	0.03	0.93
0.6	17.6	0.5	2.72	SANDY SILT AND SILT	7 20	0.02	--	---	0.04	--
0.8	16.1	0.5	3.23	CLAYEY SILT AND SILTY CLAY	7 20	---	--	---	0.05	1.00
0.9	15.0	0.6	3.74	CLAYEY SILT AND SILTY CLAY	8 19	---	--	---	0.06	0.93
1.1	14.2	0.6	3.94	CLAYEY SILT AND SILTY CLAY	7 18	---	--	---	0.07	0.88
1.2	16.3	0.6	3.68	CLAYEY SILT AND SILTY CLAY	8 19	---	--	---	0.08	1.02
1.4	17.7	0.6	3.44	CLAYEY SILT AND SILTY CLAY	8 19	---	--	---	0.09	1.10
1.6	16.3	0.6	3.74	CLAYEY SILT AND SILTY CLAY	8 18	---	--	---	0.10	1.01
1.7	18.2	0.6	3.52	CLAYEY SILT AND SILTY CLAY	9 19	---	--	---	0.11	1.13
1.9	19.0	0.5	2.73	SANDY SILT AND SILT	8 17	0.02	--	---	0.12	--
2.1	16.3	0.6	3.37	CLAYEY SILT AND SILTY CLAY	8 16	---	--	---	0.13	1.01
2.2	13.0	0.4	3.38	CLAYEY SILT AND SILTY CLAY	6 13	---	--	---	0.14	0.80
2.4	12.2	0.7	5.47	CLAY	12 25	---	--	---	0.15	0.76
2.6	39.9	1.1	2.66	SANDY SILT AND SILT	14 27	0.04	--	---	0.16	--
2.7	40.4	1.3	3.27	SANDY SILT AND SILT	16 31	0.02	--	---	0.17	--
2.9	35.4	1.2	3.31	SANDY SILT AND SILT	15 28	0.02	--	---	0.18	--
3.1	32.4	1.0	3.15	SANDY SILT AND SILT	13 25	0.02	--	---	0.19	--
3.2	28.6	1.0	3.40	SANDY SILT AND SILT	13 23	0.01	--	---	0.20	--
3.4	35.2	0.9	2.56	SANDY SILT AND SILT	12 22	0.04	--	---	0.21	--
3.5	29.9	0.9	2.91	SANDY SILT AND SILT	12 21	0.02	--	---	0.22	--
3.7	24.5	0.6	2.57	SANDY SILT AND SILT	9 16	0.03	--	---	0.23	--
3.9	19.0	0.5	2.53	SANDY SILT AND SILT	7 13	0.02	--	---	0.24	--
4.0	16.3	0.3	1.90	SANDY SILT AND SILT	6 10	0.03	38	35	0.25	--
4.2	12.2	0.3	2.38	SANDY SILT AND SILT	5 9	0.01	37	28	0.26	--
4.3	10.9	0.3	3.03	CLAYEY SILT AND SILTY CLAY	5 9	---	--	---	0.27	0.66
4.5	10.9	0.4	3.22	CLAYEY SILT AND SILTY CLAY	5 9	---	--	---	0.28	0.66
4.7	11.2	0.3	2.85	CLAYEY SILT AND SILTY CLAY	5 9	---	--	---	0.29	0.68
4.8	10.9	0.3	3.13	CLAYEY SILT AND SILTY CLAY	5 9	---	--	---	0.30	0.66
5.0	10.9	0.3	2.85	CLAYEY SILT AND SILTY CLAY	5 8	---	--	---	0.31	0.66
5.2	8.2	0.3	3.19	CLAYEY SILT AND SILTY CLAY	4 7	---	--	---	0.32	0.49
5.3	9.5	0.3	2.73	CLAYEY SILT AND SILTY CLAY	4 7	---	--	---	0.33	0.57
5.5	13.4	0.3	2.17	SANDY SILT AND SILT	5 8	0.02	36	25	0.34	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 20

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.
5.6	9.5	0.3	2.74	CLAYEY SILT AND SILTY CLAY	4	7	---	---	0.35	0.57
5.8	10.3	0.2	1.95	SANDY SILT AND SILT	4	7	0.02	34	20	0.36
6.0	12.2	0.2	1.31	SANDY SILT AND SILT	4	7	0.04	35	21	0.37
6.1	14.2	0.2	1.06	SANDY SILT AND SILT	5	7	0.04	36	24	0.38
6.3	11.7	0.2	1.53	SANDY SILT AND SILT	4	6	0.03	34	20	0.39
6.5	12.2	0.2	1.63	SANDY SILT AND SILT	5	7	0.03	35	20	0.40
6.6	12.2	0.2	1.80	SANDY SILT AND SILT	5	7	0.03	34	20	0.41
6.8	12.7	0.2	1.74	SANDY SILT AND SILT	5	7	0.03	34	20	0.42
7.0	9.2	0.2	2.49	CLAYEY SILT AND SILTY CLAY	4	6	---	---	0.43	0.55
7.1	10.9	0.2	1.93	SANDY SILT AND SILT	4	6	0.02	34	20	0.44
7.3	10.8	0.2	1.95	SANDY SILT AND SILT	4	6	0.02	33	20	0.46
7.4	9.5	0.2	2.00	SANDY SILT AND SILT	4	6	0.02	33	20	0.47
7.6	9.8	0.2	1.74	SANDY SILT AND SILT	4	5	0.02	33	20	0.48
7.8	11.0	0.2	1.55	SANDY SILT AND SILT	4	6	0.03	33	20	0.49
7.9	12.8	0.2	1.49	SANDY SILT AND SILT	5	6	0.03	34	20	0.50
8.1	12.2	0.2	1.80	SANDY SILT AND SILT	5	6	0.03	33	20	0.51
8.3	10.5	0.2	1.72	SANDY SILT AND SILT	4	5	0.02	33	20	0.52
8.4	13.6	0.3	1.84	SANDY SILT AND SILT	5	7	0.03	34	20	0.53
8.6	9.9	0.2	2.13	SANDY SILT AND SILT	4	6	0.01	32	20	0.54
8.8	6.8	0.2	2.21	CLAYEY SILT AND SILTY CLAY	3	4	---	---	0.55	0.39
8.9	8.2	0.2	2.33	CLAYEY SILT AND SILTY CLAY	4	5	---	---	0.56	0.48
9.1	10.2	0.3	2.56	CLAYEY SILT AND SILTY CLAY	5	6	---	---	0.57	0.60
9.3	10.2	0.4	4.02	CLAYEY SILT AND SILTY CLAY	6	7	---	---	0.58	0.60
9.4	10.8	0.4	3.51	CLAYEY SILT AND SILTY CLAY	6	7	---	---	0.59	0.64
9.6	9.5	0.4	3.68	CLAYEY SILT AND SILTY CLAY	5	7	---	---	0.60	0.56
9.7	12.2	0.4	3.10	CLAYEY SILT AND SILTY CLAY	6	7	---	---	0.61	0.73
9.9	13.6	0.5	3.53	CLAYEY SILT AND SILTY CLAY	7	9	---	---	0.62	0.81
10.1	11.6	0.5	4.24	CLAYEY SILT AND SILTY CLAY	7	8	---	---	0.63	0.68
10.2	12.2	0.4	2.94	CLAYEY SILT AND SILTY CLAY	6	7	---	---	0.64	0.73
10.4	83.0	0.5	0.55	SAND	13	16	1.21	42	63	0.64
10.6	89.3	0.3	0.92	SAND	17	21	0.75	42	66	0.65
10.7	55.8	1.2	2.10	SILTY SAND	17	20	0.06	40	50	0.65
10.9	23.1	0.8	3.46	CLAYEY SILT AND SILTY CLAY	11	13	---	---	0.66	1.40
11.1	13.4	0.5	3.95	CLAYEY SILT AND SILTY CLAY	7	9	---	---	0.66	0.80
11.2	33.6	0.4	1.25	SILTY SAND	10	12	0.07	37	34	0.67
11.4	58.5	0.5	0.77	SAND	13	15	0.49	40	50	0.67
11.5	66.6	0.4	0.66	SAND	12	15	0.37	40	55	0.68
11.7	61.6	0.5	0.88	SAND	14	17	0.38	40	52	0.68
11.9	55.8	0.5	0.88	SAND	14	17	0.16	39	48	0.69
12.0	50.3	0.4	0.83	SILTY SAND	13	16	0.10	39	45	0.69
12.2	43.5	0.4	0.94	SILTY SAND	12	14	0.09	38	41	0.70
12.4	33.5	0.4	1.28	SILTY SAND	10	12	0.07	37	33	0.70
12.5	27.2	0.3	1.10	SILTY SAND	8	9	0.07	36	27	0.71

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 20

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
12.7	27.5	0.2	0.75	SILTY SAND	8	9	0.08	36	27	0.71	--
12.9	23.1	0.3	1.25	SILTY SAND	7	8	0.06	35	22	0.72	--
13.0	15.0	0.2	1.47	SANDY SILT AND SILT	5	6	0.04	33	20	0.72	--
13.2	12.2	0.2	1.23	SANDY SILT AND SILT	4	5	0.04	32	20	0.73	--
13.4	8.0	0.1	1.62	SANDY SILT AND SILT	3	4	0.02	30	20	0.73	--
13.5	6.3	0.1	1.76	SANDY SILT AND SILT	3	3	0.01	29	20	0.74	--
13.7	8.1	0.2	2.22	CLAYEY SILT AND SILTY CLAY	4	4	---	---	---	0.74	0.45
13.8	6.8	0.2	3.24	CLAYEY SILT AND SILTY CLAY	4	4	---	---	---	0.75	0.37
14.0	5.4	0.2	3.49	CLAY	5	6	---	---	---	0.75	0.29
14.2	5.0	0.1	2.21	CLAYEY SILT AND SILTY CLAY	2	3	---	---	---	0.76	0.26
14.3	4.1	0.1	1.72	CLAYEY SILT AND SILTY CLAY	2	2	---	---	---	0.76	0.20
14.5	4.1	0.1	1.72	CLAYEY SILT AND SILTY CLAY	2	2	---	---	---	0.77	0.20
14.7	4.1	0.1	1.47	CLAYEY SILT AND SILTY CLAY	2	2	---	---	---	0.77	0.20
14.8	4.1	0.1	1.94	CLAYEY SILT AND SILTY CLAY	2	2	---	---	---	0.78	0.20
15.0	10.8	0.1	1.29	SANDY SILT AND SILT	4	4	0.03	31	20	0.78	--
15.2	23.1	0.2	0.74	SILTY SAND	7	7	0.08	34	20	0.79	--
15.3	45.6	0.1	0.31	SAND	8	8	1.12	38	39	0.79	--
15.5	54.4	0.3	0.51	SAND	10	11	0.89	39	44	0.80	--
15.7	51.6	0.4	0.78	SAND	13	14	0.19	38	42	0.81	--
15.8	43.5	0.0	0.09	SAND	6	7	1.71	37	37	0.81	--
16.0	38.1	0.4	0.95	SILTY SAND	10	12	0.08	37	32	0.82	--
16.2	19.0	0.3	1.79	SANDY SILT AND SILT	6	7	0.04	33	20	0.82	--
16.3	24.5	0.3	1.18	SILTY SAND	7	8	0.06	34	20	0.83	--
16.5	20.4	0.3	1.23	SILTY SAND	6	7	0.05	34	20	0.83	--
16.7	13.6	0.2	1.40	SANDY SILT AND SILT	5	5	0.04	31	20	0.84	--
16.8	8.1	0.1	1.36	SANDY SILT AND SILT	3	3	0.03	29	20	0.84	--
17.0	5.4	0.1	1.29	SANDY SILT AND SILT	2	2	0.02	27	20	0.85	--
17.2	5.4	0.1	1.29	SANDY SILT AND SILT	2	2	0.02	27	20	0.85	--
17.3	5.4	0.1	1.65	CLAYEY SILT AND SILTY CLAY	2	3	---	---	---	0.86	0.27
17.5	5.4	0.1	1.47	SANDY SILT AND SILT	2	2	0.01	27	20	0.86	--
17.7	5.4	0.1	1.12	SANDY SILT AND SILT	2	2	0.02	27	20	0.87	--
17.8	5.0	0.0	0.80	SANDY SILT AND SILT	2	2	0.03	26	20	0.87	--
18.0	5.3	0.0	0.75	SANDY SILT AND SILT	2	2	0.03	27	20	0.88	--
18.1	5.4	0.1	0.92	SANDY SILT AND SILT	2	2	0.03	27	20	0.88	--
18.3	5.4	0.1	1.11	SANDY SILT AND SILT	2	2	0.02	27	20	0.89	--
18.5	5.4	0.1	0.93	SANDY SILT AND SILT	2	2	0.03	27	20	0.89	--
18.6	5.4	0.1	0.92	SANDY SILT AND SILT	2	2	0.03	27	20	0.90	--
18.8	5.4	0.0	0.74	SANDY SILT AND SILT	2	2	0.03	27	20	0.90	--
19.0	5.4	0.0	0.74	SANDY SILT AND SILT	2	2	0.03	27	20	0.91	--
19.1	5.4	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	27	20	0.91	--
19.3	5.4	0.0	0.55	SANDY SILT AND SILT	2	2	0.04	27	20	0.92	--
19.5	5.4	0.0	0.74	SANDY SILT AND SILT	2	2	0.03	27	20	0.92	--
19.6	5.4	0.0	0.74	SANDY SILT AND SILT	2	2	0.03	26	20	0.93	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 20

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
19.8	5.4	0.0	0.74	SANDY SILT AND SILT	2	2	0.03	26	20	0.93	--
19.9	5.4	0.0	0.74	SANDY SILT AND SILT	2	2	0.03	26	20	0.94	--
20.1	5.4	0.0	0.55	SANDY SILT AND SILT	2	2	0.04	26	20	0.94	--
20.3	5.4	0.0	0.56	SANDY SILT AND SILT	2	2	0.04	26	20	0.95	--
20.4	5.4	0.0	0.55	SANDY SILT AND SILT	2	2	0.04	26	20	0.95	--
20.6	5.7	0.0	0.53	SANDY SILT AND SILT	2	2	0.04	27	20	0.96	--
20.8	5.6	0.0	0.53	SANDY SILT AND SILT	2	2	0.04	26	20	0.97	--
20.9	5.4	0.1	0.92	SANDY SILT AND SILT	2	2	0.03	26	20	0.97	--
21.1	5.4	0.1	1.10	SANDY SILT AND SILT	2	2	0.02	26	20	0.98	--
21.3	5.4	0.1	1.65	CLAYEY SILT AND SILTY CLAY	2	2	----	--	----	0.98	0.26
21.4	5.4	0.1	1.47	SANDY SILT AND SILT	2	2	0.01	26	20	0.99	--
21.6	5.4	0.1	1.29	SANDY SILT AND SILT	2	2	0.02	26	20	0.99	--
21.8	5.4	0.1	0.92	SANDY SILT AND SILT	2	2	0.03	26	20	1.00	--
21.9	5.4	0.1	1.10	SANDY SILT AND SILT	2	2	0.02	26	20	1.00	--
22.1	5.4	0.1	1.10	SANDY SILT AND SILT	2	2	0.02	26	20	1.01	--
22.2	5.4	0.1	1.29	SANDY SILT AND SILT	2	2	0.02	26	20	1.01	--
22.4	5.4	0.0	0.55	SANDY SILT AND SILT	2	2	0.04	26	20	1.02	--
22.6	5.4	0.1	1.10	SANDY SILT AND SILT	2	2	0.02	26	20	1.02	--
22.7	4.1	0.1	1.46	CLAYEY SILT AND SILTY CLAY	2	2	----	--	----	1.03	0.17
22.9	5.4	0.1	0.92	SANDY SILT AND SILT	2	2	0.03	26	20	1.03	--
23.1	5.4	0.0	0.56	SANDY SILT AND SILT	2	2	0.04	26	20	1.04	--
23.2	5.4	0.0	0.56	SANDY SILT AND SILT	2	2	0.04	26	20	1.04	--
23.4	5.4	0.1	1.10	SANDY SILT AND SILT	2	2	0.02	26	20	1.05	--
23.6	5.4	0.1	1.29	SANDY SILT AND SILT	2	2	0.02	26	20	1.05	--
23.7	5.4	0.1	1.29	SANDY SILT AND SILT	2	2	0.02	26	20	1.06	--
23.9	5.7	0.1	1.23	SANDY SILT AND SILT	2	2	0.02	26	20	1.06	--
24.0	5.4	0.1	1.29	SANDY SILT AND SILT	2	2	0.02	26	20	1.07	--
24.2	5.4	0.1	0.92	SANDY SILT AND SILT	2	2	0.03	26	20	1.07	--
24.4	5.4	0.0	0.74	SANDY SILT AND SILT	2	2	0.03	26	20	1.08	--
24.5	5.4	0.0	0.37	SANDY SILT AND SILT	2	2	0.04	26	20	1.08	--
24.7	5.4	0.0	0.37	SANDY SILT AND SILT	2	2	0.04	26	20	1.09	--
24.9	5.5	0.0	0.55	SANDY SILT AND SILT	2	2	0.04	26	20	1.09	--
25.0	5.4	0.0	0.74	SANDY SILT AND SILT	2	2	0.03	26	20	1.10	--
25.2	5.4	0.1	1.10	SANDY SILT AND SILT	2	2	0.02	26	20	1.10	--
25.4	6.0	0.1	1.18	SANDY SILT AND SILT	2	2	0.02	26	20	1.11	--
25.5	6.8	0.1	1.03	SANDY SILT AND SILT	2	2	0.03	27	20	1.11	--
25.7	6.8	0.0	0.15	SANDY SILT AND SILT	2	2	0.05	27	20	1.12	--
25.9	6.8	0.1	1.18	SANDY SILT AND SILT	3	2	0.03	27	20	1.12	--
26.0	4.1	0.1	1.72	CLAYEY SILT AND SILTY CLAY	2	2	----	--	----	1.13	0.15
26.2	6.2	0.1	1.14	SANDY SILT AND SILT	2	2	0.03	26	20	1.13	--
26.3	6.8	0.1	1.18	SANDY SILT AND SILT	3	2	0.03	27	20	1.14	--
26.5	6.8	0.1	1.18	SANDY SILT AND SILT	3	2	0.03	27	20	1.14	--
26.7	6.6	0.1	1.21	SANDY SILT AND SILT	3	2	0.02	26	20	1.15	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 20

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
26.8	6.1	0.0	0.65	SANDY SILT AND SILT	2	2	0.04	26	20	1.16	--
27.0	6.8	0.0	0.15	SANDY SILT AND SILT	2	2	0.05	26	20	1.16	--
27.2	5.4	0.0	0.13	SANDY SILT AND SILT	2	2	0.05	25	20	1.17	--
27.3	5.4	0.0	0.74	SANDY SILT AND SILT	2	2	0.03	25	20	1.17	--
27.5	6.0	0.0	0.67	SANDY SILT AND SILT	2	2	0.04	26	20	1.18	--
27.7	5.4	0.1	1.10	SANDY SILT AND SILT	2	2	0.02	25	20	1.18	--
27.8	5.7	0.0	0.71	SANDY SILT AND SILT	2	2	0.03	25	20	1.19	--
28.0	5.4	0.1	0.92	SANDY SILT AND SILT	2	2	0.03	25	20	1.19	--
28.1	5.4	0.1	1.10	SANDY SILT AND SILT	2	2	0.02	25	20	1.20	--
28.3	5.6	0.1	1.44	SANDY SILT AND SILT	2	2	0.02	25	20	1.20	--
28.5	6.3	0.1	1.32	SANDY SILT AND SILT	3	2	0.02	26	20	1.21	--
28.6	6.8	0.1	1.47	SANDY SILT AND SILT	3	2	0.02	26	20	1.21	--
28.8	6.9	0.1	1.16	SANDY SILT AND SILT	3	2	0.03	26	20	1.22	--
29.0	6.8	0.0	0.15	SANDY SILT AND SILT	2	2	0.05	26	20	1.22	--
29.1	6.8	0.1	1.32	SANDY SILT AND SILT	3	2	0.02	26	20	1.23	--
29.3	4.8	0.1	2.03	CLAYEY SILT AND SILTY CLAY	2	2	---	---	---	1.23	0.19
29.5	6.8	0.1	1.03	SANDY SILT AND SILT	2	2	0.03	26	20	1.24	--
29.6	5.9	0.0	0.68	SANDY SILT AND SILT	2	2	0.03	25	20	1.24	--
29.8	5.5	0.0	0.18	SANDY SILT AND SILT	2	2	0.05	25	20	1.25	--
30.0	5.6	0.0	0.18	SANDY SILT AND SILT	2	2	0.05	25	20	1.25	--
30.1	5.7	0.0	0.18	SANDY SILT AND SILT	2	2	0.05	25	20	1.26	--
30.3	5.5	0.0	0.18	SANDY SILT AND SILT	2	2	0.05	25	20	1.26	--
30.4	5.7	0.0	0.17	SANDY SILT AND SILT	2	2	0.05	25	20	1.27	--
30.6	6.1	0.0	0.16	SANDY SILT AND SILT	2	2	0.05	26	20	1.27	--
30.8	6.3	0.0	0.16	SANDY SILT AND SILT	2	2	0.05	26	20	1.28	--
30.9	6.8	0.0	0.44	SANDY SILT AND SILT	2	2	0.04	26	20	1.28	--
31.1	6.8	0.1	0.88	SANDY SILT AND SILT	2	2	0.03	26	20	1.29	--
31.3	6.8	0.1	1.32	SANDY SILT AND SILT	3	2	0.02	26	20	1.29	--
31.4	8.2	0.1	1.35	SANDY SILT AND SILT	3	3	0.03	27	20	1.30	--
31.6	8.2	0.2	2.08	SANDY SILT AND SILT	4	3	0.01	27	20	1.30	--
31.8	9.5	0.1	1.47	SANDY SILT AND SILT	4	3	0.03	28	20	1.31	--
31.9	9.8	0.1	1.43	SANDY SILT AND SILT	4	3	0.03	28	20	1.31	--
32.1	8.2	0.1	1.72	SANDY SILT AND SILT	3	3	0.02	27	20	1.32	--
32.3	8.3	0.1	1.70	SANDY SILT AND SILT	3	3	0.02	27	20	1.32	--
32.4	8.2	0.2	1.84	SANDY SILT AND SILT	3	3	0.02	27	20	1.33	--
32.6	7.6	0.2	2.23	CLAYEY SILT AND SILTY CLAY	3	3	---	---	---	1.33	0.35
32.7	8.2	0.2	2.21	SANDY SILT AND SILT	4	3	0.01	27	20	1.34	--
32.9	8.3	0.2	1.94	SANDY SILT AND SILT	3	3	0.02	27	20	1.35	--
33.1	8.2	0.1	1.47	SANDY SILT AND SILT	3	3	0.02	27	20	1.35	--
33.2	6.9	0.1	1.01	SANDY SILT AND SILT	3	2	0.03	26	20	1.36	--
33.4	7.1	0.1	0.70	SANDY SILT AND SILT	2	2	0.04	26	20	1.36	--
33.6	7.4	0.1	0.82	SANDY SILT AND SILT	3	2	0.03	26	20	1.37	--
33.7	7.6	0.1	1.06	SANDY SILT AND SILT	3	2	0.03	26	20	1.37	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 20

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.
33.9	8.1	0.1	1.48	SANDY SILT AND SILT	3	3	0.02	27	20	1.38 --
34.1	8.2	0.2	2.07	SANDY SILT AND SILT	4	3	0.01	27	20	1.38 --
34.2	9.5	0.2	1.79	SANDY SILT AND SILT	4	3	0.02	27	20	1.39 --
34.4	9.1	0.2	1.55	SANDY SILT AND SILT	4	3	0.02	27	20	1.39 --
34.5	8.9	0.2	1.91	SANDY SILT AND SILT	4	3	0.02	27	20	1.40 --
34.7	10.0	0.3	2.50	CLAYEY SILT AND SILTY CLAY	4	4	----	----	----	1.40 0.49
34.9	11.5	0.4	3.82	CLAYEY SILT AND SILTY CLAY	6	5	----	----	----	1.41 0.58
35.0	10.9	0.4	3.95	CLAYEY SILT AND SILTY CLAY	6	5	----	----	----	1.41 0.54
35.2	14.1	0.4	2.99	CLAYEY SILT AND SILTY CLAY	6	5	----	----	----	1.42 0.74
35.4	16.4	0.4	2.69	SANDY SILT AND SILT	7	6	0.01	30	20	1.42 --
35.5	15.0	0.0	0.07	SILTY SAND	4	3	0.10	29	20	1.43 --
35.7	12.2	0.4	3.51	CLAYEY SILT AND SILTY CLAY	6	5	----	----	----	1.43 0.63
35.9	5.4	0.4	6.62	CLAY	5	5	----	----	----	1.44 0.20
36.0	11.8	0.3	2.28	SANDY SILT AND SILT	5	4	0.02	28	20	1.44 --
36.2	10.9	0.3	2.48	SANDY SILT AND SILT	5	4	0.01	28	20	1.45 --
36.4	10.9	0.3	2.76	CLAYEY SILT AND SILTY CLAY	5	4	----	----	----	1.45 0.54
36.5	12.2	0.3	2.70	CLAYEY SILT AND SILTY CLAY	5	5	----	----	----	1.46 0.62
36.7	13.6	0.4	2.79	CLAYEY SILT AND SILTY CLAY	6	5	----	----	----	1.46 0.71
36.8	13.4	0.4	2.77	CLAYEY SILT AND SILTY CLAY	6	5	----	----	----	1.47 0.69
37.0	12.3	0.4	3.01	CLAYEY SILT AND SILTY CLAY	6	5	----	----	----	1.47 0.62
37.2	12.2	0.4	2.94	CLAYEY SILT AND SILTY CLAY	6	5	----	----	----	1.48 0.62
37.3	12.9	0.4	2.86	CLAYEY SILT AND SILTY CLAY	6	5	----	----	----	1.48 0.66
37.5	13.6	0.4	2.94	CLAYEY SILT AND SILTY CLAY	6	5	----	----	----	1.49 0.70
37.7	13.6	0.4	2.79	CLAYEY SILT AND SILTY CLAY	6	5	----	----	----	1.49 0.70
37.8	13.6	0.3	2.43	SANDY SILT AND SILT	6	5	0.02	29	20	1.50 --
38.0	14.3	0.3	2.10	SANDY SILT AND SILT	6	5	0.02	29	20	1.50 --
38.2	15.0	0.3	1.80	SANDY SILT AND SILT	5	4	0.03	29	20	1.51 --
38.3	14.3	0.2	1.26	SANDY SILT AND SILT	5	4	0.04	29	20	1.51 --
38.5	11.9	0.2	1.94	SANDY SILT AND SILT	5	4	0.02	28	20	1.52 --
38.6	15.0	0.4	2.54	SANDY SILT AND SILT	6	5	0.02	29	20	1.52 --
38.8	19.7	0.6	2.84	SANDY SILT AND SILT	8	7	0.02	30	20	1.53 --
39.0	24.5	1.0	4.08	CLAYEY SILT AND SILTY CLAY	12	10	----	----	----	1.54 1.38
39.1	34.0	1.3	3.71	CLAYEY SILT AND SILTY CLAY	15	13	----	----	----	1.54 1.97
39.3	36.0	1.5	4.19	CLAYEY SILT AND SILTY CLAY	18	14	----	----	----	1.55 2.10
39.5	44.9	1.6	3.63	SANDY SILT AND SILT	19	15	0.01	34	20	1.55 --
39.6	51.0	1.4	2.67	SANDY SILT AND SILT	17	14	0.04	35	23	1.56 --
39.8	50.5	1.4	2.72	SANDY SILT AND SILT	17	14	0.04	35	23	1.56 --
40.0	49.0	1.6	3.31	SANDY SILT AND SILT	19	15	0.02	35	22	1.57 --
40.1	58.6	1.9	3.22	SANDY SILT AND SILT	21	17	0.03	36	28	1.57 --
40.3	81.4	1.2	1.42	SILTY SAND	22	17	0.10	37	39	1.58 --
40.5	92.5	0.7	0.76	SAND	16	13	0.99	38	43	1.58 --
40.6	77.1	1.2	1.53	SILTY SAND	21	17	0.09	37	37	1.59 --
40.8	48.8	1.0	1.95	SILTY SAND	14	12	0.06	35	21	1.59 --

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 20

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
40.9	28.6	0.8	2.77	SANDY SILT AND SILT	11	9	0.03	32	20	1.60	--
41.1	19.6	0.5	2.60	SANDY SILT AND SILT	8	6	0.02	30	20	1.60	--
41.3	13.5	0.3	1.79	SANDY SILT AND SILT	6	5	0.04	30	20	1.61	--
41.4	18.4	0.3	1.33	SANDY SILT AND SILT	6	5	0.04	30	20	1.61	--
41.6	17.7	0.4	2.04	SANDY SILT AND SILT	6	5	0.03	30	20	1.62	--
41.8	23.1	0.5	1.95	SANDY SILT AND SILT	8	6	0.04	31	20	1.62	--
41.9	20.4	0.5	2.35	SANDY SILT AND SILT	8	6	0.03	30	20	1.63	--
42.1	24.5	0.9	3.30	CLAYEY SILT AND SILTY CLAY	12	9	---	---	---	1.63	1.37
42.3	31.0	1.1	3.51	CLAYEY SILT AND SILTY CLAY	14	11	---	---	---	1.64	1.77
42.4	54.3	1.0	1.82	SILTY SAND	16	12	0.07	35	24	1.64	--
42.6	73.5	0.5	0.59	SAND	12	10	0.91	36	34	1.65	--
42.7	82.3	0.6	0.67	SAND	14	11	1.04	37	38	1.65	--
42.9	77.5	1.4	1.75	SILTY SAND	21	17	0.08	37	36	1.66	--
43.1	65.3	1.3	1.98	SILTY SAND	19	15	0.07	36	30	1.66	--
43.2	70.0	1.4	1.96	SILTY SAND	20	16	0.07	36	32	1.67	--
43.4	65.3	1.4	2.11	SILTY SAND	19	15	0.07	36	30	1.67	--
43.6	66.1	1.5	2.21	SILTY SAND	19	15	0.06	36	30	1.68	--
43.7	64.3	1.1	1.68	SILTY SAND	18	14	0.08	36	29	1.68	--
43.9	69.4	0.7	0.98	SAND	16	12	0.38	36	32	1.69	--
44.1	74.5	0.8	1.13	SAND	18	14	0.25	36	34	1.69	--
44.2	74.8	1.3	1.75	SILTY SAND	21	16	0.08	36	34	1.70	--
44.4	75.5	1.0	1.34	SILTY SAND	20	15	0.10	36	34	1.70	--
44.6	74.0	0.9	1.26	SILTY SAND	20	15	0.10	36	34	1.71	--
44.7	81.0	0.6	0.78	SAND	15	11	0.87	37	37	1.71	--
44.9	84.3	0.5	0.58	SAND	14	10	1.18	37	38	1.72	--
45.0	83.0	0.6	0.59	SAND	14	11	1.02	37	38	1.73	--
45.2	82.0	0.7	0.82	SAND	15	12	0.83	37	37	1.73	--
45.4	69.4	1.1	1.51	SILTY SAND	19	14	0.09	36	31	1.74	--
45.5	56.5	1.2	2.07	SILTY SAND	17	13	0.06	35	24	1.74	--
45.7	59.2	0.7	1.20	SILTY SAND	16	12	0.09	35	25	1.75	--
45.9	48.6	0.6	1.25	SILTY SAND	13	10	0.08	34	20	1.75	--
46.0	28.8	0.7	2.36	SANDY SILT AND SILT	10	7	0.04	32	20	1.76	--
46.2	19.7	0.5	2.43	SANDY SILT AND SILT	8	6	0.03	30	20	1.76	--
46.4	16.3	0.4	2.14	SANDY SILT AND SILT	6	5	0.03	29	20	1.77	--
46.5	17.6	0.5	2.72	SANDY SILT AND SILT	7	6	0.02	29	20	1.77	--
46.7	23.1	0.6	2.38	SANDY SILT AND SILT	8	6	0.03	30	20	1.78	--
46.8	37.7	0.7	1.96	SILTY SAND	11	9	0.05	33	20	1.78	--
47.0	43.8	1.0	2.17	SILTY SAND	13	10	0.05	34	20	1.79	--
47.2	40.8	0.9	2.11	SILTY SAND	12	9	0.05	33	20	1.79	--
47.3	78.8	0.5	0.69	SAND	14	10	0.99	36	35	1.80	--
47.5	111.5	0.3	0.29	SAND	15	11	1.67	38	47	1.80	--
47.7	119.1	0.6	0.47	SAND	17	13	1.46	38	49	1.81	--
47.8	98.6	0.7	0.75	SAND	17	12	1.04	37	42	1.81	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 20

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
48.0	72.6	0.7	0.89	SAND	15	11	0.53	36	32	1.82	--
48.2	55.8	0.7	1.22	SILTY SAND	15	11	0.09	35	22	1.82	--
48.3	29.2	0.5	1.74	SILTY SAND	9	7	0.05	31	20	1.83	--
48.5	19.0	0.2	1.26	SANDY SILT AND SILT	6	4	0.05	29	20	1.83	--
48.7	16.5	0.3	1.64	SANDY SILT AND SILT	6	4	0.04	29	20	1.84	--
48.8	15.0	0.4	2.34	SANDY SILT AND SILT	6	4	0.02	28	20	1.84	--
49.0	16.1	0.4	2.30	SANDY SILT AND SILT	6	5	0.02	28	20	1.85	--
49.1	16.3	0.4	2.63	SANDY SILT AND SILT	7	5	0.02	28	20	1.85	--
49.3	15.6	0.3	2.05	SANDY SILT AND SILT	6	4	0.03	28	20	1.86	--
49.5	17.7	0.2	1.24	SANDY SILT AND SILT	6	4	0.05	29	20	1.86	--
49.6	16.1	0.3	1.93	SANDY SILT AND SILT	6	4	0.03	28	20	1.87	--
49.8	17.7	0.4	2.09	SANDY SILT AND SILT	6	5	0.03	29	20	1.87	--
50.0	19.0	0.3	1.68	SANDY SILT AND SILT	6	5	0.04	29	20	1.88	--
50.1	52.4	0.3	0.53	SAND	10	7	0.78	34	20	1.88	--
50.3	54.8	0.4	0.80	SAND	13	9	0.28	34	21	1.89	--
50.5	52.2	0.3	0.48	SAND	9	7	0.90	34	20	1.89	--
50.6	34.0	0.4	1.21	SILTY SAND	10	7	0.07	32	20	1.90	--
50.8	19.7	0.4	1.98	SANDY SILT AND SILT	7	5	0.03	29	20	1.90	--
51.0	14.3	0.2	1.26	SANDY SILT AND SILT	5	3	0.04	28	20	1.91	--
51.1	12.2	0.1	1.14	SANDY SILT AND SILT	4	3	0.04	27	20	1.92	--
51.3	10.9	0.1	1.29	SANDY SILT AND SILT	4	3	0.03	26	20	1.92	--
51.4	10.9	0.1	1.10	SANDY SILT AND SILT	4	3	0.04	26	20	1.93	--
51.6	10.9	0.1	1.29	SANDY SILT AND SILT	4	3	0.03	26	20	1.93	--
51.8	10.9	0.1	1.29	SANDY SILT AND SILT	4	3	0.03	26	20	1.94	--
51.9	13.6	0.1	0.88	SANDY SILT AND SILT	4	3	0.05	27	20	1.94	--
52.1	12.2	0.4	3.59	CLAYEY SILT AND SILTY CLAY	6	4	---	---	---	1.95	0.56
52.3	23.1	0.3	1.30	SILTY SAND	7	5	0.06	30	20	1.95	--
52.4	41.5	0.4	0.89	SILTY SAND	11	8	0.09	33	20	1.96	--
52.6	49.0	0.3	0.67	SAND	12	8	0.29	34	20	1.96	--
52.8	43.8	0.3	0.71	SILTY SAND	12	8	0.10	33	20	1.97	--
52.9	30.6	0.4	1.31	SILTY SAND	9	6	0.07	31	20	1.97	--
53.1	20.4	0.4	1.86	SANDY SILT AND SILT	7	5	0.04	29	20	1.98	--
53.2	14.9	0.3	2.01	SANDY SILT AND SILT	6	4	0.03	28	20	1.98	--
53.4	15.9	0.2	1.32	SANDY SILT AND SILT	5	4	0.04	28	20	1.99	--
53.6	15.6	0.2	1.35	SANDY SILT AND SILT	5	4	0.04	28	20	1.99	--
53.7	12.2	0.2	1.64	SANDY SILT AND SILT	4	3	0.03	27	20	2.00	--
53.9	12.2	0.1	0.82	SANDY SILT AND SILT	4	3	0.04	27	20	2.00	--
54.1	10.9	0.1	0.92	SANDY SILT AND SILT	4	3	0.04	26	20	2.01	--
54.2	9.5	0.1	1.16	SANDY SILT AND SILT	3	2	0.03	25	20	2.01	--
54.4	11.1	0.2	2.06	SANDY SILT AND SILT	5	3	0.02	26	20	2.02	--
54.6	18.4	0.2	1.14	SILTY SAND	6	4	0.05	29	20	2.02	--
54.7	11.6	0.3	2.51	SANDY SILT AND SILT	5	4	0.01	26	20	2.03	--
54.9	12.9	0.2	1.39	SANDY SILT AND SILT	5	3	0.03	27	20	2.03	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 20

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	PO TSF	SU TSF.	
55.1	19.4	0.1	0.62	SILTY SAND	5	4	0.08	29	20	2.04	--
55.2	12.2	0.0	0.25	SILTY SAND	3	2	0.08	27	20	2.04	--
55.4	10.9	0.3	2.30	SANDY SILT AND SILT	5	3	0.01	26	20	2.05	--
55.5	9.5	0.2	1.68	SANDY SILT AND SILT	4	3	0.02	25	20	2.05	--
55.7	13.0	0.3	2.08	SANDY SILT AND SILT	5	4	0.02	27	20	2.06	--
55.9	28.6	0.3	0.93	SILTY SAND	3	6	0.07	21	20	2.05	--
56.0	40.1	0.4	1.05	SILTY SAND	11	8	0.08	32	20	2.07	--
56.2	25.1	0.4	1.43	SILTY SAND	8	5	0.06	30	20	2.07	--
56.4	12.6	0.3	2.35	SANDY SILT AND SILT	6	4	0.02	27	20	2.08	--
56.5	21.1	0.4	1.66	SANDY SILT AND SILT	7	5	0.04	29	20	2.08	--
56.7	19.9	0.6	2.02	SANDY SILT AND SILT	9	6	0.01	29	20	2.09	--
56.9	48.9	0.6	1.12	SILTY SAND	13	9	0.09	33	20	2.09	--
57.0	61.9	0.5	0.72	SAND	12	8	0.66	34	22	2.10	--
57.2	49.3	0.3	0.63	SAND	11	7	0.43	33	20	2.10	--
57.3	32.6	0.2	0.70	SILTY SAND	9	6	0.09	31	20	2.11	--
57.5	21.0	0.3	1.24	SILTY SAND	6	4	0.05	29	20	2.12	--
57.7	14.0	0.2	1.42	SANDY SILT AND SILT	5	3	0.04	27	20	2.12	--
57.8	12.9	0.2	1.55	SANDY SILT AND SILT	5	3	0.03	27	20	2.13	--
58.0	13.2	0.1	0.91	SANDY SILT AND SILT	4	3	0.04	27	20	2.13	--
58.2	12.3	0.1	0.90	SANDY SILT AND SILT	4	3	0.04	26	20	2.14	--
58.3	11.5	0.1	0.87	SANDY SILT AND SILT	4	3	0.04	26	20	2.14	--
58.5	10.9	0.1	0.92	SANDY SILT AND SILT	4	2	0.04	26	20	2.15	--
58.7	10.2	0.1	1.18	SANDY SILT AND SILT	4	2	0.03	25	20	2.15	--
58.8	10.9	0.2	1.34	SANDY SILT AND SILT	4	3	0.02	26	20	2.15	--
59.0	13.0	0.1	0.93	SANDY SILT AND SILT	4	3	0.04	27	20	2.16	--
59.2	10.9	0.1	1.01	SANDY SILT AND SILT	4	2	0.04	26	20	2.17	--
59.3	10.5	0.1	1.33	SANDY SILT AND SILT	4	3	0.03	26	20	2.17	--
59.5	12.2	0.1	0.82	SANDY SILT AND SILT	4	3	0.04	26	20	2.18	--
59.6	10.5	0.1	0.86	SANDY SILT AND SILT	4	2	0.04	26	20	2.18	--
59.8	9.5	0.1	0.95	SANDY SILT AND SILT	3	2	0.04	25	20	2.19	--
60.0	10.2	0.1	0.98	SANDY SILT AND SILT	3	2	0.04	25	20	2.19	--
60.1	10.8	0.1	0.74	SANDY SILT AND SILT	4	2	0.04	26	20	2.20	--
60.3	10.4	0.1	0.77	SANDY SILT AND SILT	3	2	0.04	25	20	2.20	--
60.5	10.2	0.1	0.88	SANDY SILT AND SILT	3	2	0.04	25	20	2.21	--
60.6	10.9	0.2	1.56	SANDY SILT AND SILT	4	3	0.03	26	20	2.21	--
60.8	14.9	0.1	0.87	SANDY SILT AND SILT	5	3	0.05	27	20	2.22	--
61.0	27.8	0.4	1.29	SILTY SAND	8	5	0.06	30	20	2.22	--
61.1	23.1	0.6	2.64	SANDY SILT AND SILT	9	6	0.02	29	20	2.23	--
61.3	15.6	0.5	3.07	CLAYEY SILT AND SILTY CLAY	7	5	---	---	---	2.23	0.74
61.4	11.5	0.3	2.70	CLAYEY SILT AND SILTY CLAY	5	3	---	---	---	2.24	0.48
61.6	9.8	0.2	1.54	SANDY SILT AND SILT	4	2	0.03	25	20	2.24	--
61.7	9.5	0.1	1.26	SANDY SILT AND SILT	3	2	0.03	25	20	2.25	--
61.9	9.6	0.1	1.26	SANDY SILT AND SILT	3	2	0.03	25	20	2.25	--

(CONTINUED)

INTERPRETED CONE PENETRATION TEST DATA

C.P.T. - 20

DEPTH FT.	QC TSF.	FS TSF.	FR %	SOIL BEHAVIOR TYPE	SPT N N1	D50 MM	PHI DEG	DR %	P0 TSF	SU TSF.	
62.1	10.2	0.1	1.37	SANDY SILT AND SILT	4	2	0.03	25	20	2.26	--
62.2	10.6	0.1	1.32	SANDY SILT AND SILT	4	2	0.03	25	20	2.26	--
62.4	10.5	0.1	1.14	SANDY SILT AND SILT	4	2	0.03	25	20	2.27	--
62.6	10.2	0.1	1.18	SANDY SILT AND SILT	4	2	0.03	25	20	2.27	--
62.7	9.9	0.1	1.22	SANDY SILT AND SILT	4	2	0.03	25	20	2.27	--
62.9	10.2	0.1	1.27	SANDY SILT AND SILT	4	2	0.03	25	20	2.28	--
63.1	10.2	0.1	1.27	SANDY SILT AND SILT	4	2	0.03	25	20	2.28	--
63.2	10.3	0.2	1.74	SANDY SILT AND SILT	4	2	0.03	25	20	2.29	--
63.4	11.6	0.2	1.99	SANDY SILT AND SILT	4	3	0.02	25	20	2.29	--
63.5	12.0	0.2	1.92	SANDY SILT AND SILT	5	3	0.02	26	20	2.30	--
63.7	11.7	0.2	1.88	SANDY SILT AND SILT	5	3	0.02	26	20	2.30	--
63.9	12.3	0.2	1.79	SANDY SILT AND SILT	5	3	0.02	26	20	2.31	--
64.0	12.2	0.2	1.88	SANDY SILT AND SILT	5	3	0.03	26	20	2.31	--
64.2	12.2	0.2	1.88	SANDY SILT AND SILT	5	3	0.02	26	20	2.32	--
64.4	12.2	0.2	1.80	SANDY SILT AND SILT	5	3	0.03	26	20	2.32	--
64.5	12.9	0.3	2.01	SANDY SILT AND SILT	5	3	0.03	26	20	2.33	--
64.7	13.6	0.3	2.13	SANDY SILT AND SILT	5	3	0.02	26	20	2.34	--
64.9	12.5	0.2	1.77	SANDY SILT AND SILT	5	3	0.02	26	20	2.34	--
65.0	11.5	0.0	0.09	SILTY SAND	5	3	0.03	26	20	2.35	--
65.2	11.6	0.1	0.87	SANDY SILT AND SILT	3	2	0.09	26	20	2.35	--
65.4	7.2	0.1	1.39	SANDY SILT AND SILT	4	2	0.04	26	20	2.36	--
65.5	10.2	0.1	0.98	SANDY SILT AND SILT	3	2	0.02	23	20	2.36	--
65.7	10.7	0.1	1.03	SANDY SILT AND SILT	4	2	0.04	25	20	2.37	--
65.8	10.8	0.1	1.11	SANDY SILT AND SILT	4	2	0.04	25	20	2.37	--
66.0	10.9	0.1	1.29	SANDY SILT AND SILT	4	2	0.04	25	20	2.38	--
66.2	11.0	0.1	1.00	SANDY SILT AND SILT	4	2	0.03	25	20	2.38	--
66.3	10.2	0.1	0.98	SANDY SILT AND SILT	4	2	0.04	25	20	2.39	--
66.5	10.5	0.1	0.57	SANDY SILT AND SILT	3	2	0.04	25	20	2.39	--
66.7	11.6	0.6	5.45	SANDY SILT AND SILT	3	2	0.04	25	20	2.40	--
66.8	25.8	1.1	4.19	CLAY	12	7	---	---	---	2.40	0.46
67.0	307.4	1.8	0.60	CLAYEY SILT AND SILTY CLAY	13	8	---	---	---	2.41	1.35
67.2	442.8	3.1	0.71	SAND	41	26	1.74	42	82	2.41	--
				SAND	57	35	1.87	43	98	2.42	--

